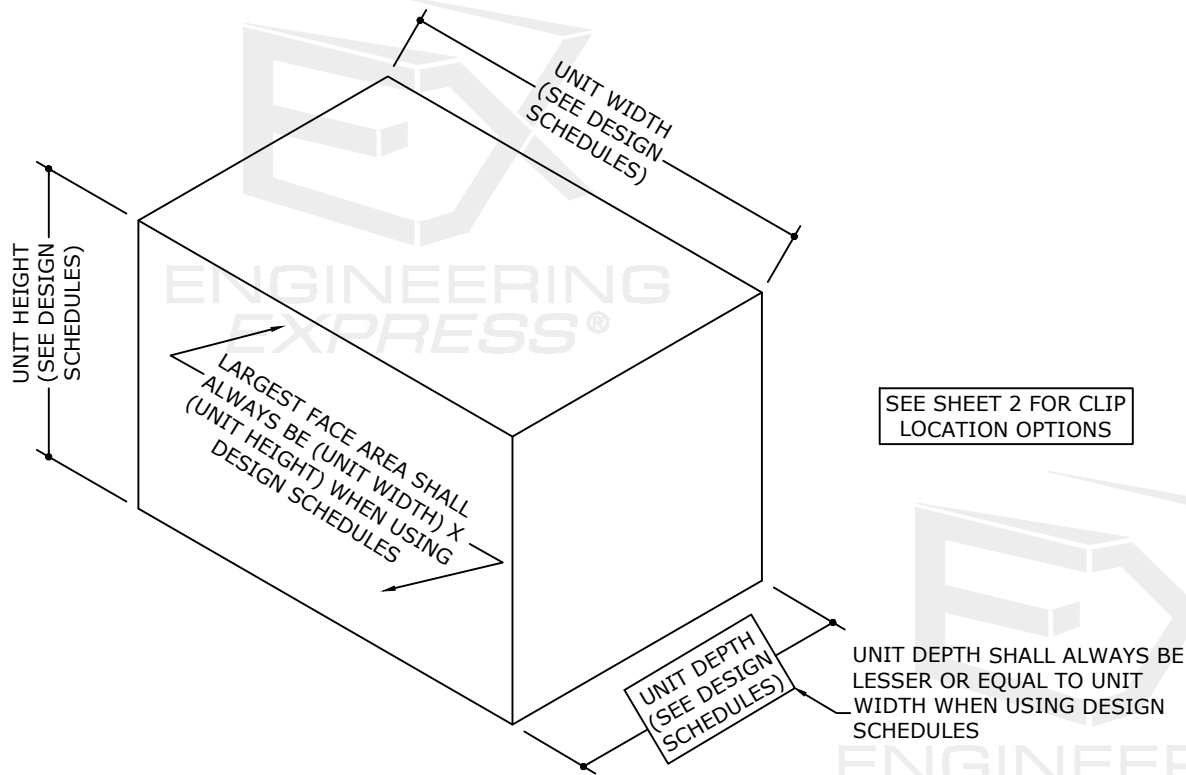
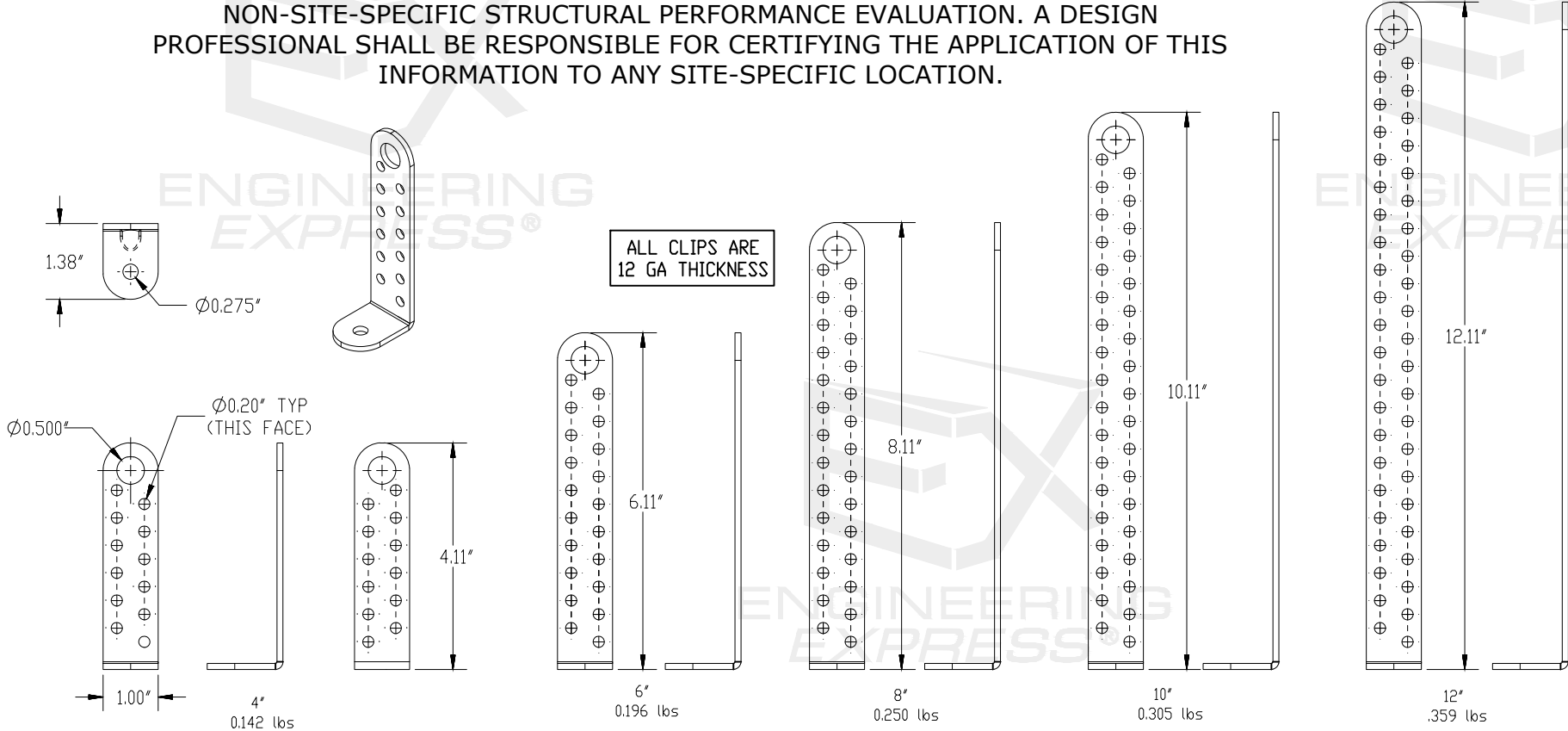


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HURRICANE TIE-DOWN CLIP (STEEL): AT GRADE & ROOF-TOP MOUNTED APPLICATIONS APPROVED FOR HVHZ / NON-HVHZ USE

NON-SITE-SPECIFIC STRUCTURAL PERFORMANCE EVALUATION. A DESIGN PROFESSIONAL SHALL BE RESPONSIBLE FOR CERTIFYING THE APPLICATION OF THIS INFORMATION TO ANY SITE-SPECIFIC LOCATION.



DESIGN NOTES:

- THIS PRODUCT HAS BEEN DESIGNED IN ACCORDANCE WITH THE CURRENT VERSION OF ASCE AND THE FLORIDA BUILDING CODE NINTH EDITION (2026) FOR USE WITHIN AND OUTSIDE THE HIGH VELOCITY HURRICANE ZONE AS INDICATED IN THE ACCOMPANYING DESIGN SCHEDULES. THE DESIGN CRITERIA USED TO CALCULATE THE REQUIRED DESIGN PRESSURES CONSIDERS FBC SECTION 1609 FOR NON-HVHZ AND SECTION 1620 FOR HVHZ (GC_r)_{Lateral} = 1.90 WITHIN THE HVHZ & OUTSIDE THE HVHZ, (GC_r)_{Uplift} = 1.5 FOR ALL LOCATIONS (CONCURRENT).
- ALL OTHER DESIGN VARIABLES ARE IN ACCORDANCE WITH ASCE 7 CHAPTERS 26 & 29.
- THE HEIGHTS LISTED IN THE ALLOWABLE PRESSURE SCHEDULE REPRESENTS THE MAXIMUM ALLOWABLE HEIGHT (MRH) OF THE BUILDING.
- DESIGN IS BASED ON CLIENT PROVIDED PRODUCT AND DIE SHEETS FROM TEST REPORT PROJECT #12-0967 BY SGS TESTING, ENGINEERING & CONSULTING SERVICES, INC. . NO SUBSTITUTIONS WITHOUT WRITTEN APPROVAL BY THIS ENGINEER SHALL BE PERMITTED.
- STEEL CLIPS SHALL BE 12ga ASTM A653 STEEL WITH F_y = 39 KSI OR BETTER. STEEL MEMBERS SHALL BE PROTECTED AGAINST CORROSION WITH AN APPROVED COAT OF PAINT, ENAMEL OR OTHER APPROVED PROTECTION. G90-RATED COATING REQUIRED FOR COASTAL INSTALLATIONS.

TERMINOLOGY:

(SEE NEXT PAGE FOR THIS SECTION).

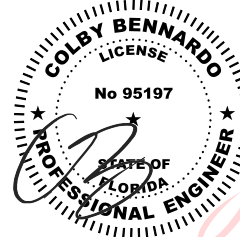
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GENERAL NOTES:

- MAXIMUM & MINIMUM DIMENSIONS AND MINIMUM WEIGHT OF MECHANICAL UNIT SHALL CONFORM TO SPECIFICATIONS STATED HEREIN. ALL MECHANICAL SPECIFICATIONS (CLEAR SPACE, TONNAGE, ETC.) SHALL BE AS PER MANUFACTURER RECOMMENDATIONS AND ARE THE EXPRESS RESPONSIBILITY OF THE CONTRACTOR.
- FASTENERS TO BE #8 X 3/4" OR GREATER SAE GR 5 OR STAINLESS STEEL 410 UNLESS NOTED OTHERWISE. ANCHORS REFERRED TO HEREIN SHALL BE DEWALT BRAND, INSTALLED TO 3000 PSI MIN CONCRETE. ALTERNATE CONCRETE ANCHORS MAY BE USED IF CERTIFIED BY OTHERS TO HAVE GREATER OR EQUAL ALLOWABLE TENSION/SHEAR CAPACITIES. SEE ANCHOR TO HOST SCHEDULE FOR ANCHOR REQUIREMENTS. ALL FASTENERS SHALL HAVE APPROPRIATE CORROSION PROTECTION TO PREVENT ELECTROLYSIS.
- FOR PERMANENT OUTDOOR INSTALLATIONS, ALL EXPOSED ANCHORS, FASTENERS, CONNECTORS, AND METAL PLATES SHALL BE CORROSION RESISTANT. CORROSION RESISTANCE MAY BE PROVIDED BY INHERENT MATERIAL COMPOSITION OR, WHERE PERMITTED BY CODE, BY AN APPROVED PROTECTIVE COATING OR GALVANIZATION. WHERE APPLICABLE NEAR A SALTWATER COASTLINE, SUCH COMPONENTS SHALL COMPLY WITH FBC SECTION 1711, INCLUDING APPLICABLE REQUIREMENTS FOR EXPOSED, PROTECTED, OR ENCLOSED CONDITIONS. MATERIALS, COATINGS, AND FINISHES SHALL BE COMPATIBLE WITH CONNECTED MATERIALS, AND PROTECTIVE COATINGS SHALL BE CONTINUOUS OVER EXPOSED SURFACES. FASTENERS USED WITH ALUMINUM MEMBERS SHALL ALSO COMPLY WITH SECTION J.3.7.2 OF THE ALUMINUM DESIGN MANUAL, "SPECIFICATION FOR ALUMINUM STRUCTURES." SUBSTITUTION OF COMPONENTS FROM ALTERNATE MANUFACTURERS, OR WITH DIFFERENT MATERIALS, COATINGS, OR FINISHES, SHALL NOT BE ASSUMED EQUIVALENT AND MAY REQUIRE ADDITIONAL ENGINEERING REVIEW OR SITE-SPECIFIC ANALYSIS.
- ALL CONCRETE SPECIFIED HEREIN IS NOT PART OF THIS CERTIFICATION. AS A MINIMUM, ALL CONCRETE SHALL BE STRUCTURAL CONCRETE 4" MIN. THICK AND SHALL HAVE MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI, UNLESS NOTED OTHERWISE.
- THE CONTRACTOR IS RESPONSIBLE TO INSULATE ALL MEMBERS FROM DISSIMILAR MATERIALS TO PREVENT ELECTROLYSIS.
- ELECTRICAL GROUND, WHEN REQUIRED, TO BE DESIGNED & INSTALLED BY OTHERS.
- THE ADEQUACY OF ANY EXISTING STRUCTURE TO WITHSTAND SUPERIMPOSED LOADS SHALL BE VERIFIED BY THE ONSITE DESIGN PROFESSIONAL AND IS NOT INCLUDED IN THIS CERTIFICATION. EXCEPT AS EXPRESSLY PROVIDED HEREIN, NO ADDITIONAL CERTIFICATIONS OR AFFIRMATIONS ARE INTENDED.
- THE SYSTEM DETAILED HEREIN IS GENERIC AND DOES NOT PROVIDE INFORMATION FOR A SPECIFIC SITE. FOR SITE CONDITIONS DIFFERENT FROM THE CONDITIONS DETAILED HEREIN, A LICENSED ENGINEER OR REGISTERED ARCHITECT SHALL PREPARE SITE SPECIFIC DOCUMENTS FOR USE IN CONJUNCTION WITH THIS DOCUMENT.
- WATER-TIGHTNESS OF EXISTING HOST SUBSTRATE SHALL BE THE FULL RESPONSIBILITY OF THE INSTALLING CONTRACTOR. CONTRACTOR SHALL ENSURE THAT ANY REMOVED OR ALTERED WATERPROOFING MEMBRANE IS RESTORED AFTER FABRICATION AND INSTALLATION OF STRUCTURE PROPOSED HEREIN. THIS ENGINEER SHALL NOT BE RESPONSIBLE FOR ANY WATERPROOFING OR LEAKAGE ISSUES WHICH MAY OCCUR AS WATER-TIGHTNESS SHALL BE THE FULL RESPONSIBILITY OF THE INSTALLING CONTRACTOR.
- FOR AN EXPLANATION OF EXPOSURE AND RISK CATEGORIES THAT ACCOMPANY THE VULT WIND SPEEDS USED IN THIS APPROVAL, SEE SECTION 26 OF ASCE 7.
- ENGINEER SEAL AFFIXED HERETO VALIDATE STRUCTURAL DESIGN AS SHOWN ONLY. USE OF THIS SPECIFICATION BY CONTRACTOR, et. al. INDEMNIFIES & SAVES HARMLESS THIS ENGINEER FOR ALL COST & DAMAGES INCLUDING LEGAL FEES & APPELLATE FEES RESULTING FROM MATERIAL FABRICATION, SYSTEM ERECTION, & CONSTRUCTION PRACTICES BEYOND THAT WHICH IS CALLED FOR BY LOCAL, STATE & FEDERAL CODES & FROM DEVIATIONS OF THIS PLAN.
- EXCEPT AS EXPRESSLY PROVIDED HEREIN, NO ADDITIONAL CERTIFICATIONS OR AFFIRMATIONS ARE INTENDED.

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HURRICANE TIE-DOWN CLIP (STEEL)

FLORIDA BUILDING CODE NINTH EDITION (2026)

FLORIDA STATEWIDE APPROVAL (FSA FL#31178.1)

REMARKS	DRWN	CHKD	DATE
INIT ISSUE	RWN	RWN	12/16/19
2020 FBC (20-31532)	CCB	RWN	9/16/20
2023 FBC (23-61184)	MLD	CCB	8/16/23
2026 FBC UPDATE	NRD	EPR	07/28/26

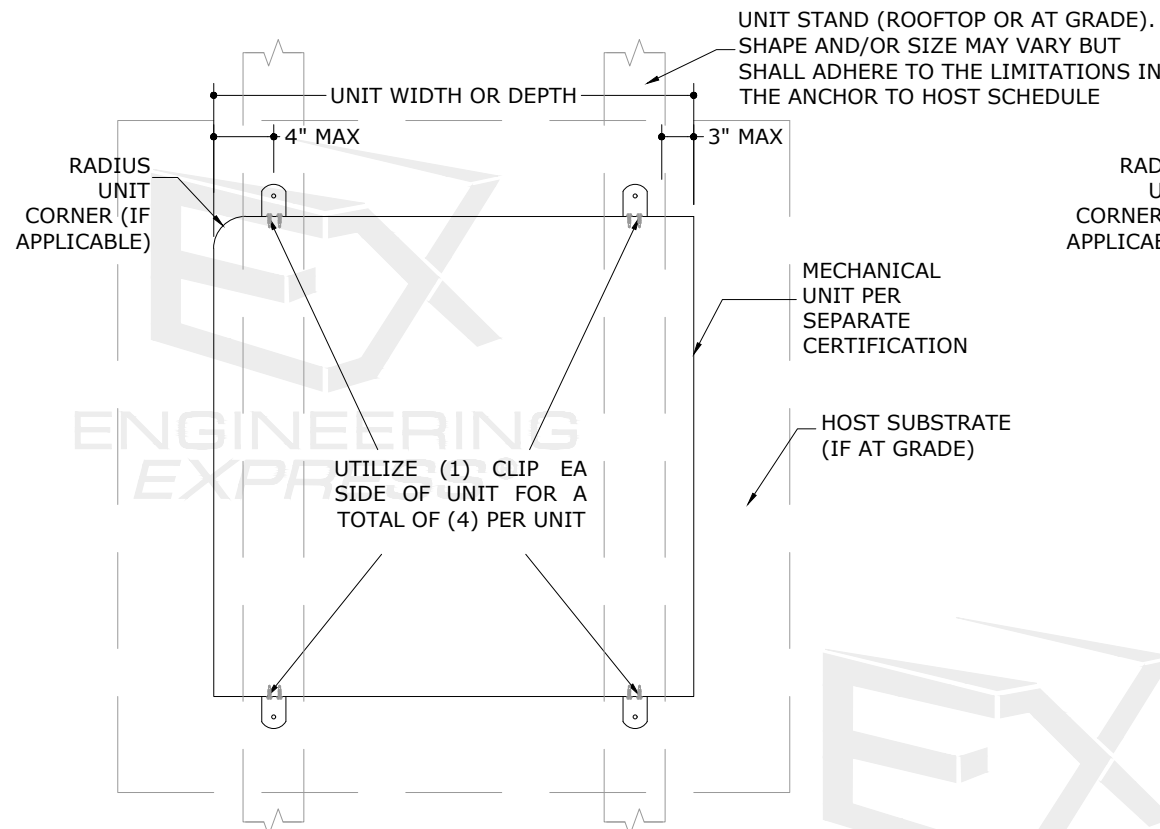
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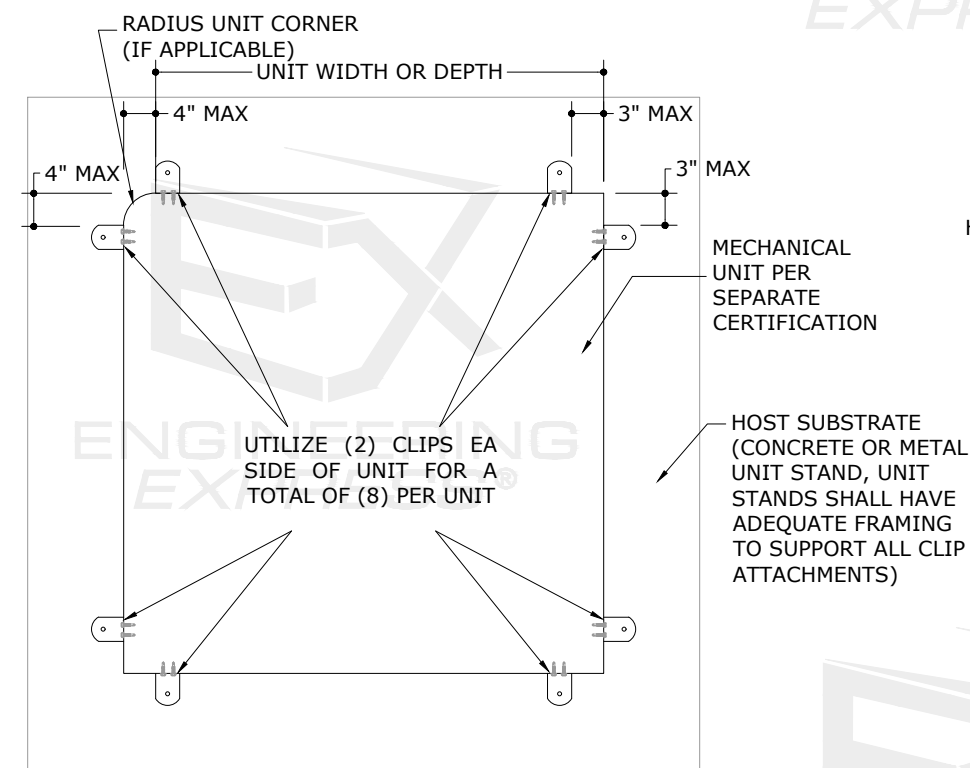
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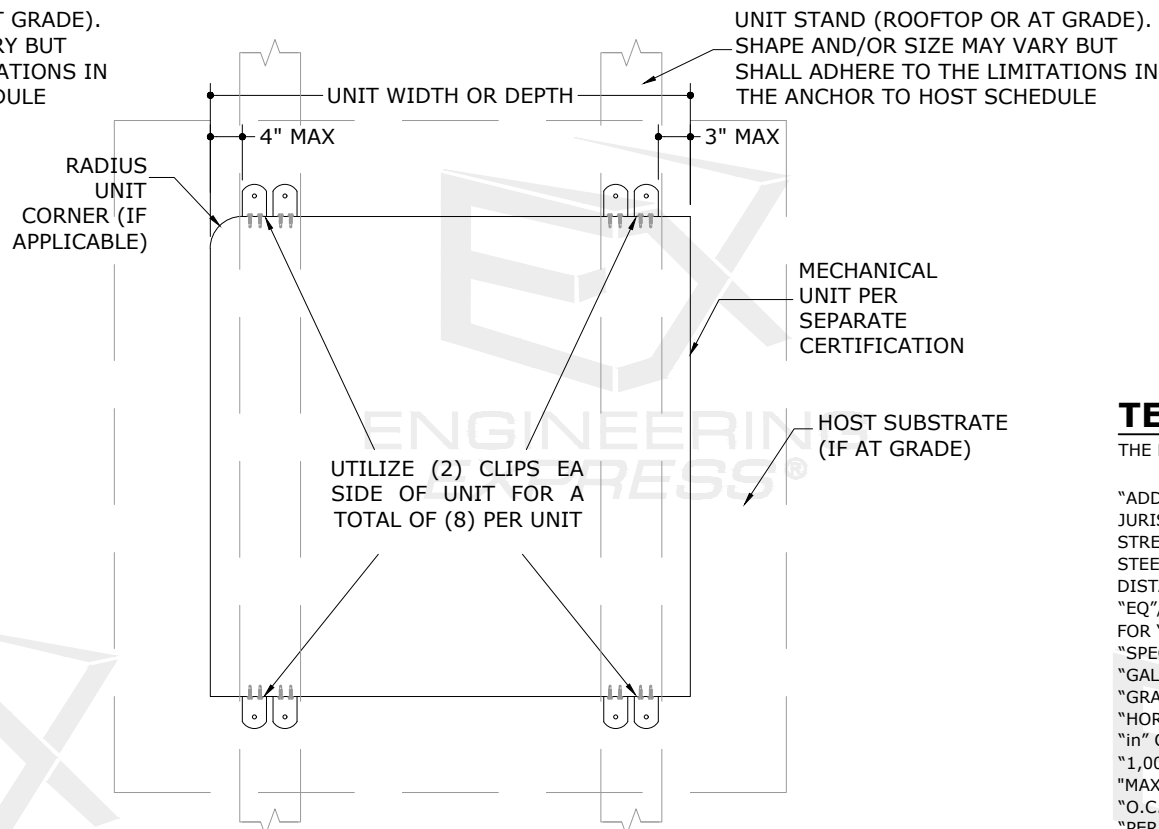


1 (4) CLIP DETAIL (ROOFTOP OR AT GRADE)
2 N.T.S. PLAN VIEW



3 ALT. (8) CLIP DETAIL (ROOFTOP OR AT GRADE)
2 N.T.S. PLAN VIEW

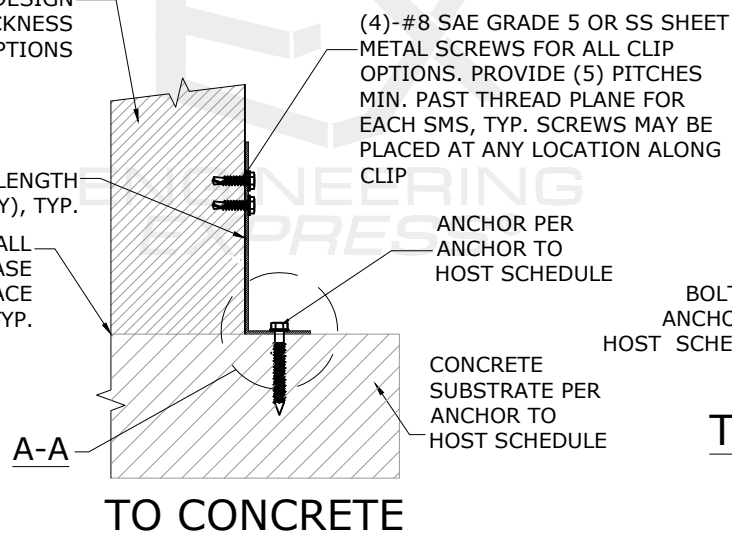
THIS DETAIL MAY BE USED AS AN ALTERNATE GEOMETRIC PATTERN FOR ALL CONNECTION TYPES THAT UTILIZE (2) CLIPS AT EACH CORNER FOR A TOTAL OF (8) CLIPS PER UNIT.



2 (8) CLIP DETAIL (ROOFTOP OR AT GRADE)
2 N.T.S. PLAN VIEW

MECHANICAL UNIT BY OTHERS. STEEL HOUSING SHALL BE ASTM A653 Fy=33KSI MIN. STEEL, GRADE 33. SEE DESIGN SCHEDULES FOR HOUSING THICKNESS OPTIONS

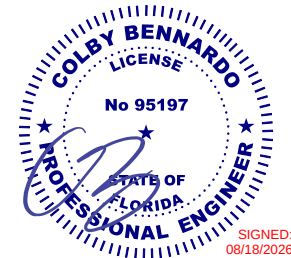
TIE-DOWN CLIP (LENGTH MAY VARY), TYP.
BASE OF UNIT SHALL BE FLUSH WITH BASE OF CLIP, NO SPACE PERMITTED, TYP.



4 TIE-DOWN CLIP ANCHOR DETAIL
2 NTS SECTION VIEW

CLIP IS DESIGNED FOR FULL CONTACT WITH THE BASE OF EACH MECHANICAL UNIT, TYP.

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TERMINOLOGY:

THE FOLLOWING ABBREVIATIONS MAY APPEAR IN THIS APPROVAL:

"ADDTL." FOR "ADDITIONAL", "AHJ" FOR "AUTHORITY HAVING JURISDICTION", "ALUM" FOR "ALUMINUM", "ASD" FOR "ALLOWABLE STRESS DESIGN", "BO" FOR "BUILD-OUT", "CS" FOR "CARBON STEEL", "EA." FOR "EACH", "E.D."/"EDGE"/"EDGE DIST." FOR "EDGE DISTANCE", "ELEV" FOR "ELEVATION", "EMBED" FOR "EMBEDMENT", "EQ"/"EQUIV." FOR "EQUIVALENT", "EXT" FOR "EXTERIOR", "FBC" FOR "FLORIDA BUILDING CODE", "ft" OR " ' " FOR "FEET", "G" FOR "SPECIFIC GRAVITY", "GA" FOR "GAUGE", "GALV" FOR "GALVANIZED", "GFB" FOR "GROUT-FILLED BLOCK", "GR" FOR "GRADE", "HOLLOW" FOR "HOLLOW BLOCK", "HORIZ" FOR "HORIZONTAL", "HVHZ" FOR "HIGH-VELOCITY HURRICANE ZONE", "in" OR " \" " FOR "INCHES", "INT" FOR "INTERIOR", "KSI" FOR "1,000 lb / in²", "L" FOR "LENGTH", "LB" FOR "POUND", "MAX" FOR "MAXIMUM", "MIN" FOR "MINIMUM", "N.T.S." FOR "NOT TO SCALE", "O.C." FOR "ON-CENTER", "P.E." FOR "PROFESSIONAL ENGINEER", "PERP" FOR "PERPENDICULAR", "PSF" FOR "POUNDS PER SQUARE FOOT (lb/ft²)", "PSI" FOR "POUNDS PER SQUARE INCH (lb/in²)", "QTY" FOR "QUANTITY", "REF." FOR "REFERENCE", "SCHED." FOR "SCHEDULE", "SDS" FOR "SELF-DRILLING SCREWS", "SMS" FOR "SHEET METAL SCREWS", "SPECS" FOR "SPECIFICATIONS", "SS" FOR "STAINLESS STEEL", "SUB" FOR "SUBMITTAL", "TAS" FOR "TESTING APPLICATION STANDARD", "TYP." FOR "TYPICAL", "ULT" FOR "ULTIMATE LOADS", "U.N.O." FOR "UNLESS NOTED OTHERWISE", "UTS" OR "Fu" FOR "ULTIMATE TENSILE STRENGTH/STRESS", "VERT" FOR "VERTICAL", "WLL" FOR "WORKING LOAD LIMIT", "W/" FOR "WITH", "W/O" FOR "WITHOUT", "YS" FOR "YIELD STRENGTH", "#" FOR "NUMBER", "&" FOR "AND", AND "Ø" FOR "DIAMETER".

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TABLE 1: CLIP DESIGN SCHEDULE (18 GA MIN. UNIT HOUSING)

MAX UNIT HEIGHT	MAX UNIT FACE AREA	MIN UNIT DEPTH	MAX ALLOWABLE DESIGN (ASD) PRESSURES W/ (1) CLIP PER CORNER		MAX ALLOWABLE DESIGN (ASD) PRESSURES W/ (2) CLIPS PER CORNER		MIN UNIT WEIGHT (LB)
			LATERAL DESIGN PRESSURE	UPLIFT DESIGN PRESSURE	LATERAL DESIGN PRESSURE	UPLIFT DESIGN PRESSURE	
24 in	576 in^2	15 in	133.0 psf	105.0 psf	180.0 psf	142.1 psf	50
		24 in	154.0 psf	121.6 psf	180.0 psf	142.1 psf	50
36 in	1296 in^2	15 in	49.0 psf	38.7 psf	96.0 psf	75.8 psf	75
		24 in	65.0 psf	51.3 psf	127.0 psf	100.3 psf	125
		36 in	76.0 psf	60.0 psf	144.0 psf	113.7 psf	200
48 in	2304 in^2	24 in	33.0 psf	26.1 psf	63.0 psf	49.7 psf	150
		36 in	40.0 psf	31.6 psf	77.0 psf	60.8 psf	200
		48 in	45.0 psf	35.5 psf	83.0 psf	65.5 psf	300
60 in	3600 in^2	36 in	24.0 psf	18.9 psf	45.0 psf	35.5 psf	200
		48 in	26.0 psf	20.5 psf	50.0 psf	39.5 psf	250
		60 in	30.0 psf	23.7 psf	54.0 psf	42.6 psf	350

TABLE 2: CLIP DESIGN SCHEDULE (22 GA MIN. UNIT HOUSING)

MAX UNIT HEIGHT	MAX UNIT FACE AREA	MIN UNIT DEPTH	MAX ALLOWABLE DESIGN (ASD) PRESSURES W/ (1) CLIP PER CORNER		MAX ALLOWABLE DESIGN (ASD) PRESSURES W/ (2) CLIPS PER CORNER		MIN UNIT WEIGHT (LB)
			LATERAL DESIGN PRESSURE	UPLIFT DESIGN PRESSURE	LATERAL DESIGN PRESSURE	UPLIFT DESIGN PRESSURE	
24 in	576 in^2	15 in	122.0 psf	96.3 psf	180.0 psf	142.1 psf	50
		24 in	140.0 psf	110.5 psf	180.0 psf	142.1 psf	50
36 in	1296 in^2	15 in	45.0 psf	35.5 psf	89.0 psf	70.3 psf	75
		24 in	59.0 psf	46.6 psf	115.0 psf	90.8 psf	125
		36 in	68.0 psf	53.7 psf	129.0 psf	101.8 psf	200
48 in	2304 in^2	24 in	30.0 psf	23.7 psf	58.0 psf	45.8 psf	150
		36 in	36.0 psf	28.4 psf	69.0 psf	54.5 psf	200
		48 in	40.0 psf	31.6 psf	75.0 psf	59.2 psf	300
60 in	3600 in^2	36 in	22.0 psf	17.4 psf	41.0 psf	32.4 psf	200
		48 in	24.0 psf	18.9 psf	46.0 psf	36.3 psf	250
		60 in	27.0 psf	21.3 psf	50.0 psf	39.5 psf	350

TABLE 3: ANCHOR TO HOST SCHEDULE

SUBSTRATE	DESCRIPTION
CONCRETE: (2-5/8" THICK MIN, 3000 PSI MIN.)	(1)-1/4"Ø DEWALT OR DEWALT ULTRACON, 1¾" FULL EMBED TO CONCRETE, 1 1/4" MIN. EDGE DISTANCE, 3" MIN. SPACING TO ANY ADJACENT ANCHOR.
ALUMINUM: (0.090" MIN. THICK, 6061-T6 MIN. ALUMINUM)	(1)-#14 SAE GR 5 OR STAINLESS STEEL BOLT 410 WITH NUT AND WASHER TOP & BOTTOM SS OD 1", ½" MINIMUM EDGE DISTANCE TO METAL EDGE
STEEL: (0.100" MIN. THICK, 36 KSI MIN. STEEL)	(1)-#14 SAE GR 5 OR STAINLESS STEEL BOLT 410 WITH NUT AND WASHER TOP & BOTTOM SS OD 1", ½" MINIMUM EDGE DISTANCE TO METAL EDGE

TABLE 3 NOTES:

1. EMBEDMENT AND EDGE DISTANCE EXCLUDES FINISHES, IF APPLICABLE.
2. ENSURE MINIMUM EDGE DISTANCE AS NOTED IN ANCHOR SCHEDULE.
3. ENSURE MINIMUM SPACING TO ANY ADJACENT ANCHORS.
4. SEE DETAILS ON SHEET 2 FOR ANCHORS ATTACHING TO MECHANICAL UNIT.
5. PROTECT ALL METALS FROM DISSIMILAR METALS GENERAL NOTE #5.
6. ALTERNATE HOST ANCHORS MAY BE USED IN LIEU OF THE OPTIONS LISTED IN THIS TABLE AS LONG AS THEY MEET THE FOLLOWING MINIMUM REQUIREMENTS:
 - ALLOWABLE (ASD) TENSION CAPACITY = 428 LBS
 - ALLOWABLE (ASD) SHEAR CAPACITY = 445 LBS

TABLE 1 & 2 NOTES:

1. UTILIZE THE UNIT DIMENSION DIRECTIVE ON SHEET 1 FOR DEFINITIONS OF UNIT HEIGHT, UNIT DEPTH AND FACE AREA.
2. MAXIMUM ALLOWABLE ANCHOR CLIP TENSION AND SHEAR LOADINGS UTILIZED TO CREATE THESE SCHEDULES ARE BASED ON TESTED DATA PER THE TEST REPORT LISTED IN THE DESIGN NOTES WITH A SAFETY FACTOR OF 2.
3. INDIVIDUAL UNIT WEIGHTS SHALL NOT BE LESS THAN THE WEIGHTS LISTED IN THESE DESIGN SCHEDULES.
4. LINEAR INTERPOLATION BETWEEN UNIT FACE AREAS AND UNIT DEPTHS IS PERMITTED.
5. REQUIRED ASD DESIGN PRESSURES SHALL BE LESS THAN OR EQUAL TO THE ALLOWABLE ASD PRESSURES WITHIN THESE DESIGN SCHEDULES.
6. THE REQUIRED DESIGN PRESSURE SCHEDULE (TABLE 4) MAY BE USED TO FIND REQUIRED DESIGN PRESSURES OR REQUIRED DESIGN PRESSURES SHALL BE CALCULATED SEPARATELY.

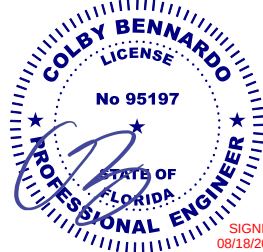
TABLE 4: REQUIRED DESIGN PRESSURE SCHEDULE

MAXIMUM MEAN ROOF HEIGHT	ULTIMATE WIND SPEED	EXPOSURE CATEGORY	LATERAL DESIGN PRESSURE (ASD)	UPLIFT DESIGN PRESSURE (ASD)
AT GRADE	140 mph	C	43.7 psf	34.5 psf
		D	53.0 psf	41.9 psf
25 ft		C	50.4 psf	39.8 psf
		D	59.7 psf	47.1 psf
50 ft		C	57.3 psf	45.3 psf
		D	66.4 psf	52.4 psf
75 ft		C	62.1 psf	49.0 psf
		D	70.9 psf	56.0 psf
100 ft		C	65.8 psf	51.9 psf
		D	74.3 psf	58.7 psf
150 ft		C	71.4 psf	56.4 psf
		D	79.6 psf	62.8 psf
AT GRADE	170 mph	C	64.5 psf	50.9 psf
		D	78.2 psf	61.8 psf
25 ft		C	74.3 psf	58.7 psf
		D	90.0 psf	69.4 psf
50 ft		C	84.6 psf	66.8 psf
		D	97.9 psf	77.3 psf
75 ft		C	91.5 psf	72.3 psf
		D	104.5 psf	82.5 psf
100 ft		C	97.0 psf	76.6 psf
		D	109.6 psf	86.5 psf
150 ft		C	105.3 psf	83.1 psf
		D	117.3 psf	92.6 psf
200 ft		C	111.7 psf	88.2 psf
		D	123.2 psf	97.3 psf
AT GRADE	175 mph	C	68.3 psf	53.9 psf
		D	82.9 psf	65.4 psf
25 ft		C	78.7 psf	62.2 psf
		D	93.2 psf	73.6 psf
50 ft		C	89.6 psf	70.7 psf
		D	103.7 psf	81.9 psf
75 ft		C	97.0 psf	76.6 psf
		D	110.8 psf	87.4 psf
100 ft		C	102.8 psf	81.1 psf
		D	116.2 psf	91.7 psf
150 ft		C	111.6 psf	88.1 psf
		D	124.3 psf	89.2 psf
200 ft		C	118.4 psf	93.4 psf
		D	130.5 psf	103.1 psf

TABLE 4 NOTES:

1. THE REQUIRED ASD DESIGN PRESSURES LISTED IN THIS SCHEDULE WERE CALCULATED BASED ON THE FOLLOWING PARAMETERS:
 - 1.1. ASCE 7 SECTION 29.5 "ROOFTOP STRUCTURES AND EQUIPMENT"
 - 1.2. STRUCTURE SHAPE = SQUARE
 - 1.3. z = 4.5 FT
 - 1.4. LATERAL GC_f = 1.90; UPLIFT GC_f = 1.50
2. THIS REQUIRED PRESSURE SCHEDULE IS FOR REFERENCE ONLY AND SHALL BE APPROVED BY THE AUTHORITY HAVING JURISDICTION AS BEING ADEQUATE FOR USE.
3. IF THE DESIGN PRESSURES LISTED IN THE SCHEDULE ARE NOT USED, REQUIRED DESIGN PRESSURES SHALL BE CALCULATED ON A SITE SPECIFIC BASIS BY OTHERS.

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