Draft 2006 International Building Code, Building, Chapters 20-24, Modified for the 2007 Florida Building Code

Yellow = Florida Specific Text

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
CHAPTER 20 Aluminum	**LIGHT METAL ALLOYS**	
SECTION 2001 **GENERAL**	**SECTION 2001** **GENERAL**	
2001.1 Scope. This chapter shall govern the quality, design, fabrication and erection of aluminum.	**2001.1 Scope.** Provisions of this chapter shall govern the quality, design, fabrication and erection of light metal alloys used in building construction. **Exception:** Buildings and structures located within the high-velocity hurricane zone shall comply with the provisions of Section 2003.	No overlap exists. Move Florida specific language forward
SECTION 2002 **MATERIALS**	**SECTION 2002** **STRUCTURAL ALUMINUM**	
2002.1 General. Aluminum used for structural purposes in buildings and structures shall comply with AA ASM 35 and AA ADM 1. The nominal loads shall be the minimum design loads required by Chapter 16.	**2002.1 General.** The quality, design, fabrication and erection of aluminum used structurally in buildings or structures shall conform to good engineering practice, the provisions of this chapter and other applicable requirements of this code. **Exception:** All buildings located within the high-velocity hurricane zone shall comply with the requirements of Section 2003.	No overlap exists. Move Florida specific language forward
NA **NA**	**2002.2 Structural aluminum construction.** The design, fabrication and assembly of structural aluminum for buildings or structures shall conform to AA ASM 35 and Specifications for Aluminum Structures, Aluminum Design Manual, Part 1-A and 1-B, of the Aluminum Association. The use of aluminum alloys not listed in the manual shall be permitted provided their standard of performance is not less than those required in the manual and the performance is substantiated to the satisfaction of the building official.	No overlap exists. Move Florida specific language forward
NA	**2002.2.1 Definitions** **PRIMARY MEMBER.** Structural framing members providing structural support to other members and/or surfaces of a structure including, but not limited to beams, posts, columns, joists, structural gutters, headers, purlins etc. **SECONDARY MEMBERS.** Structural framing members which do not provide basic support for the entire structure, generally including, but not limited to, such members as	No overlap exists. Move Florida specific Table forward

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	kickplate rails, chair rails, roof or wall panels, etc. **STRUCTURAL MEMBERS.** Members or sections that provide support of to an assembly and/or resist applied loads.	
NA	**2002.3 Screen enclosures.** **2002.3.1** Actual wall thickness of extruded aluminum members shall be not less than 0.040 inch (1 mm). **2002.3.2** Reserved. ~~Screen density shall be a maximum of 20 x 20 mesh.~~	(See 2006 Supplement) No overlap exists. Move Florida specific Table forward
NA	**2002.3.3 Vinyl and acrylic panels shall be removable.** Removable panels shall be identified as removable by a decal. The identification decal shall essentially state "Removable panel SHALL be removed when wind speeds exceed 75 mph (34 m/s)." Decals shall be placed such that the decal is visible when the panel is installed.	No overlap exists. Move Florida specific Table forward
NA	**2002.4 ~~Design.~~** ~~Structural members supporting screen enclosures shall be designed to support minimum wind loads given in Table 2002.4. Where any value is less than 10 psf (479 Pa) use 10 psf.~~ Loads. Structural members supporting screened enclosures shall be designed for wind in either of two orthogonal directions using the pressures given in Table 2002.4. Each primary member shall also be designed for a 300 lb (1.33 kN) load applied vertically downward along any 1 ft (0.3 m) of any member, not occurring simultaneously with wind load.	(See 2006 Supplement) No overlap exists. Move Florida specific Table forward
NA	**Table 2002.4** DESIGN WIND PRESSURES FOR ALUMINUM SCREEN ENCLOSURE FRAMING WITH AN IMPORTANCE FACTOR OF 0.771,2,3	(See 2006 Supplement) No overlap exists. Move Florida specific Table forward
NA	**2002.4.1** The following design guides shall be accepted as conforming to accepted engineering practices: AAF Guide to Aluminum Construction in High Wind Areas. **2002.5** Wall panels. The minimum thickness for formed sheet aluminum structural wall panels shall be not less than 0.024 inch (0.6 mm), subject to approved tolerances.	No overlap exists. Move Florida specific language forward
NA	**2002.6 Sunrooms**. Sunrooms shall comply with AAMA/NPEA/NSA 2100 with the structural requirements and	No overlap exists. Move Florida specific language

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	testing provisions of Chapter 5 modified to incorporate ASCE 7.	forward
NA	**Section 2003 High-Velocity Hurricane Zones- Aluminum**	No overlap exists. Move Florida specific language forward
CHAPTER 21	**MASONRY**	
SECTION 2101 GENERAL	**SECTION 2101 GENERAL**	
2101.1 Scope. This chapter shall govern the materials, design, construction and quality of masonry.	**2101.1 Scope.** This chapter shall govern the materials, design, construction and quality of masonry. **Exception**: Buildings and structures located within the high-velocity hurricane zone shall comply with the provisions of Sections 2114 and Sections 2118 through 2122.	No overlap exists. Move Florida specific language forward
2101.2.1 Allowable stress design. Masonry designed by the allowable stress design method shall comply with the provisions of Sections 2106 and 2107.	**2101.2.1 Working stress design.** Masonry designed by the working stress design method shall comply with the provisions of Sections 2106 and 2107.	Review this section
2101.2.3 Prestressed masonry. Prestressed masonry shall be designed in accordance with Chapters 1 and 4 of ACI 530/ASCE 5/TMS 402 and Section 2106. Special inspection during construction shall be provided as set forth in Section 1704.5.	**2101.2.3 Prestressed masonry.** Prestressed masonry shall be designed in accordance with Chapters 1 and 4 of ACI 530/ASCE 5/TMS 402.	No overlap exists. Move Florida specific language forward
NA	**2101.2.7 Prescriptive methods.** Masonry construction is permitted in accordance with applicable standards reference in Section 1609.1.1.	No overlap exists. Move Florida specific language forward
SECTION 2102 DEFINITIONS AND NOTATIONS	**SECTION 2102 DEFINITIONS AND NOTATIONS** **SHEAR WALL.** A wall designed to resist lateral forces parallel to the plane of the wall.	No overlap exists. Move Florida specific language forward
SECTION 2104 CONSTRUCTION	**SECTION 2104 CONSTRUCTION**	

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NA	**2104.1.9 Bracing of masonry.** Bracing that will ensure stability of masonry during construction shall be provided and installed. Bracing shall be in accordance with the Standard Practice for Bracing Masonry Walls Under Construction. **Exceptions:** 1. Bracing shall not be required for the unsupported wall heights specified in the Standard Practice for Bracing Masonry Walls Under Construction, Appendix A, when an evacuation system complying with the Standard Practice for Bracing Masonry Walls Under Construction with Commentary is provided. 2.Walls 8 feet (2438 mm) and less in height above grade shall not require bracing.	No overlap exists. Move Florida specific language forward
2105.1 General. A quality assurance program shall be used to ensure that the constructed masonry is in compliance with the construction documents. The quality assurance program shall comply with the inspection and testing requirements of Chapter 17.	**2105.1 General.** A quality assurance program shall be used to ensure that the constructed masonry is in compliance with the construction documents.	No overlap exists. Move Florida specific language forward
Section 2106 Seismic Design	**Section 2106 Seismic Design. Reserved**	No overlap exists. Move Florida specific language forward
SECTION 2107 **ALLOWABLE STRESS DESIGN**	**SECTION 2107** **WORKING STRESS DESIGN**	
2107.1 General. The design of masonry structures using allowable stress design shall comply with Section 2106 and the requirements of Chapters 1 and 2 of ACI 530/ASCE 5/TMS 402 except as modified by Sections 2107.2 through 2107.8.	**2107.1 General.** The design of masonry structures using working stress design shall comply with Section 2106 and the requirements of Chapters 1 and 2, except Section 2.1.2.1 and 2.1.3.3 of ACI 530/ASCE 5/TMS 402. The text of ACI 530/ASCE 5/TMS 402 shall be modified as follows. **Exception:** Where inspections are performed by a local building department in accordance with 105, the provisions of ACI 530/ASCE 5/TMS 402, Chapter 1, Section 1.14, shall not apply unless specified by the architect or engineer.	Overlap exists. Needs determination.
2107.2 ACI 530/ASCE 5/TMS 402, Section 2.1.2, load combinations. Delete Section 2.1.2.1.	**2107.2.1 ACI 530/ASCE 5/TMS 402, Chapter 2. Reserved.**	No overlap exists. Move Florida specific language

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2107.3 ACI 530/ASCE 5/TMS 402, Section 2.1.3, design strength. Delete Sections 2.1.3.4 through 2.1.3.4.3.		forward
2107.4 ACI 530/ASCE 5/TMS 402, Section 2.1.6, columns. Add the following text to Section 2.1.6: 2.1.6.6 Light-frame construction. Masonry columns used only to support light-frame roofs of carports, porches, sheds or similar structures with a maximum area of 450 square feet (41.8 m_2) assigned to Seismic Design Category A, B or Care permitted to be designed and constructed as follows: 1. Concrete masonry materials shall be in accordance with Section 2103.1 of the *International Building Code*. Clay or shale masonry units shall be in accordance with Section 2103.2 of the *International Building Code*. 2. The nominal cross-sectional dimension of columns shall not be less than 8 inches (203 mm). 3. Columns shall be reinforced with not less than one No. 4 bar centered in each cell of the column. 4. Columns shall be grouted solid. 5. Columns shall not exceed 12 feet (3658 mm) in height. 6. Roofs shall be anchored to the columns. Such anchorage shall be capable of resisting the design loads specified in Chapter 16 of the *International Building Code*. 7. Where such columns are required to resist uplift loads, the columns shall be anchored to their footings with two No. 4 bars extending a minimum of 24 inches (610 mm) into the columns and bent horizontally a minimum of 15 inches (381 mm) in opposite directions into the footings. One of these bars is permitted to be the reinforcing bar specified in Item 3 above. The total weight of a column and its footing shall not be less than 1.5 times the design uplift load.	**2107.2.2 ACI 530/ASCE 5/TMS 402, Section 2.1.6.** Masonry columns used only to support light-frame roofs of carports, porches, sheds or similar structures with a maximum area of 450 square feet (41.8 m2) are permitted to be designed and constructed as follows: 1. Concrete masonry materials shall be in accordance with Section 2103.1. Clay or shale masonry units shall be in accordance with Section 2103.2. 2. The nominal cross-sectional dimension of columns shall not be less than 8 inches (203 mm). 3. Columns shall be reinforced with not less than one No. 4 bar centered in each cell of the column. 4. Columns shall be grouted solid. 5. Columns shall not exceed 12 feet (3658 mm) in height. 6. Roofs shall be anchored to the columns. Such anchorage shall be capable of resisting the design loads specified in Chapter 16. 7. Where such columns are required to resist uplift loads, the columns shall be anchored to their footings with two No. 4 bars extending a minimum of 24 inches (610 mm) into the columns and bent horizontally a minimum of 15 inches (381 mm) in opposite directions into the footings. One of these bars is permitted to be the reinforcing bar specified in Item 3 above. The total weight of a column and its footing shall not be less than 1.5 times the design uplift load.	Overlap exists. Needs determination.
2107.5 ACI 530/ASCE 5/TMS 402, Section 2.1.10.7.1.1, lap splices. Modify Section 2.1.10.7.1.1 as follows: 2.1.10.7.1.1 The minimum length of lap splices for reinforcing bars in tension or compression, l_d, shall be $l_d = 0.002 d_b f_s$ **(Equation 21-2)** For SI: $l_d = 0.29 d_b f_s$ but not less than 12 inches (305 mm). In no case shall the length of the lapped splice be less than 40 bar diameters. where:	**2107.2.3 ACI 530/ASCE 5/TMS 402, Section 2.1.10.7.1.1, lap splices.** The minimum length of lap splices for reinforcing bars in tension or compression, l_{ld}, shall be calculated by Equation 21-2, ~~but shall not be less than 15 inches (380 mm).~~ $\underline{l_{ld} = 0.002 d_b f_s}$ (Equation 21-2) $\underline{\text{For SI: } l_{ld} = 0.29 d_b f_s}$ but not less than 12 inches (305 mm). In no case shall the length of the lapped splice be less than 40 bar diameters. where:	Overlap exists. Needs determination. (See 2006 Supplement)

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d_b = Diameter of reinforcement, inches (mm). f_s = Computed stress in reinforcement due to design loads, psi (MPa). In regions of moment where the design tensile stresses in the reinforcement are greater than 80 percent of the allowable steel tension stress, F_s, the lap length of splices shall be increased not less than 50 percent of the minimum required length. Other equivalent means of stress transfer to accomplish the same 50 percent increase shall be permitted. Where epoxy coated bars are used, lap length shall be increased by 50 percent.	$\underline{d_b}$ = Diameter of reinforcement, inches (mm). $\underline{f_s}$ = Computed stress in reinforcement due to design loads, psi (MPa). ~~$l_{ld} = \frac{0.16 d_b^2 f_y \gamma}{K\sqrt{f'_m}}$~~ ~~For SI: $l_{ld} = \frac{1.95 d_b^2 f_y \gamma}{K\sqrt{f'_m}}$~~ ~~where:~~ ~~$d_b$ = Diameter of reinforcement, inches (mm).~~ ~~f_y = Specified yield stress of the reinforcement or the anchor bolt, psi (MPa).~~ ~~f'_m = Specified compressive strength of masonry at age of 28 days, psi (MPa).~~ ~~l_{ld} = Minimum lap splice length, inches (mm).~~ ~~K = The lesser of the masonry cover, clear spacing between adjacent reinforcement or five times d_b, inches~~ ~~λ = (mm).~~ ~~= 1.0 for No. 3 through No. 5 reinforcing bars. 1.4 for No. 6 and No. 7 reinforcing bars. 1.5 for No. 8 through No. 9 reinforcing bars.~~ In regions of moment where the design tensile stresses in the reinforcement are greater than 80 percent of the allowable steel tension stress F_s, the lap length of splices shall be increased not less than 50 percent of the minimum required length. Other equivalent means of stress transfer to accomplish the same 50 percent increase shall be permitted to be used.	
SECTION 2108 **STRENGTH DESIGN OF MASONRY**	**SECTION 2108** **STRENGTH DESIGN OF MASONRY**	
2108.1 General. The design of masonry structures using strength design shall comply with Section 2106 and the requirements of Chapters 1 and 3 of ACI 530/ASCE 5/TMS 402, except as modified by Sections 2108.2 through 2108.4. **Exception:** AAC masonry shall comply with the requirements of Chapter 1 and Appendix A of ACI 530/ASCE 5/TMS 402.	**2108.1 General.** The design of masonry structures using strength design shall comply with Section 2106 and the requirements of Chapters 1 and 3 of ACI 530/ASCE 5/TMS 402. The minimum nominal thickness for hollow clay masonry in accordance with Section 3.2.5.5 of ACI 530/ASCE 5/TMS 402	No overlap exists. Move Florida specific language forward

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	shall be 4 inches (102 mm). **Exception**: Where inspections are performed by a local building department in accordance with 105, the provisions of ACI 530/ASCE 5/TMS 402, Chapter 1, Section 1.14, shall not apply unless specified by the architect or engineer.	
2108.2 ACI 530/ASCE 5/TMS 402, Section 3.3.3.3 development. Add the following text to Section 3.3.3.3: The required development length of reinforcement shall be determined by Equation (3-15), but shall not be less than 12 inches (305 mm) and need not be greater than 72 d_b.	**2108.3 ACI 530/ASCE 5/TMS 402, Section 3.3.3.3.** Modify Section 3.3.3.3 as follows: The required development length of reinforcement shall be determined by Eq. (3-15), but shall not be less than 12 in. (305 mm) and need not be greater than 72 d_b.	(See 2006 Supplement) Overlap exists. Needs determination.
2109.1.1 Limitations. The use of empirical design of masonry shall be limited as follows: 1. Empirical design shall not be used for buildings assigned to Seismic Design Category D, E or F as specified in Section 1613, nor for the design of the seismic-force-resisting system for buildings assigned to Seismic Design Category B or C. 2. Empirical design shall not be used for masonry elements that are part of the lateral-force-resisting system where the basic wind speed exceeds 110 mph (79 m/s). 3. Empirical design shall not be used for interior masonry elements that are not part of the lateral force- resisting system in buildings other than enclosed buildings as defined in Chapter 6 of ASCE 7 in: 3.1. Buildings over 180 feet (55 100 mm) in height. 3.2. Buildings over 60 feet (18 400 mm) in height where the basic wind speed exceeds 90 mph (40 m/s). 3.3. Buildings over 35 feet (10 700 mm) in height where the basic wind speed exceeds 100 mph (45 m/s). 3.4. Where the basic wind speed exceeds 110 mph (79 m/s). 4. Empirical design shall not be used for exterior masonry elements that are not part of the lateral force-resisting system and that are more than 35 feet (10 700 mm) above ground: 4.1. Buildings over 180 feet (55 100 mm) in height. 4.2. Buildings over 60 feet (18 400 mm) in height where the basic wind speed exceeds 90 mph (40 m/s). 4.3. Buildings over 35 feet (10 700 mm) in height where the basic wind speed exceeds 100 mph (45 m/s).	**2109.1.1 Limitations.** Empirical masonry design shall not be utilized for any of the following conditions: 1. The design or construction of masonry structures located in areas where the basic wind speed exceeds 100 mph (177 km/hr). 2. Buildings more than 35 feet (10 668 mm) in height which have masonry wall lateral-force-resisting systems. In buildings that exceed one or more of the above limitations, masonry shall be designed in accordance with the engineered design provisions of Section 2107 or 2108, or the foundation wall provisions of Section 1805.5.	Overlap exists. Needs determination.

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5. Empirical design shall not be used for exterior masonry elements that are less than or equal to 35 feet (10 700 mm) above ground where the basic wind speed exceeds 110 mph (79 m/s). 6. Empirical design shall only be used when the resultant of gravity loads is within the center third of the wall thickness and within the central area bounded by lines at one-third of each cross-sectional dimension of foundation piers. 7. Empirical design shall not be used for AAC masonry. In buildings that exceed one or more of the above limitations, masonry shall be designed in accordance with the engineered design provisions of Section 2107 or 2108 or the foundation wall provisions of Section 1805.5.		
2110.1.1 Limitations. Solid or hollow approved glass block shall not be used in fire walls, party walls, fire barriers or fire partitions, or for load-bearing construction. Such blocks shall be erected with mortar and reinforcement in metal channel-type frames, structural frames, masonry or concrete recesses, embedded panel anchors as provided for both exterior and interior walls or other approved joint materials. Wood strip framing shall not be used in walls required to have a fire-resistance rating by other provisions of this code. **Exceptions:** 1. Glass-block assemblies having a fire protection rating of not less than 3/4 hour shall be permitted as opening protectives in accordance with Section 715 in fire barriers and fire partitions that have a required fire-resistance rating of 1 hour or less and do not enclose exit stairways or exit passageways. 2. Glass-block assemblies as permitted in Section 404.5, Exception 2.	**2110.1.1 Limitations.** Solid or hollow approved glass block shall not be used in fire walls, party walls, fire barriers or fire partitions, or for load-bearing construction. Such blocks shall be erected with mortar and reinforcement in metal channel-type frames, structural frames, masonry or concrete recesses, embedded panel anchors as provided for both exterior and interior walls or other approved joint materials. Wood strip framing shall not be used in walls required to have a fire-resistance rating by other provisions of this code. **Exceptions**: 1. Glass-block assemblies having a fire protection rating of not less than 3/4 hour shall be permitted as opening protectives in accordance with Section 715 in fire barriers and fire partitions that have a required fire-resistance rating of 1 hour or less and do not enclose exit stairways or exit passageways. 2. Glass-block assemblies as permitted in Section 404.5, Exception 2. 3. Fire tested and listed glass unit masonry shall be permitted for use in accordance with its listing.	Overlap exists. Needs determination.
Table 2111.1 Summary of Requirements for Masonry Fireplaces and Chimneys. Deleted	**Table 2111.1 Summary of Requirements for Masonry Fireplaces and Chimneys.**	No overlap exists. However, deleting the Table by the I-Code will result in deleting the table from the 2007 FBC.

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Figure 2111.1 Fireplace and Chimney Details. Deleted	**Figure 2111.1 Fireplace and Chimney Details.**	No overlap exists. However, deleting the Table by the I-Code will result in deleting the table from the 2007 FBC.
2111.3 Seismic reinforcing. Masonry or concrete fireplaces shall be constructed, anchored, supported and reinforced as required in this chapter. In Seismic Design Category D, masonry and concrete fireplaces shall be reinforced and anchored as detailed in Sections 2111.3.1, 2111.3.2, 2111.4 and 2111.4.1 for chimneys serving fireplaces. In Seismic Design Category A, B or C, reinforcement and seismic anchorage is not required. In Seismic Design Category E or F, masonry and concrete chimneys shall be reinforced in accordance with the requirements of Sections 2101 through 2108.	**Section 2111.3 Seismic reinforcing. Reserved**	No overlap exists. Move Florida specific language forward
2111.4 Seismic anchorage. Masonry and concrete chimneys in Seismic Design Category D shall be anchored at each floor, ceiling or roof line more than 6 feet (1829 mm) above grade, except where constructed completely within the exterior walls. Anchorage shall conform to the following requirements.	**Section 2111.4 Seismic anchorage. Reserved**	No overlap exists. Move Florida specific language forward

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NA	**TERMITE INSPECTION** **2114.1 Cleaning.** Cells and cavities in masonry units and air gaps between brick, stone or masonry veneers and the structure shall be cleaned of all nonpreservative treated or nonnaturally durable wood, or other cellulose-containing material prior to concrete placement. **Exception**: Inorganic material manufactured for closing cells in foundation concrete masonry unit construction or clean earth fill placed in concrete masonry unit voids below slab level before termite treatment is performed. **2114.2 Concrete bearing ledge.** Brick, stone or other veneer shall be supported by a concrete bearing ledge at least equal to the total thickness of the brick, stone or other veneer, which is poured integrally with the concrete foundation. No supplemental concrete foundation pours which will create a hidden cold joint shall be used without supplemental treatment in the foundation unless there is an approved physical barrier. An approved physical barrier shall also be installed from below the wall sill plate or first block course horizontally to embed in a mortar joint. If masonry veneer extends below grade, a termite protective treatment must be applied to the cavity created between the veneer and the foundation, in lieu of a physical barrier. **Exception**: Veneer supported by a shelf, angle or lintel secured to the foundation sidewall in accordance with ACI 530/ASCE 5/TMS 402, provided at least a 6-inch (152 mm) clear inspection space of the foundation sidewall exterior exist between the veneer and the top of any soil, sod, mulch or other organic landscaping component, deck, apron, porch, walk or any other work immediately adjacent to or adjoining the structure.	No overlap exists. Move Florida specific language forward
NA	**SECTION 2115** **SPECIAL WIND PROVISIONS FOR MASONRY** **2115.1 Gable endwalls.** **2115.1.1 General.** Gable endwalls shall be structurally	No overlap exists. Move Florida specific language forward

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	continuous between points of lateral support. **2115.1.2 Cathedral endwalls.** Gable endwalls adjacent to cathedral ceilings shall be structurally continuous from the uppermost floor to the ceiling diaphragm or to the roof diaphragm.	
NA	**SECTIONS 2116 – 2117 RESERVED** **SECTION 2118-2122 HIGH-VELOCITY HURRICANE ZONES—DESIGN**	No overlap exists. Move Florida specific language forward
CHAPTER 22	**STEEL**	
SECTION 2201 **GENERAL**	**SECTION 2201** **GENERAL**	
2201.1 Scope. The provisions of this chapter govern the quality, design, fabrication and erection of steel used structurally in buildings or structures.	**2201.1 Scope.** The provisions of this chapter govern the quality, design, fabrication and erection of steel used structurally in buildings or structures. **Exception**: Buildings and structures located within the high-velocity hurricane zone shall comply with the provision of 2214 through 2224.	No overlap exists. Move Florida specific language forward
2204.1Welding. The details of design, workmanship and technique for welding, inspection of welding and qualification of welding operators shall conform to the requirements of the specifications listed in Sections 2205, 2206, 2207, 2209 and 2210. Special inspection of welding shall be provided where required by Section 1704.	**2204.1 Welding.** The details of design, workmanship and technique for welding, inspection of welding and qualification of welding operators shall conform to the requirements of the specifications listed in Sections 2205, 2206, 2207, 2209 and 2210.	No overlap exists. Move Florida specific language forward
2204.2 Bolting. The design, installation and inspection of bolts shall be in accordance with the requirements of the specifications listed in Sections 2205, 2206, 2209 and 2210. Special inspection of the installation of high-strength bolts shall be provided where required by Section 1704.	**2204.2 Bolting.** The design, installation and inspection of bolts shall be in accordance with the requirements of the specifications listed in Sections 2205, 2206, 2209 and 2210.	No overlap exists. Move Florida specific language forward
2205.2 Seismic requirements for steel structures. The design of structural steel structures to resist seismic forces shall be in accordance with the provisions of Section 2205.2.1 or 2205.2.2 for the appropriate seismic design category.	**2205.2 Seismic requirements for steel structures. Reserved.**	No overlap exists. Move Florida specific language forward
2205.3 Seismic requirements for composite construction. The design, construction and quality of composite steel and concrete components that resist seismic forces shall conform	**2205.3 Seismic requirements for composite construction. Reserved.**	No overlap exists. Move Florida specific language forward

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to the requirements of the AISC 360 and ACI 318. An *R* factor as set forth in Section 12.2.1 of ASCE 7 for the appropriate composite steel and concrete system is permitted where the structure is designed and detailed in accordance with the provisions of AISC 341, Part II. In Seismic Design Category B or above, the design of such systems shall conform to the requirements of AISC 341, Part II.		
2206 Steel Joists **2206.1 General.** The design, manufacture and use of open web steel joists and joist girders shall be in accordance with one of the following Steel Joist Institute (SJI) specifications: 1. SJI K-1.1 2. SJI LH/DLH-1.1 3. SJI JG-1.1 Where required, the seismic design of buildings shall be in accordance with the additional provisions of Section 2205.2 or 2210.5. **2206.2 Design…..** **2206.3 Calculations…..** **2206.4 Steel Joist Drawings….** **2206.5 Certification….**	**2206 Steel Joists** **2206.1 General.** The design, manufacturing and use of open web steel joists and joist girders shall be in accordance with one of the following Steel Joist Institute specifications: 1. Standard Specifications for Open Web Steel Joists, K Series. 2. Standard Specifications for Longspan Steel Joists, LH Series and Deep Longspan Steel Joists, DLH Series. 3. Standard Specifications for Joist Girders.	No overlap exists. Move Florida specific language forward (2006 ICC has expanded language)
2208.1 Storage racks. The design, testing and utilization of industrial steel storage racks shall be in accordance with the *RMI Specification for the Design, Testing and Utilization of Industrial Steel Storage Racks*. Racks in the scope of this specification include industrial pallet racks, movable shelf racks and stacker racks and does not apply to other types of racks, such as drive-in and drive-through racks, cantilever racks, portable racks or rack buildings. Where required, the seismic design of storage racks shall be in accordance with the provisions of Section 15.5.3 of ASCE 7.	**2208.1 Storage racks.** The design, testing and utilization of industrial steel storage racks shall be in accordance with the RMI Specification for the Design, Testing and Utilization of Industrial Steel Storage Racks. Racks in the scope of this specification include industrial pallet racks, movable shelf racks and stacker racks, and does not apply to other types of racks, such as drive-in and drive-through racks, cantilever racks, portable racks or rack buildings.	No overlap exists. Move Florida specific language forward
NA	**SECTION 2211 COLD-FORMED STEEL** **LIGHT-FRAMED SHEAR WALLS**	

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Section was deleted	**2211.2 Type I shear walls.** The design of Type I shear walls, of cold-formed steel light-framed construction, to resist wind loads, shall be in accordance the requirements of this section. 1. The nominal shear value for Type I shear walls, as shown in Tables 2211.2(1) and 2211.2(2) for wind load, is permitted to establish allowable shear values or design shear values.	No overlap exists. However, deleting the Table by the I-Code will result in deleting the table from the 2007 FBC.
Section was deleted.	**2211.2.2 Limitations for systems.** The lateral-resistant systems listed in Tables 2211.2(1) and 2211.2(2): 5. The height-to-width shear wall aspect ratio (h/w) of wall systems shall not exceed the values in Tables 2211.2(1) and 2211.2(2). Where the limiting ratio of h/w is greater than 2:1, the shear values shall be multiplied by 2w/h. 8. The design shear values for shear panels with different nominal shear values applied to the same side of a wall are not cumulative except as permitted in Tables 2211.2(1) and 2211.2(2). For walls with material applied to both faces of the same wall, the design shear value of material of the same capacity is cumulative. Where the material nominal shear values are not equal, the design shear value shall be either two times the design shear value of the material with the smaller values or shall be taken as the value of the stronger side, whichever is greater. Summing shear values of dissimilar material applied to opposite faces or to the same wall line is not allowed unless permitted by Table 2211.2(1).	No overlap exists. However, deleting the Table by the I-Code will result in deleting the table from the 2007 FBC.
Section was deleted.	**2211.2.2.2 Wood structural panel sheathing.** Cold-formed steel framed wall systems, sheathed with wood structural panels, are permitted to resist horizontal forces produced by wind loads subject to the following: 1. Nominal shear values, used to establish the allowable shear value or design shear value, are given in Tables 2211.2(1) for wind loads. 6. Increases of the nominal loads shown in Table 2211.2(1) shall not be permitted for duration of load as permitted in Chapter 23.	No overlap exists. However, deleting the Table by the I-Code will result in deleting the table from the 2007 FBC.

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Section was deleted	**2211.3.1 Limitations.** The following limitations shall apply to the use of Type II shear walls: 1. A Type II shear wall segment, meeting the minimum aspect ratio (h/w) of Section 2211.3.2, Item 3, shall be located at each end of a Type II shear wall. Openings shall be permitted to occur beyond the ends of the Type II shear wall; however, the width of such openings shall not be included in the width of the perforated shear wall.	No overlap exists. However, deleting the Table by the I-Code will result in deleting the table from the 2007 FBC.
Section deleted	**2211.3.2 Type II shear wall resistance.** The Type II shear wall resistance shall be equal to the adjusted shear resistance multiplied by the sum of the widths (SLi) of the Type II shear wall segments and shall be calculated in accordance with the following: 3. The unadjusted shear resistance shall be the design shear values calculated in accordance with Section 2211.2.1 based upon the values in Table 2211.2(1). The aspect ratio of all Type II shear wall segments used in calculations shall not exceed 2:1. **Exception**: Where permitted by Table 2211.2(1), the aspect ratio (h/w) of Type II wall segments greater than 2:1, but in no case greater than 4:1, is permitted to be included in the calculation of the unadjusted shear resistance for the wall, provided the values are multiplied by 2w/h.	No overlap exists. However, deleting the Table by the I-Code will result in deleting the table from the 2007 FBC.
Section deleted	**2211.3.3.2 Uplift anchorage at Type II shear wall ends.** Anchorage for uplift forces due to overturning shall be provided at each end of the Type II shear wall.	No overlap exists. However, deleting the Table by the I-Code will result in deleting the table from the 2007 FBC.
	SECTION 2212 **SPECIAL WIND PROVISIONS FOR STEEL STUD WALL SYSTEMS**	

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
NA	**SECTION 2212** **SPECIAL WIND PROVISIONS FOR STEEL STUD WALL SYSTEMS** **2212.1 Gable end walls.** Gable endwalls shall be structurally continuous between points of lateral support. **2212.2 Catherdral end walls.** Gable endwalls adjacent to cathederal ceilings shall be continuous from the uppermost floor to ceilings shall be continuous from the uppermost floor to ceiling diaphragm or to the roof diaphragm.	No over lap exists. Move Florida specific language forward.
NA	**SECTION 2213 Reserved.** **SECTION 2214 –SECTION 2224** HIGH-VELOCITY HURRICANE ZONES—	No over lap exists. Move Florida specific language forward.
CHAPTER 23	**WOOD**	
	SECTION 2301 **GENERAL**	
2303.1.1 Sawn lumber. Sawn lumber used for load-supporting purposes, including end-jointed or edge-glued lumber, machine stress-rated or machine-evaluated lumber, shall be identified by the grade mark of a lumber grading or inspection agency that has been approved by an accreditation body that complies with DOC PS 20 or equivalent. Grading practices and identification shall comply with rules published by an agency approved in accordance with the procedures of DOC PS 20 or equivalent procedures. In lieu of a grade mark on the material, a certificate of inspection as to species and grade issued by a lumber grading or inspection agency meeting the requirements of this section is permitted to be accepted for precut, remanufactured or rough-sawn lumber and for sizes larger than 3 inches (76 mm) nominal thickness. Approved end-jointed lumber is permitted to be used interchangeably with solid-sawn members of the same species and grade.	**2301.1 Scope.** The provisions of this chapter shall govern the materials, design, construction and quality of wood members and their fasteners. **Exception**: Buildings and structures located within the high-velocity hurricane zone shall comply with the provisions of Sections 2314 through 2330.	No over lap exists. Move Florida specific language forward.
2303.2.2.2 Lumber. For each species of wood that is treated, the effects of the treatment, the method of redrying after treatment and exposure to high temperatures and high	**2303.2.2.2 Lumber.** For each species of wood treated, the effect of the treatment and the method of redrying after treatment and exposure to high temperatures and high	Overlap exists. Needs determination. (See 2006 Supplement)

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
humidities on the allowable design properties of fire-retardant-treated lumber shall be determined in accordance with ASTM D 5664. The test data developed by ASTM D 5664 shall be used to develop modification factors for use at or near room temperature and at elevated temperatures and humidity in accordance with ASTM D 6841. Each manufacturer shall publish the modification factors for service at temperatures of not less than 80°F (27°C) and for roof framing. The roof framing modification factors shall take into consideration the climatological location.	humidities on the allowable design properties of fire-retardant-treated lumber shall be determined in accordance with ASTM D 5664. The test data developed by ASTM D 5664 shall be used to develop modification factors for use at or near room temperature and at elevated temperatures and humidity in accordance with ~~an approved method of investigation~~ ASTM D 6841. Each manufacturer shall publish the modification factors for service at temperatures of not less than 80°F (26.7°C) and for roof framing. The roof framing modification factors shall take into consideration the climatological location.	
NA	**2304.3.4 Gable endwalls.** **2304.3.4.1 General.** Gable endwalls shall be structurally continuous between points of lateral support. **2304.3.4.2 Cathedral endwalls.** Gable endwalls adjacent to cathedral ceilings shall be structurally continuous from the uppermost floor to the ceiling diaphragm or to the roof diaphragm. **2304.3.4.3 Full height studs**. Full height studs may be sized using the bracing at a ceiling diaphragm for determining stud length requirements.	No over lap exists. Move Florida specific language forward.
Table 2304.7(3)	**TABLE 2304.7(3)** ALLOWABLE SPANS AND LOADS FOR WOOD STRUCTURAL PANEL SHEATHING AND SINGLE FLOOR GRADES CONTINUOUS OVER TWO OR MORE SPANS WITH LONG PANEL DIMENSION PERPENDICULAR TO SUPPORTSa,b	No overlap exists. Move Florida specific language forward
Table 2304.7(5)	**TABLE 2304.7(5)** ALLOWABLE LOAD (PSF) FOR WOOD STRUCTURAL PANEL ROOF SHEATHING CONTINUOUS OVER TWO OR MORE SPANS AND STRENGTH AXIS PARALLEL TO SUPPORTS (plywood structural panels are 5-ply, 5-layer unless otherwise noted)a,b	No over lap exists. Move Florida specific language forward.

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
TABLE 2304.9.1 **FASTENING SCHEDULE**	**TABLE 2304.9.1** FASTENING SCHEDULE	Overlap exists. Needs determination. (see 2006 supplement)
2304.11.2.2 Wood supported by exterior foundation walls. Wood framing members, including wood sheathing, that rest on exterior foundation walls and are less than 8 inches (203 mm) from exposed earth shall be of naturally durable or preservative-treated wood.	**2304.11.2.2 Framing.** Wood framing members, including wood sheathing, which rest on exterior foundation walls and are less than 8 inches (203 mm) from exposed earth shall be of naturally durable or preservative-treated wood. Wood framing members and furring strips attached directly to masonry or concrete walls shall be of approved naturally durable or preservative-treated wood.	Overlap exists. Needs determination.
NA	**2304.11.4.3** Decks, fences, patios, planters, or other wooden building components that directly abut the sidewall of the foundation or structure shall be constructed so as to provide: 1.Eighteen-inch (457 mm) clearance beneath or, 2.Six-inch (152 mm) clearance between the top of the component and the exterior wall covering or, 3. have components that are easily removable by screws or hinges to allow access for inspection of the foundation sidewall and treatment for termites.	No overlap exists. Move Florida specific language forward
2304.11.6 Termite protection. In geographical areas where hazard of termite damage is known to be very heavy, wood floor framing shall be of naturally durable species (termite resistant) or preservative treated in accordance with AWPA U1 for the species, product preservative and end use or provided with approved methods of termite protection.	**2304.11.6 Termite protection.** Termite protection shall be provided by floor framing of naturally durable or preservative-treated wood, soil treatment or other approved methods of termite protection.	Overlap exists. Needs determination.
NA	**2304.11.10 Foam-plastic insulation.** **2304.11.10.1** The provisions of Section 2603. 9 shall apply to the installation of foam plastic insulation in close proximity to the ground. **Exception**: Materials which are of naturally durable wood or are pressure treated for ground contact, and which are installed with at least 6 inches (152 mm) clear space from the structure to allow for inspection and treatment for termites.	No overlap exists. Move Florida specific language forward

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
	In order to reduce chances of termite infestation, no wood, vegetation, stumps, dead roots, cardboard, trash, or other cellulose-containing material shall be buried on the building lot within 15 feet (4.6 m) of any building or the position of any building proposed to be built.	
NA	**2304.13 Preparation of building site and removal of debris.** **2304.13.1** All building sites shall be graded to provide drainage under all portions of the building not occupied by basements. **2304.13.2** The foundation and the area encompassed within 1 foot (305 mm) therein shall have all vegetation, stumps, dead roots, cardboard, trash, and foreign material removed and the fill material shall be free of vegetation and foreign material. The fill shall be compacted to assure adequate support of the foundation. **2304.13.3** After all work is completed, loose wood and debris shall be completely removed from under the building and within 1 foot (305 mm) thereof. All wood forms and supports shall be completely removed. This includes, but is not limited to: wooden grade stakes, forms, contraction spacers, tub trap boxes, plumbing supports, bracing, shoring, forms, or other cellulose-containing material placed in any location where such materials are not clearly visible and readily removable prior to completion of the work. Wood shall not be stored in contact with the ground under any building.	No overlap exists. Move Florida specific language forward
2305.1 General. Structures using wood shear walls and diaphragms to resist wind, seismic and other lateral loads shall be designed and constructed in accordance with the provisions of this section. Alternatively, compliance with the AF&PA SDPWS shall be permitted subject to the limitations therein and the limitations of this code.	**2305.1 General.** Structures using wood shear walls and diaphragms to resist wind, and other lateral loads shall be designed and constructed in accordance with the provisions of this section.	No overlap exists. Move Florida specific language forward
2305.1.4 Shear panel connections. Positive connections and anchorages capable of resisting the design forces shall be provided between the shear panel and the attached components.	**2305.1.4 Shear panel connections.** Positive connections and anchorages, capable of resisting the design forces, shall be provided between the shear panel and the attached components.	No overlap exists. Move Florida specific language forward

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
In Seismic Design Category D, E or F, the capacity of toenail connections shall not be used when calculating lateral load resistance to transfer lateral earthquake forces in excess of 150 pounds per foot (2189 N/m) from diaphragms to shear walls, drag struts (collectors) or other elements, or from shear walls to other elements.		
2305.1.5 Wood members resisting horizontal seismic forces contributed by masonry and concrete walls. Wood shear walls, diaphragms, horizontal trusses and other members shall not be used to resist horizontal seismic forces contributed by masonry or concrete walls in structures over one story in height. **Exceptions:** 1. Wood floor and roof members are permitted to be used in horizontal trusses and diaphragms to resist horizontal seismic forces contributed by masonry or concrete walls, provided such forces do not result in torsional force distribution through the truss or diaphragm. 2. Wood structural panel sheathed shear walls are permitted to be used to provide resistance to seismic forces contributed by masonry or concrete walls in two-story structures of masonry or concrete walls, provided the following requirements are met: 2.1. Story-to-story wall heights shall not exceed 12 feet (3658 mm). 2.2. Diaphragms shall not be designed to transmit lateral forces by rotation and shall not cantilever past the outermost supporting shear wall. 2.3. Combined deflections of diaphragms and shear walls shall not permit story drift of supported masonry or concrete walls to exceed the limit of Section 12.12.1 in ASCE 7. 2.4. Wood structural panel sheathing in diaphragms shall have unsupported edges blocked. Wood structural panel sheathing for both stories of shear walls shall have unsupported edges blocked and, for the lower story, shall have a minimum thickness of 15/32 inch (11.9 mm). 2.5. There shall be no out-of-plane horizontal offsets between	**2305.1.5 Wood members resisting horizontal seismic forces contributed by masonry and concrete walls. Reserved.**	No overlap exists. Move Florida specific language forward

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
the first and second stories of wood structural panel shear walls.		
2305.2.4.1 Seismic Design Category F. Structures assigned to Seismic Design Category F shall conform to the additional requirements of this section. Wood structural panel sheathing used for diaphragms and shear walls that are part of the seismic-force-resisting system shall be applied directly to the framing members. **Exception:** Wood structural panel sheathing in a diaphragm is permitted to be fastened over solid lumber planking or laminated decking, provided the panel joints and lumber planking or laminated decking joints do not coincide.	**2305.2.4.1 Seismic Design Category F. Reserved.**	No overlap exists. Move Florida specific language forward
2305.2.5 Rigid diaphragms. Design of structures with rigid diaphragms shall conform to the structure configuration requirements of Section 12.3.2 of ASCE 7 and the horizontal shear distribution requirements of Section 12.8.4 of ASCE 7.	**2305.2.5 Rigid diaphragms.** Design of structures with rigid diaphragms shall conform to the structure configuration requirements of Section 9.5.2.3 of ASCE 7 and the horizontal shear distribution requirements of Section 9.5.5.5 of ASCE 7. Open front structures with rigid wood diaphragms resulting in torsional force distribution are permitted provided the length, *l*, of the diaphragm normal to the open side does not exceed 25 feet (7620 mm), the diaphragm sheathing conforms to Section 2305.2.4, and the *l/w* ratio [as shown in Figure 2305.2.5(1)] is less than 1.0 for one-story structures or 0.67 for structures over one story in height. **Exception:** Where calculations show that diaphragm deflections can be tolerated, the length, *l*, normal to the open end is permitted to be increased to a *l/w* ratio not greater than 1.5 where sheathed in compliance with Section 2305.2.4 or to 1.0 where sheathed in compliance with Section 2306.3.4 or 2306.3.5. Rigid wood diaphragms are permitted to cantilever past the outermost supporting shear wall (or other vertical resisting element) a length, *l*, of not more than 25 feet (7620 mm) or two-thirds of the diaphragm width, *w*, whichever is the smaller. Figure 2305.2.5(2) illustrates the dimensions of *l* and *w* for a cantilevered diaphragm.	Overlap exists. Needs determination.

<table>
<tr><th>2006 IBC</th><th>Florida Specific Requirements, 2004 FBC</th><th>TAC Action</th></tr>
<tr><td>2305.3 Design of wood shear walls.
2305.3.1 General. Wood shear walls are permitted to resist horizontal forces in vertical distributing or resisting elements, provided the deflection in the plane of the shearwall, as determined by calculations, tests or analogies drawn therefrom, does not exceed the more restrictive of the permissible deflection of attached distributing or resisting elements or the drift limits of Section 12.12.1 of ASCE 7. Shear wall sheathing other than wood structural panels shall not be permitted in Seismic Design Category E or F (see Section 1613).</td><td>2305.3 Design of wood shear walls.
2305.3.1 General. Wood shear walls are permitted to resist horizontal forces in vertical distributing or resisting elements, provided the deflection in the plane of the shear wall, as determined by calculations, tests or analogies drawn therefrom, does not exceed the more restrictive of the permissible deflection of attached distributing or resisting elements.</td><td>Overlap exists. Needs determination.</td></tr>
<tr><td>TABLE 2305.3.4
MAXIMUM SHEAR WALL DIMENSION RATIOS
<table><tr><th>TYPE</th><th>MAXIMUM HEIGHT-WIDTH RATIO</th></tr><tr><td>Wood structural panels or particleboard, nailed edges</td><td>For other than seismic: 3½:1
For seismic: 2:1a</td></tr><tr><td>Diagonal sheathing, single</td><td>2:1</td></tr><tr><td>Fiberboard</td><td>1½:1</td></tr><tr><td>Gypsum board, gypsum lath, cement plaster</td><td>1½:1b</td></tr></table>a. For design to resist seismic forces, shear wall height-width ratios greater than 2:1, but not exceeding 3½:1, are permitted provided the allowable shear values in Table 2306.4.1 are multiplied by 2w/h.
b. Ratio shown is for unblocked construction. Height-to-width ratio is permitted to be 2:1 where the wall is installed as blocked construction in accordance with Section 2306.4.5.1.2.</td><td>TABLE 2305.3.3
MAXIMUM SHEAR WALL ASPECT RATIOS
TYPEMAXIMUM HEIGHT-WIDTH RATIO
Wood structural panels or particleboard, nailed edges 31/2:1
Diagonal sheathing, single 2:1
Fiberboard 11/2:1
Gypsum board, gypsum lath, cement plaster 11/2:1b
a. Reserved.
b. Ratio shown is for unblocked construction. Aspect ratio is permitted to be 2:1 where the wall is installed as blocked construction in accordance with Section 2306.4.5.1.2.</td><td>No overlap exists. Move Florida specific language forward</td></tr>
<tr><td>2305.3.11 Sill plate size and anchorage in Seismic Design Category D, E or F. Anchor bolts for shear walls shall include steel plate washers, a minimum of 0.229 inch by 3 inches by 3 inches (5.82 mm by 76 mm by 76 mm) in size, between the sill plate and nut. The hole in the platewasher is permitted to be diagonally slotted with a width of up to 3/16 inch (4.76 mm) larger than the bolt diameter and a slot length not to exceed 1¾ inches (44 mm), provided a standard cut washer is placed between the plate washer and the nut. Sill plates resisting a design load greater than 490 plf (7154 N/m) using load and resistance factor design or 350</td><td>2305.3.11 Sill plate size and anchorage in Seismic Design Category D, E or F. Reserved.</td><td>No overlap exists. Move Florida specific language forward</td></tr>
</table>

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
plf (5110 N/m) using allowable stress design shall not be less than a 3-inch (76 mm) nominal member. Where a single 3- inch (76 mm) nominal sill plate is used, 2- 20d box end nails shall be substituted for 2-16d common end nails found in line 8 of Table 2304.9.1. **Exception:** In shear walls where the design load is greater than 490 plf (7151 N/m) but less than 840 plf (12 264 N/m) using load and resistance factor design or greater than 350 plf (5110 N/m) but less than 600 plf (8760 N/m) using allowable stress design, the sill plate is permitted to be a 2-inch (51 mm) nominal member if the sill plate is anchored by two times the number of bolts required by design and 0.229-inch by 3-inch by 3-inch (5.82mmby 76mmby 76mm) platewashers are used.		
2306.4.3 Particleboard shear walls. The design shear capacity of particleboard shear walls shall be in accordance with Table 2306.4.3. Shear panels shall be constructed with particleboard sheets not less than 4 feet by 8 feet (1219 mm by 2438 mm), except at boundaries and changes in framing. Particleboard panels shall be designed to resist shear only, and chords, collector members and boundary elements shall be connected at all corners. Panel edges shall be backed with 2-inch (51 mm) nominal or wider framing. Sheets are permitted to be installed either horizontally or vertically. For 3/8-inch (9.5 mm) particleboard sheets installed with the long dimension parallel to the studs spaced 24 inches (610 mm) o.c, nails shall be spaced at 6 inches (152 mm) o.c. along intermediate framing members. For all other conditions, nails of the same size shall be spaced at 12 inches (305mm)o.c. along intermediate framing members. Particleboard panels less than 12 inches (305 mm) wide shall be blocked. Particleboard shall not be used to resist seismic forces in structures in Seismic Design Category D, E or F.	**2306.4.3 Particleboard shear walls.** The design shear capacity of particleboard shear walls shall be in accordance with Table 2306.4.3. Shear panels shall be constructed with particleboard sheets not less than 4 feet by 8 feet (1219 mm by 2438 mm), except at boundaries and changes in framing. Particleboard panels shall be designed to resist shear only, and chords, collector members and boundary elements shall be connected at all corners. Panel edges shall be backed with 2-inch (51 mm) nominal or wider framing. Sheets are permitted to be installed either horizontally or vertically. For 3/8-inch (9.5 mm) particleboard sheets installed with the long dimension parallel to the studs spaced 24 inches (610 mm) o.c, nails shall be spaced at 6 inches (152 mm) o.c. along intermediate framing members. For all other conditions, nails of the same size shall be spaced at 12 inches (305 mm) o.c. along intermediate framing members. Particleboard panels less than 12 inches (305 mm) wide shall be blocked.	No overlap exists. Move Florida specific language forward
2306.4.4 Fiberboard shear walls. The design shear capacity of fiberboard shear walls shall be in accordance with Table 2306.4.4. The fiberboard sheathing shall be applied vertically or horizontally to wood studs not less than 2 inch (51 mm) in nominal thickness spaced 16 inches (406 mm) o.c. Blocking not less than 2 inch (51 mm) nominal in thickness shall be	**2306.4.4 Fiberboard shear walls**. The design shear capacity of fiberboard shear walls shall be in accordance with Table 2308.9.3(4). The fiberboard sheathing shall be applied vertically or horizontally to wood studs not less than 2 inch (51 mm) nominal thickness spaced 16 inches (406 mm) o.c. Blocking not less than 2 inch (51 mm) nominal in thickness	No overlap exists. Move Florida specific language forward

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
provided at horizontal joints. Fiberboard shall not be used to resist seismic forces in structures in Seismic Design Category D, E or F.	shall be provided at horizontal joints.	
2306.4.5 Shear walls sheathed with other materials. Shear capacities for walls sheathed with lath, plaster or gypsum board shall be in accordance with Table 2306.4.5. Shear walls sheathed with lath, plaster or gypsum board shall be constructed in accordance with Chapter 25 and Section 2306.4.5.1. Walls resisting seismic loads shall be subject to the limitations in Section 12.2.1 of ASCE 7.	**2306.4.5 Shear walls sheathed with other materials.** Shear capacities for walls sheathed with lath and plaster, and gypsum board shall be in accordance with Table 2306.4.5. Shear walls sheathed with lath, plaster and gypsum board shall be constructed in accordance with Chapter 25 and Section 2306.4.5.1.	No overlap exists. Move Florida specific language forward
Table 2306.4.5 ALLOWABLE SHEAR FOR WIND OR SEISMIC FORCES FOR SHEAR WALLS OF LATH AND PLASTER OR GYPSUM BOARD WOOD FRAMED WALL ASSEMBLIES	**Table 2306.4.5 ALLOWABLE SHEAR FOR WIND FORCES FOR SHEAR WALLS OF LATH AND PLASTER OR GYPSUM BOARD WOOD FRAMED WALL ASSEMBLIES**	Overlap exists. Needs determination
2308.2 Limitations. Buildings are permitted to be constructed in accordance with the provisions of conventional light-frame construction, subject to the following limitations, and to further limitations of Sections 2308.11 and 2308.12. 1. Buildings shall be limited to a maximum of three stories above grade. For the purposes of this section, for buildings in Seismic Design Category D or E as determined in Section 1613, cripple stud walls shall be considered to be a story. **Exception:** Solid blocked cripplewalls not exceeding 14 inches (356 mm) in height need not be considered a story. 2. Bearing wall floor-to-floor heights shall not exceed a stud height of 10 feet (3048 mm) plus a height of floor framing not to exceed 16 inches (406 mm). 3. Loads as determined in Chapter 16 shall not exceed the following: 3.1. Average dead loads shall not exceed 15 psf (718 N/m2) for combined roof and ceiling, exterior walls, floors and partitions. **Exceptions:** 1. Subject to the limitations of Sections 2308.11.2 and 2308.12.2, stone or masonry veneer up to the lesser of 5 inches (127 mm) thick or 50 psf (2395 N/m2) and installed in accordance with Chapter 14 is permitted to a height of 30 feet (9144 mm) above a noncombustible	**2308.2 Limitations.** Buildings are permitted to be constructed in accordance with the provisions of conventional light-frame construction, subject to the following limitations, and to further limitations of Sections 2308.11 and 2308.12. 1.Buildings shall be limited to a maximum of three stories above grade. 2.Bearing wall floor-to-floor heights shall not exceed 10 feet (3048 mm). 3.Loads as determined in Chapter 16 shall not exceed the following: 3.1.Average dead loads shall not exceed 15 psf (718 N/m2) for roofs and exterior walls, floors and partitions. 3.2.Live loads shall not exceed 40 psf (1916 N/m2) for floors. 4.Wind speeds shall not exceed 100 miles per hour (mph) (44 m/s) (3-second gust). 5.Roof trusses and rafters shall not span more than 40 feet (12 192 mm) between points of vertical support.	Overlap exists. Needs determination.

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
foundation, with an additional 8 feet (2438 mm) permitted for gable ends. 2. Concrete or masonry fireplaces, heaters and chimneys shall be permitted in accordance with the provisions of this code. 3.2. Live loads shall not exceed 40 psf (1916 N/m2) for floors. 3.3. Ground snow loads shall not exceed 50 psf (2395 N/m2).		
2308.2.2 Buildings in Seismic Design Category B, C,D or E. Buildings of conventional light-frame construction in Seismic Design Category B or C, as determined in Section 1613, shall comply with the additional requirements in Section 2308.11. Buildings of conventional light-frame construction in Seismic Design Category D or E, as determined in Section 1613, shall comply with the additional requirements in Section 2308.12.	**2308.2.2 Buildings in Seismic Design Category B, C,D or E. Reserved.**	No overlap exists. Move Florida specific language forward
2308.9.4.1 Bracing. For the purposes of this section, cripple walls having a stud height exceeding 14 inches (356 mm) shall be considered a story and shall be braced in accordance with Table 2308.9.3(1) for Seismic Design Category A, B or C. See Section 2308.12.4 for Seismic Design Category D or E.	**2308.9.4.1 Bracing**. For the purposes of this section, cripple walls having a stud height exceeding 14 inches (356 mm) shall be considered a story and shall be braced in accordance with Table 2308.9.3(1).	No overlap exists. Move Florida specific language forward
Table 2308.9.3(1) Braced Wall Panels	**Table 2308.9.3(1) Braced Wall Panels**	Overlap exists. Needs determination.
Table 2308.9.5 Header and Girder Spans for Exterior Bearing Walls… **Table 2308.4.1 Rafter Tie Connections.**	**Table 2308.9.5 Header and Girder Spans for Exterior Bearing Walls…** **Table 2308.4.1 Rafter Tie Connections.**	No overlap exists. Move Florida specific language forward
Table 2308.10.3(3) **Table 2308.10.3(4)** **Table 2308.10.3(5)** **Table 2308.10.3(6)**	**Table 2308.10.3(3) Reserved.** **Table 2308.10.3(4) Reserved.** **Table 2308.10.3(5) Reserved.** **Table 2308.10.3(6) Reserved.**	No overlap exists. Move Florida specific language forward
2308.11 Additional requirements for conventional construction in Seismic Design Category B or C.	**2308.11 Additional requirements for conventional construction in Seismic Design Category B or C. Reserved.**	No overlap exists. Move Florida specific language

2006 IBC	Florida Specific Requirements, 2004 FBC	TAC Action
2308.11.1 Number of stories. **2308.11.2 Concrete or masonry.** **2308.11.3 Framing and connection details.** **2308.11.3.1 Anchorage.** **2308.12 Additional requirements for conventional construction in Seismic Design Category D or E.**	**2308.11.1 Number of stories. Reserved.** **2308.11.2 Concrete or masonry. Reserved.** **2308.11.3 Framing and connection details. Reserved.** **2308.11.3.1 Anchorage. Reserved.** **2308.12 Additional requirements for conventional construction in Seismic Design Category D or E. Reserved.**	forward
NA	**SECTION 2314 -2330 (see 2006 Supplement)** HIGH-VELOCITY HURRICANE ZONES — GENERAL	No overlap exists. Move Florida specific language forward
CHAPTER 24	**GLASS AND GLAZING**	
SECTION 2401 **GENERAL**	**SECTION 2401** **GENERAL**	
2401.1 Scope. The provisions of this chapter shall govern the materials, design, construction and quality of glass, light transmitting ceramic and light-transmitting plastic panels for exterior and interior use in both vertical and sloped applications in buildings and structures.	**2401.1 Scope.** The provisions of this chapter shall govern the materials, design, construction and quality of glass, light-transmitting ceramic and light-transmitting plastic panels for exterior and interior use in both vertical and sloped applications in buildings and structures. **Exception:** Buildings and structures located within the high-velocity hurricane zone shall comply with the provisions of 2410 through 2415.	No overlap exists. Move Florida specific language forward
2403.1 Identification. Each pane shall bear the manufacturer's mark designating the type and thickness of the glass or glazing material. The identification shall not be omitted unless approved and an affidavit is furnished by the glazing contractor certifying that each light is glazed in accordance with approved construction documents that comply with the provisions of this chapter. Safety glazing shall be identified in accordance with Section 2406.2. Each pane of tempered glass, except tempered spandrel glass, shall be permanently identified by the manufacturer. The identification mark shall be acid etched, sand blasted, ceramic fired, laser etched, embossed or of a type that, once applied, cannot be removed without being destroyed.	**2403.1 Identification.** Each pane shall bear the manufacturer's label designating the type and thickness of the glass or glazing material. With the exception of tempered glazing materials or laminated materials, the identification shall not be omitted unless approved and an affidavit is furnished by the glazing contractor certifying that each light is glazed in accordance with approved construction documents that comply with the provisions of this chapter. Safety glazing shall be identified in accordance with Section 2406.2. Each pane of tempered or laminated glass, except tempered or laminated spandrel glass, shall be permanently identified by the manufacturer. The identification label shall be acid etched,	Overlap exists. Need determination.

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Tempered spandrel glass shall be provided with a removable paper marking by the manufacturer.	sand blasted, ceramic fired, embossed or shall be of a type that once applied cannot be removed without being destroyed. Tempered or laminated spandrel glass shall be provided with a removable paper marking by the manufacturer.	
SECTION 2404 **WIND, SNOW, SEISMIC** **AND DEAD LOADS ON GLASS**	**SECTION 2404** **WIND AND DEAD LOADS ON GLASS**	
2404.1 Vertical glass. Glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors and other exterior applications shall be designed to resist the wind loads in Section 1609 for components and cladding. Glass in glazed curtain walls, glazed storefronts and glazed partitions shall meet the seismic requirements of ASCE 7, Section 13.5.9. The load resistance of glass under uniform load shall be determined in accordance with ASTM E 1300. The design of vertical glazing shall be based on the following equation: *Fgw* δ *Fga* **(Equation 24-1)** where: *Fgw* = Wind load on the glass computed in accordance with Section 1609. *Fga* = Short duration load on the glass as determined in accordance with ASTM E 1300.	**2404.1 Vertical glass.** Glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors and other exterior applications shall be designed to resist the wind loads for components and cladding. The load resistance of glass under uniform load shall be determined in accordance with ASTM E 1300. Design of exterior windows and glass doors in accordance with Section 2404.1 shall utilize the same edition of ASTM E 1300 used for testing in accordance with Section 1714.5. The design of vertical glazing shall be based on the following equation: Fgw ≤ Fga (Equation 24-1) where: Fgw is the wind load on the glass computed in accordance with Section 1609 and Fga is the short duration load resistance of the glass as determined in accordance with ASTM E 1300.	Overlap exists. Need determination.
2404.2 Sloped glass. Glass sloped more than 15 degrees (0.26 rad) from vertical in skylights, sunrooms, sloped roofs and other exterior applications shall be designed to resist the most critical of the following combinations of loads. *Fg=Wo–D* **(Equation 24-2)** *Fg=Wi+D+0.5 S* **(Equation 24-3)** *Fg=0.5Wi+D+S* **(Equation 24-4)** where: *D* = Glass dead load psf (kN/m2). For glass sloped 30 degrees (0.52 rad) or less from horizontal, *D* = 13 *tg* (For SI: 0.0245 *tg*). For glass sloped more than 30 degrees (0.52 rad) from horizontal, *D* = 13 *tg* cos θ (For SI: 0.0245 *tg* cos θ).	**2404.2 Sloped glass.** Glass sloped more than 15 degrees (0.26 rad) from vertical in skylights, sunrooms, sloped roofs and other exterior applications shall be designed to resist the most critical of the following combinations of loads. Fg = Wo – D (Equation 24-3) Fg = Wi + D (Equation 24-4) Fg = 0.5 Wi + D (Equation 24-5) where: D = Glass dead load (psf)(kN/m2) For glass sloped 30 degrees (0.52 rad) or less from horizontal, D = 13 tg cos θ (For SI: 0.0245 tg cos θ). For glass sloped more than 30 degrees (0.52 rad) from	Overlap exists. Need determination.

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Fg = Total load, psf (kN/m2) on glass. S = Snow load, psf (kN/m2) as determined in Section 1608. *tg* = Total glass thickness, inches (mm) of glass panes and plies. *Wi* = Inward wind force, psf (kN/m2) as calculated in Section 1609. *Wo* = Outward wind force, psf (kN/m2) as calculated in Section 1609. θ = Angle of slope from horizontal. **Exception:** Unit skylights shall be designed in accordance with Section 2405.5. The design of sloped glazing shall be based on the following equation: *Fg≤Fga* **(Equation 24-5)** where: *Fg* = Total load on the glass determined from the load combinations above. *Fga* = Short duration load resistance of the glass as determined according to ASTM E 1300 for Equations 24-2 and 24-3; or the long duration load resistance of the glass as determined according to ASTM E 1300 for Equation 24-4 **Table 2404.2 C_2 Factors for Sloped Glass**. Figure 2404	horizontal, D = 13 tg cos θ (For SI: 0.0245 tg cos θ). Fg = Total load, psf (kN/m2) on glass. tg = Total glass thickness, inches (mm) of glass panes and plies. Wi = Inward wind force, psf (kN/m2) as calculated in Section 1609. Wo = Outward wind force, psf (kN/m2) as calculated in Section 1609. θ = Angle of slope from horizontal. The design of sloped glazing shall be based on the following equation: Fg ≤ Fga (Equation 24-6) where Fg is the total load on the glass determined from the load combinations above and Fga is the short duration load resistance of the glass as determined according to ASTM E 1300 for load combinations of equations 24-3 and 24-4; or the long duration load resistance of the glass as determined according to ASTM E 1300 for load combination 3 of Equation 24-5. **Table 2404.2 C_2 Factors for Sloped Glass**. Reserved. **Figure 2404 Reserved.**	
2404.3 Wired, patterned and sandblasted glass. **2404.3.1 Vertical wired glass.** Wired glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors and other exterior applications shall be designed to resist the wind loads in Section 1609 for components and cladding according to the following equation: *Fgw<0.5 Fge* **(Equation 24-6)** where: *Fgw*= Is the wind load on the glass computed per Section 1609. *Fge* = Nonfactored load from ASTM E 1300 using a thickness designation for monolithic glass that is not greater	**2404.3 Wired, patterned, and sandblasted glass.** **2404.3.1 Vertical wired glass.** Wired glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors and other exterior applications shall be designed to resist the wind loads in Section 1609 for components and cladding according to the following formula: Fgw < 0.5 Fge where Fgw is the wind load on the glass computed per Section 1609 and Fge is the nonfactored load from ASTM E 1300 using a thickness designation for monolithic glass that is not greater than the thickness of wired glass.	Overlap exists. Need determination.

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than the thickness of wired glass.		
2404.3.2 Sloped wired glass. Wired glass sloped more than 15 degrees (0.26 rad) from vertical in skylights, sunspaces, sloped roofs and other exterior applications shall be designed to resist the most critical of the combinations of loads from Section 2404.2. For Equations 24-2 and 24-3: *Fg<0.5 Fge* **(Equation 24-7)** For Equation 24-4: *Fg<0.3 Fge* **(Equation 24-8)** where: *Fg* = Total load on the glass. *Fge* = Nonfactored load from ASTM E 1300.	**2404.3.2** Wired glass sloped more than 15 degrees (0.26 rad) from vertical in skylights, sun rooms, sloped roofs, and other exterior applications shall be designed to resist the most critical of the combinations of loads from Section 2404.2 for load combinations of Equations 24-3 and 24-4: Fg < 0.5 Fge For load combinations of Equation 24-5. Fg < 0.3 Fge where Fg is total load on the glass and Fge is the nonfactored load from ASTM E 1300.	Overlap exists. Need determination.
2404.3.3 Vertical patterned glass. Patterned glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors and other exterior applications shall be designed to resist the wind loads in Section 1609 for components and cladding according to the following equation: *Fgw<1.0 Fge* **(Equation 24-9)** *Fgw*= Wind load on the glass computed per Section 1609. *Fge* = Nonfactored load from ASTM E 1300. The value for patterned glass shall be based on the thinnest part of the glass. Interpolation between nonfactored load charts in ASTM E 1300 shall be permitted.	**2404.3.3 Vertical patterned glass.** Patterned glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors, and other exterior applications shall be designed to resist the wind loads in Section 1609 for components and cladding according to the following formula: Fgw < 1.0 Fge where Fgw is the wind load on the glass computed per Section 1609 and Fge is the nonfactored load from ASTM E 1300. The value for patterned glass shall be based on the thinnest part of the glass. Interpolation between nonfactored load charts in ASTM E 1300 shall be permitted.	Overlap exists. Need determination.
2404.3.4 Sloped patterned glass. Patterned glass sloped more than 15 degrees (0.26 rad) from vertical in skylights, sunspaces, sloped roofs and other exterior applications shall be designed to resist the most critical of the combinations of loads from Section 2404.2. For Equations 24-2 and 24-3: *Fg<1.0 Fge* **(Equation 24-10)** For Equation 24-4: *Fg<0.6Fge* **(Equation 24-11)** where *Fg* = Total load on the glass. *Fge* = Nonfactored load from ASTM E 1300. The value for patterned glass shall be based on the thinnest part of the glass. Interpolation between the nonfactored load charts in ASTM E 1300 shall be permitted.	**2404.3.4** Patterned glass sloped more than 15 degrees (0.26 rad) from vertical in skylights, sun rooms, sloped roofs, and other exterior applications shall be designed to resist the most critical of the combinations of loads from Section 2404.2 for load combinations of Equations 24-3 and 24-4: Fg < 1.0 Fge For load combinations of Equations 24-5 Fg < 0.6 Fge where Fg is total load on the glass and Fge is the nonfactored load from ASTM E 1300. The value for patterned glass shall be based on the thinnest part of the glass. Interpolation between the nonfactored load charts in ASTM E 1300 shall be	Overlap exists. Need determination.

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	permitted.	
2404.3.5 Vertical sandblasted glass. Sandblasted glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors, and other exterior applications shall be designed to resist the wind loads in Section 1609 for components and cladding according to the following equation: *Fg 0.5 Fge* **(Equation 24-12)** where: *Fg* = Total load on the glass. *Fge* = Nonfactored load from ASTM E 1300. The value for sandblasted glass is for moderate levels of sandblasting.	**2404.3.5 Vertical sandblasted glass.** Sandblasted glass sloped 15 degrees (0.26 rad) or less from vertical in windows, curtain and window walls, doors, and other exterior applications shall be designed to resist the wind loads in Section 1609 for components and cladding according to the following formula: Fg < 0.5 Fge where Fg is total load on the glass and Fge is the nonfactored load from ASTM E 1300. The value for sandblasted glass is for moderate levels of sandblasting.	Overlap exists. Need determination.
2405.5 Unit skylights. Unit skylights shall be tested and labeled as complying with AAMA/WDMA/CSA 101/I.S.2/A440. The label shall state the name of the manufacturer, the approved labeling agency, the product designation and the performance grade rating as specified in AAMA/WDMA/CSA 101/I.S.2/A440. If the product manufacturer has chosen to have the performance grade of the skylight rated separately for positive and negative design pressure, then the label shall state both performance grade ratings as specified in AAMA/WDMA/CSA 101/I.S.2/A440 and the skylight shall comply with Section 2405.5.2. If the skylight is not rated separately for positive and negative pressure, then the performance grade rating shown on the label shall be the performance grade rating determined in accordance with AAMA/WDMA/CSA 101/I.S.2/A440 for both positive and negative design pressure and the skylight shall conform to Section 2405.5.1.	**2405.5 Unit skylights.** Unit skylights shall be tested and labeled as complying with 101/I.S.2/NAFS *Voluntary Performance Specification for Windows, Skylights and Glass Doors*. The label shall state the name of the manufacturer, the approved labeling agency, the product designation and the performance grade rating as specified in 101/I.S.2/NAFS. If the product manufacturer has chosen to have the performance grade of the skylight rated separately for positive and negative design pressure, then the label shall state both performance grade ratings as specified in 101/I.S.2/NAFS and the skylight shall comply with Section 2405.5.2. If the skylight is not rated separately for positive and negative pressure, then the performance grade rating shown on the label shall be the performance grade rating determined in accordance with 101/I.S.2/NAFS for both positive and negative design pressure, and the skylight shall conform to Section 2405.5.1.	Overlap exists. Need determination.
2409.1 Glass in elevator enclosures. Glass in elevator enclosures shall be laminated glass conforming to ANSI Z97.1 or 16 CFR Part 1201. Markings as specified in the applicable standard shall be on each separate piece of glass and shall remain visible after installation.	**2409 Glass in Floors and Sidewalks. Reserved**	Overlap exists. Need determination.
NA	**HVHZ Sections 2410-2415**	No overlap exists. Use Fl specific requirements.