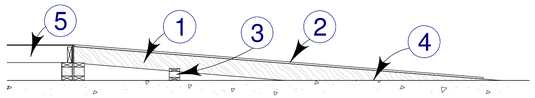
LAYOUT PAGE TABLE	
LABEL	TITLE
A1	1ST FLOOR PLAN
A2	2ND FLOOR PLAN
S1	SECTION AND ENGINEERING
S2	FLOOR FRAMING PLAN
L1	1ST FLOOR LIFE PLAN
L2	2ND FLOOR LIFE PLAN

Section Keyed No
1. 2x6 joists (16" O.C.) YP. Taper the ends as needed and shown.
2. 3/4" plywood surface
3. 2x4 blocking
4. Use metal clips on floor.
5. Raised Platform



1/2" Scale
Section C

MISC. NOTES

MECHANICAL, PLUMBING & ELECTRICAL SYSTEMS

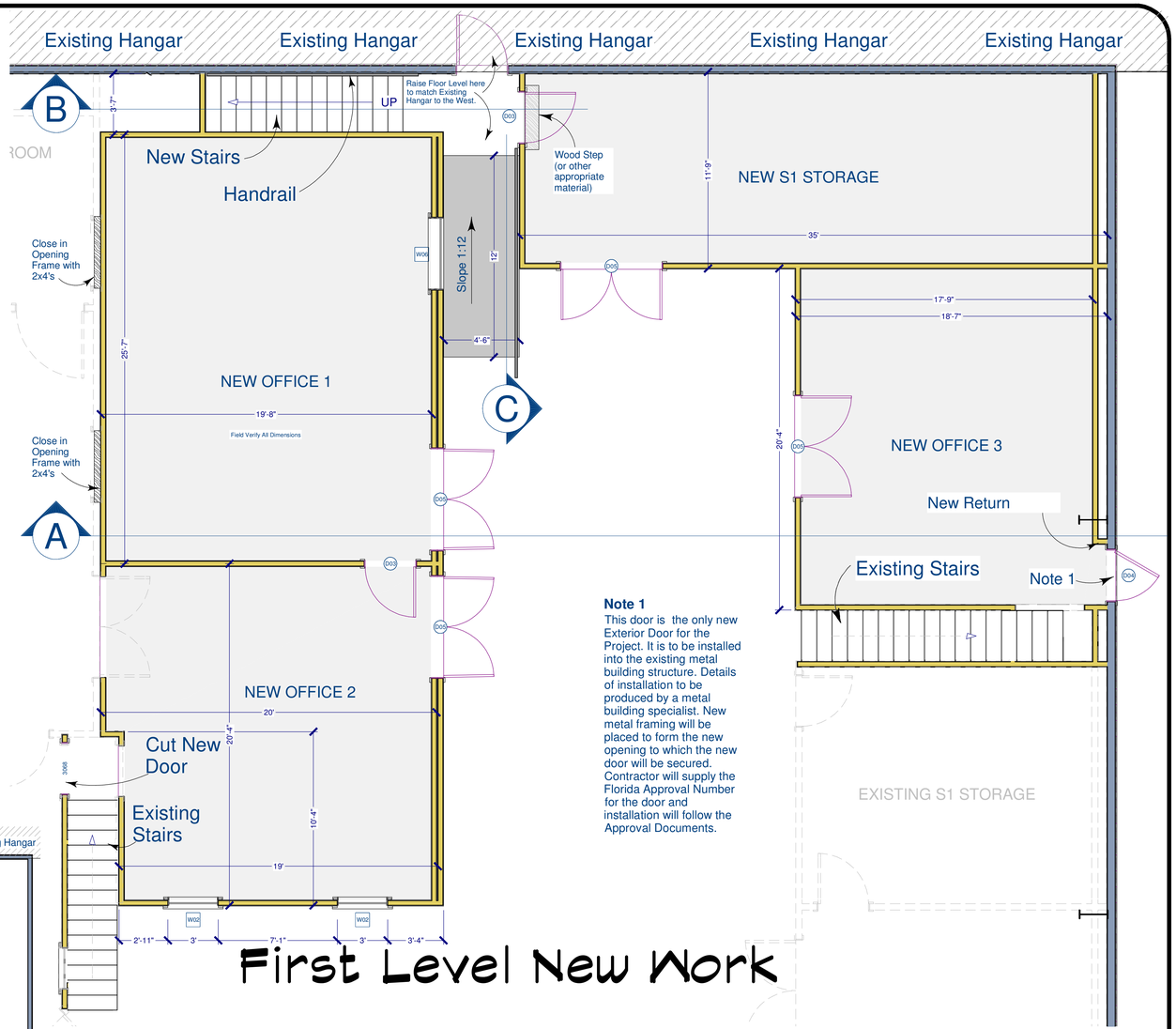
The Mechanical, Plumbing and Electrical subcontractors are responsible for compliance with all respective Codes affecting their trades. Any MPE data on these plans are for placement only and have not been engineered by the Engineer of Record or reviewed against the applicable codes.

FLORIDA APPROVAL NUMBERS

Contractor will supply Florida Approval Numbers as required for the new exterior door (Note 1).

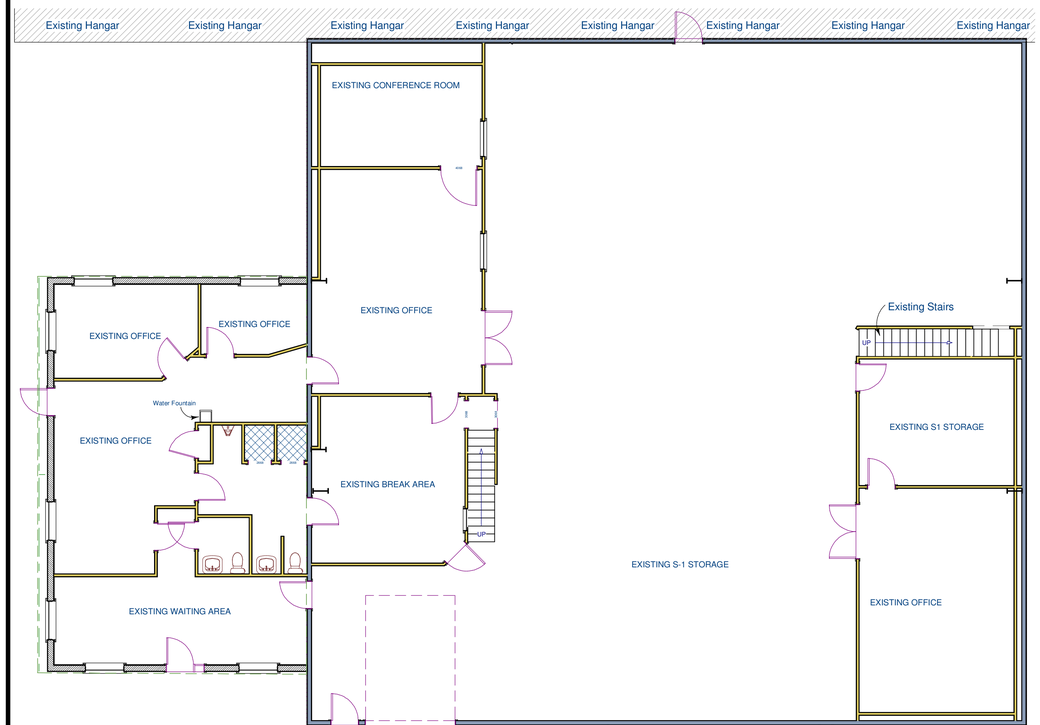
Building Data

Item	Info
Office Total	6592 sqft
S1 Storage Total	4065 sqft
Construction Type	III unprotected
New Office Area	2538 sqft



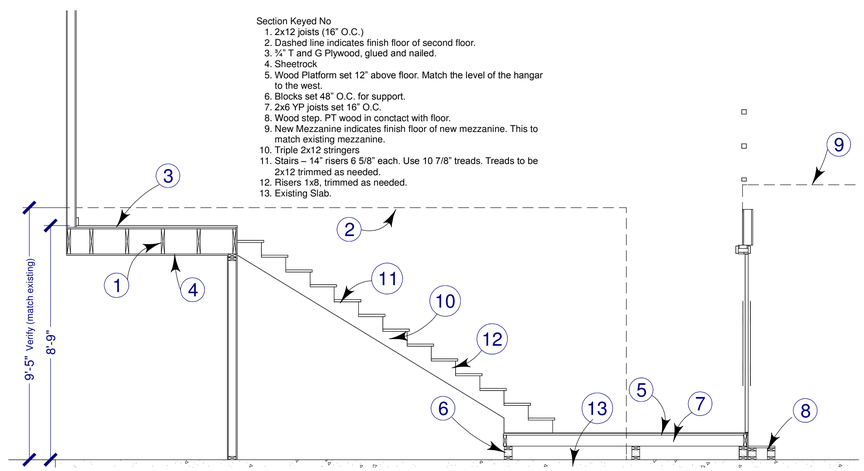
Note 1
This door is the only new Exterior Door for the Project. It is to be installed into the existing metal building structure. Details of installation to be produced by a metal building specialist. New metal framing will be placed to form the new opening to which the new door will be secured. Contractor will supply the Florida Approval Number for the door and installation will follow the Approval Documents.

First Level New Work



First Level As-Builts 1/8" Scale

Section Keyed No
1. 2x12 joists (16" O.C.)
2. Dashed line indicates finish floor of second floor.
3. ¾" T and G Plywood, glued and nailed.
4. Sheetrock
5. Wood Platform set 12" above floor. Match the level of the hangar to the west.
6. Blocks set 48" O.C. for support.
7. 2x6 YP joists set 16" O.C.
8. Wood step. PT wood in conctact with floor.
9. New Mezzanine indicates finish floor of new mezzanine. This to match existing mezzanine.
10. Triple 2x12 stringers
11. Stairs – 14" risers 6 5/8" each. Use 10 7/8" treads. Treads to be 2x12 trimmed as needed.
12. Risers 1x8, trimmed as needed.
13. Existing Slab.



Section B 3/8" Scale

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Owner:
ITEC
SE 147th Circle
Dunnellon, FLorida

Builder:
Pat Kelley Builder, Inc.
8085 N. W. 120th Street
Reddick, FL 32686

Office Space
Interior Additions
ITEC
SE 147th Circle
Dunnellon, FLorida

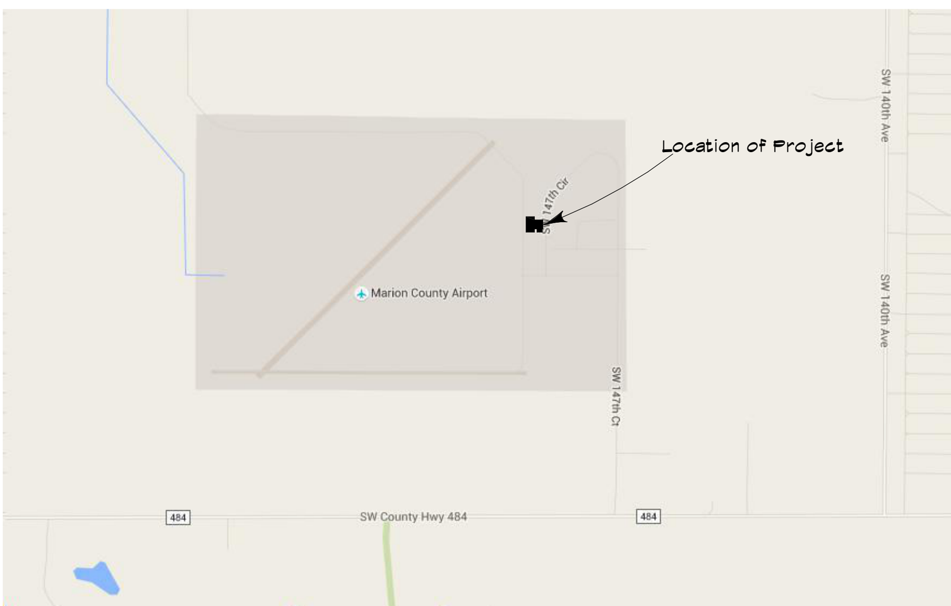
All Scales are 1/4" = 1'0"
Unless otherwise noted

Sheet Description
1st Floor Plan

Issue Date: 21 Oct 2015 A

REVISION TABLE	
NUMBER	DATE

A1



Locater Map (NTS)

MISC. NOTES

MECHANICAL, PLUMBING & ELECTRICAL SYSTEMS

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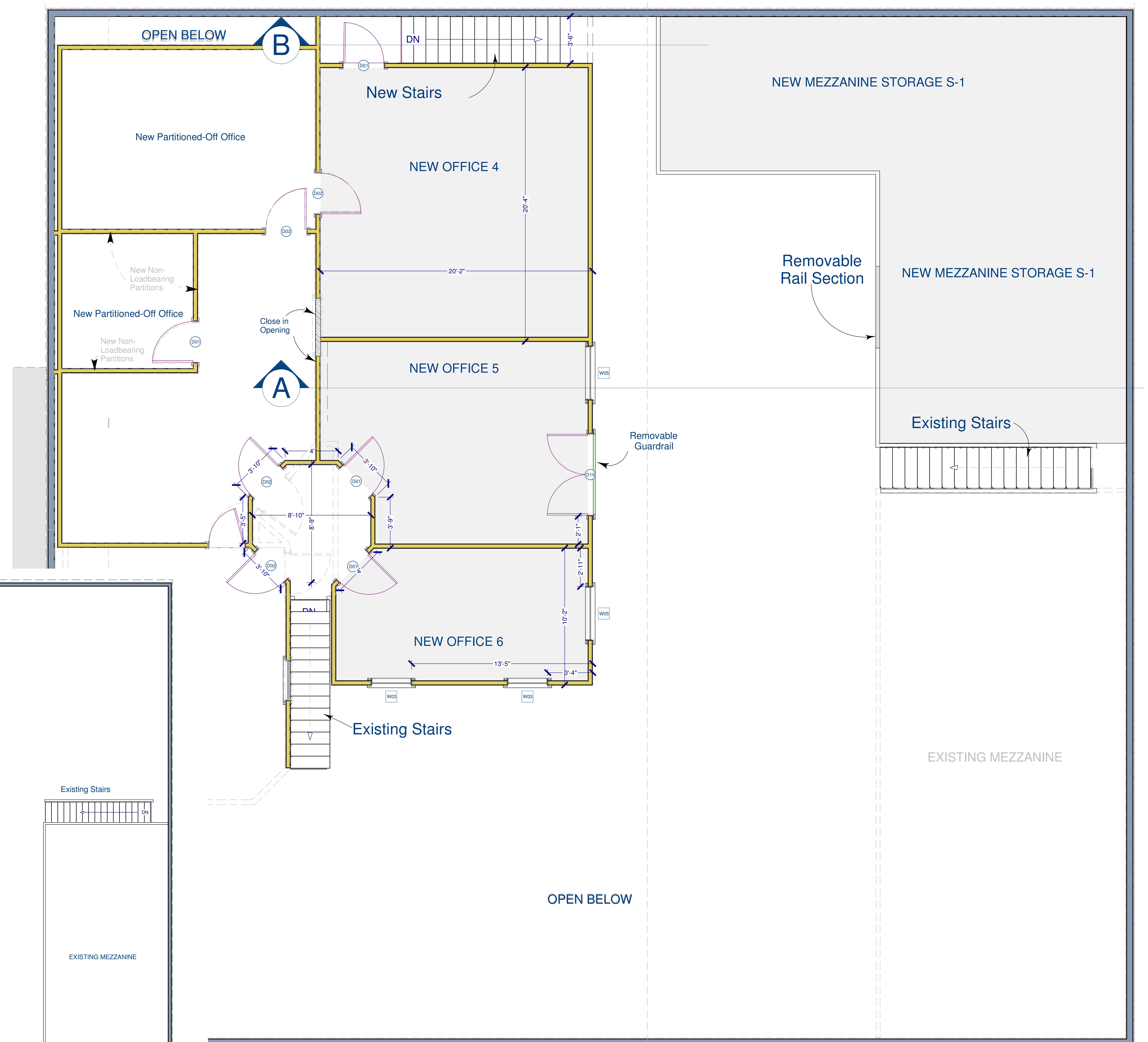
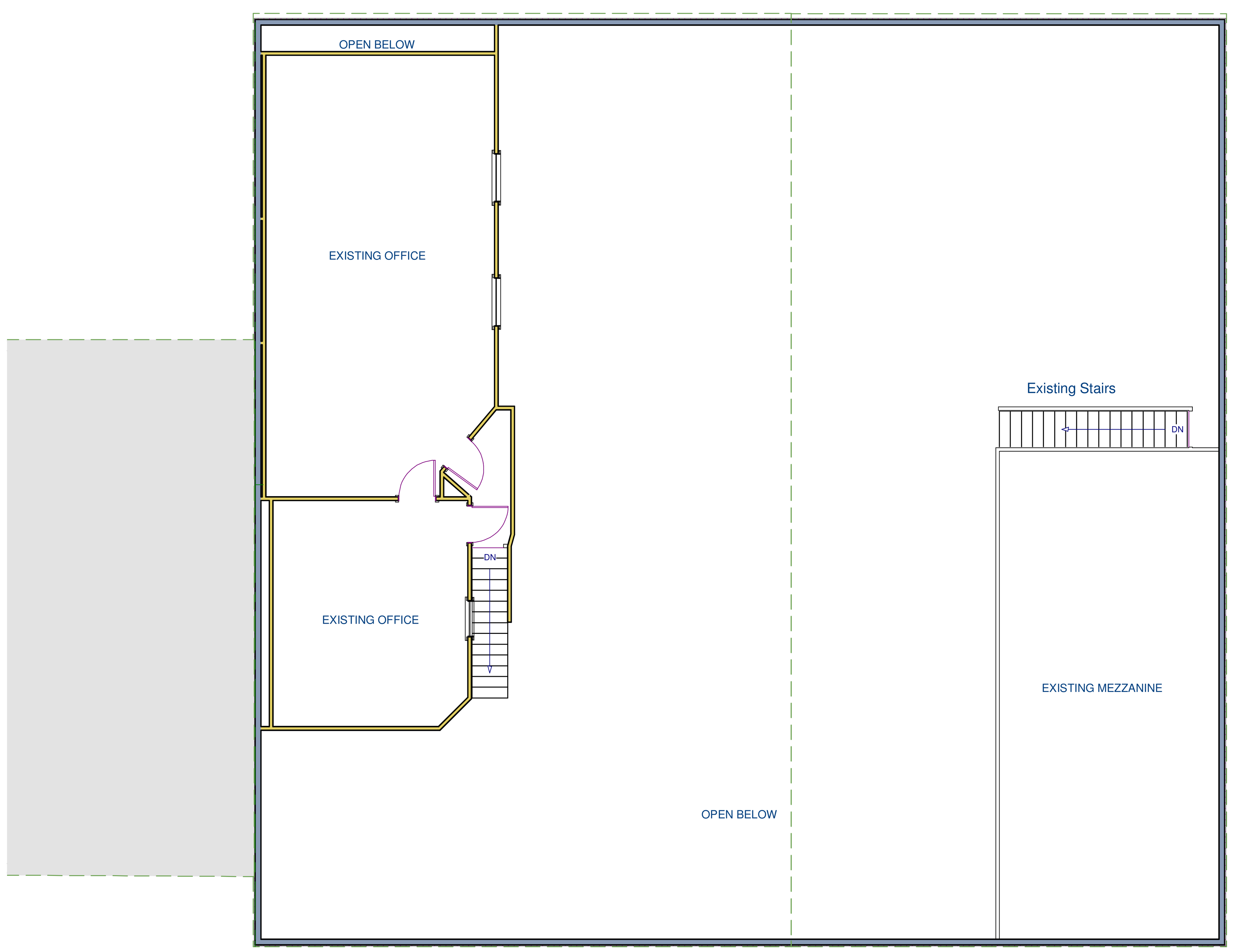
NEW DOORS (SOME OTHERS MAY NEED TO BE REVERSED, SEE PLANS)						
NUMBER	LABEL	QTY	SIZE	DESCRIPTION	THICKNESS	CODE
D01	3068	4	3068 L IN	HINGED-SLAB	1 3/8"	
D02	3068	4	3068 R IN	HINGED-SLAB	1 3/8"	
D19	6068	1	6068 L/R IN	DOUBLE HINGED-SLAB	1 3/8"	

NEW WINDOWS SCHEDULE (ALL INTERIOR WINDOWS)					
NUMBER	LABEL	QTY	SIZE	EGRESS	DESCRIPTION
W03	3040FX	2	3040FX		FIXED GLASS
W04	3040SH	1	3040SH		SINGLE HUNG
W05	4042FX	2	4042FX		FIXED GLASS



Second Level New Work



Second Level As-Builts 1/8" Scale

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Dunnellon, FLorida

Builder: Pat Kelley Builder, Inc.
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Reddick, FL 32686

Office Space Interior Additions
ITEC
SE 147th Circle
Dunnellon, FLorida

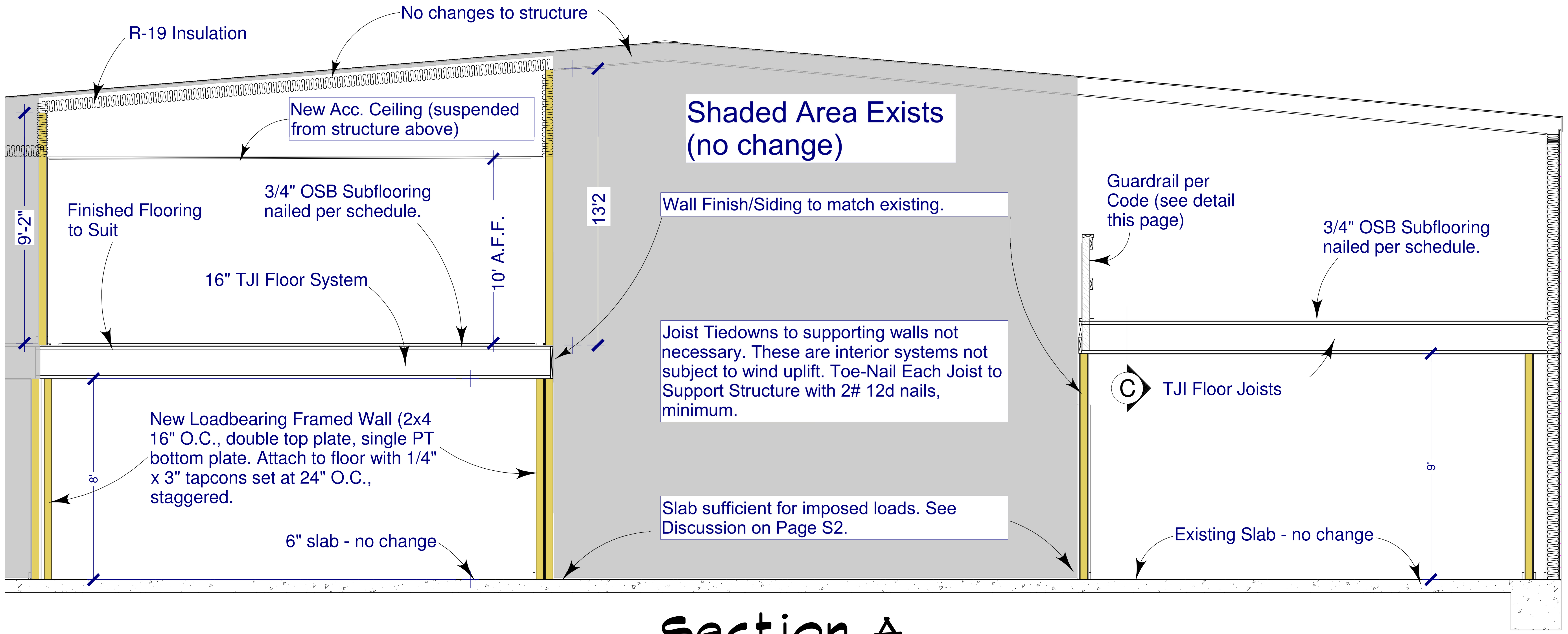
All Scales are 1/4" = 1'0"
Unless otherwise noted

Sheet Description
2nd Floor Plan

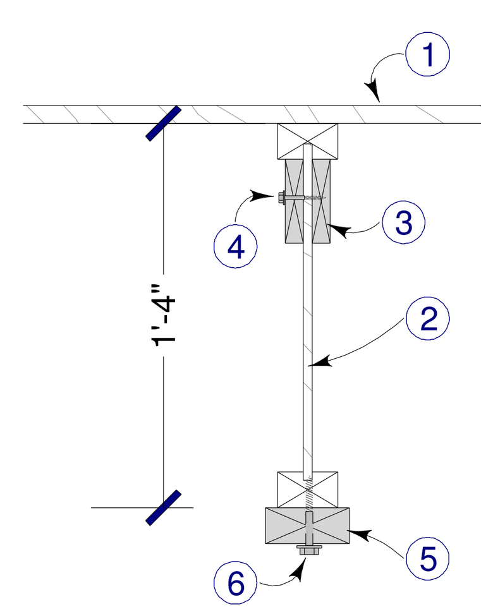
Issue Date: 21 Oct 2015 A

REVISION TABLE	
NUMBER	DATE

A2

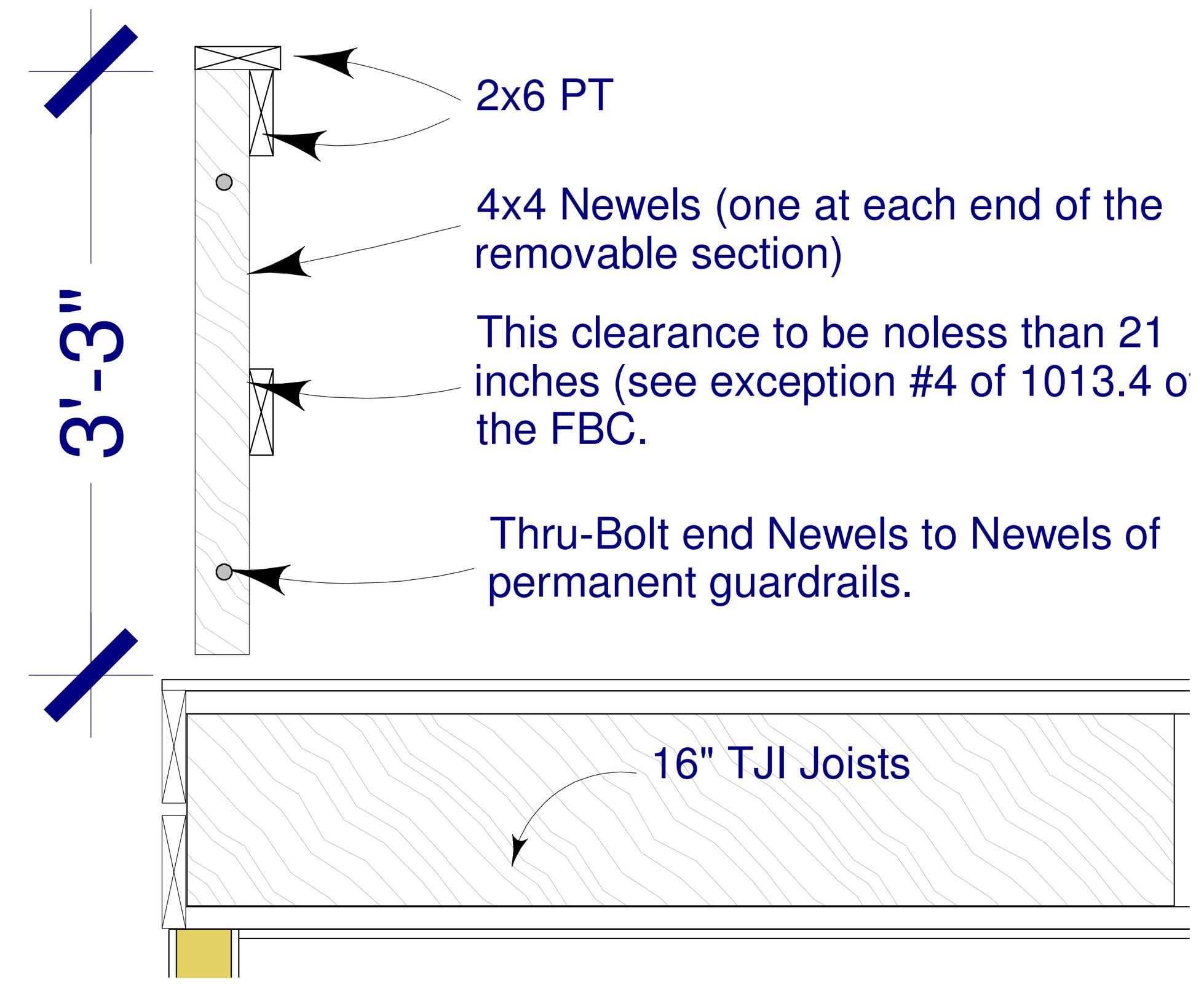


Section A
3/8" Scale

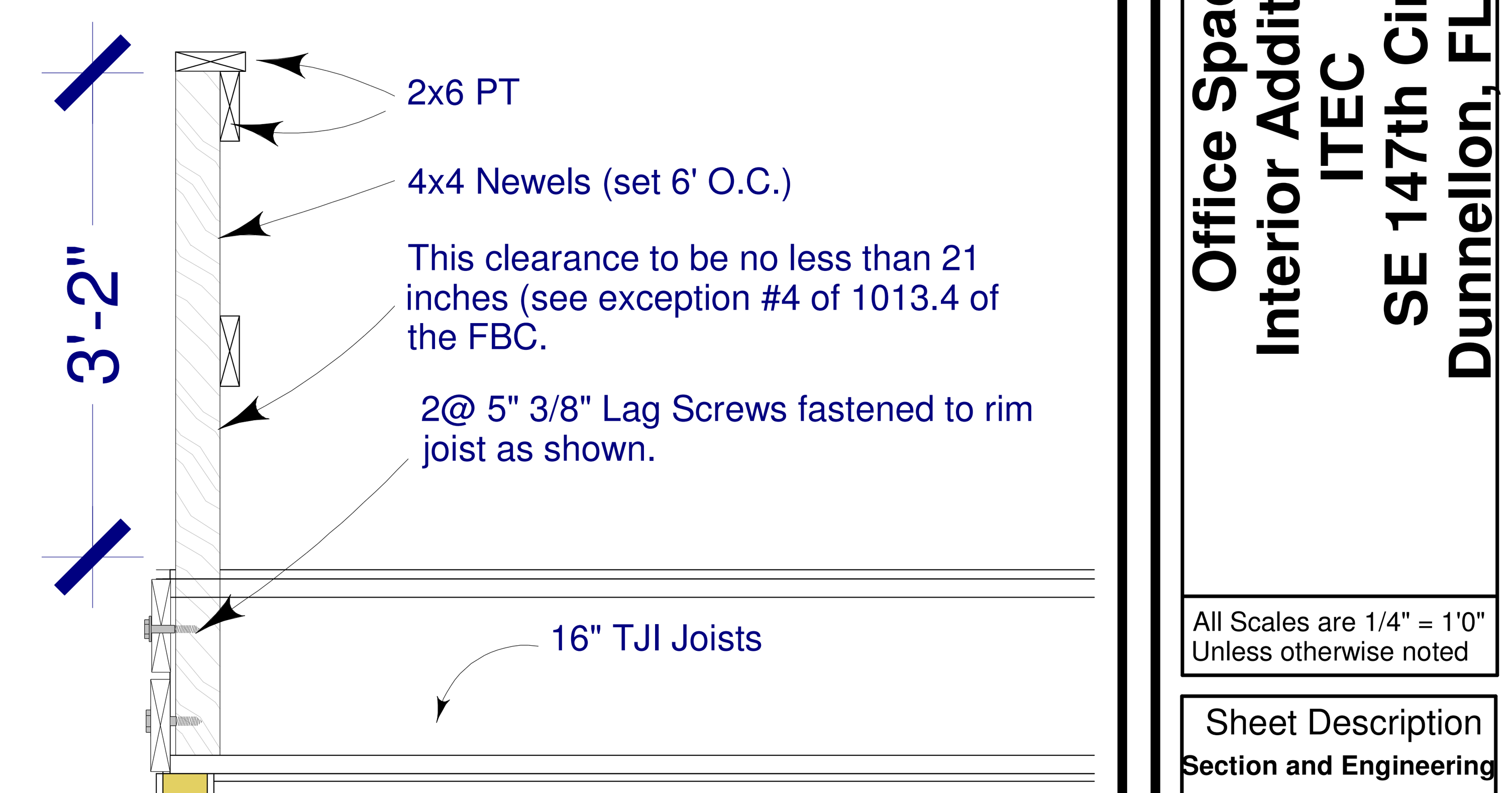


Section Keyed Note
1. ¾" plywood subflooring
2. LPI32 x 16" Plywood Floor Joists
3. Double 1x4's (YP). Glued and Lag Screwed as Shown.
4. 3/8" x 1.5" Lag Screws placed 16" O.C.
5. Add 2x4 glued to bottom flange of the joists.
6. 3/8" x 3" Lag Screws placed 16" O.C.

Section C
1.5" Scale



Removable Guard Section
1" Scale



Guard Rail for Mezzanine
1" Scale

Project: **Component Pressure New Door on North Side**

Floor and Roof Live Loads	
Offices:	50 psf uniform; 2000 lbs concentrated
Lobbies:	100 psf uniform; 2000 lbs concentrated
First Floor Corridors:	100 psf uniform; 2000 lbs concentrated
Upper Floor Corridors:	80 psf uniform; 2000 lbs concentrated
Roofs:	20 psf uniform; 300 lbs concentrated
Wind Design Data	
Ultimate Wind Speed:	130 mph
Nominal Wind Speed:	101 mph
Risk Category:	II
Wind Exposure:	C
Enclosure Classification:	Enclosed
Internal Pressure Coefficient:	0.18 +/-
Components and Cladding Design Pressures:	
Roofing Zone 1:	+17.4 psf max., -42.6 psf min.
Roofing Zone 2:	+17.4 psf max., -71.4 psf min.
Roofing Zone 3:	-107.5 psf min.
Roofing at Zone 2 Overhangs:	-61.3 psf min.
Roofing at Zone 3 Overhangs:	-100.9 psf min.
Stucco, Cladding, Doors & Windows:	
	+42.6 psf max., -46.2 psf min.
Zone 5:	+42.6 psf max., -57.0 psf min.
End Zone Width:	8.00 ft.
The Ultimate Wind Speed was used to determine the above Component and Cladding Design Pressures.	
This Building is not in a Wind-Borne Debris Region, and opening protection is not required.	
The site of this building is not subject to special topographic wind effects as per Section 1609.1.1.1 of the 2014 FBC.	
Geotechnical Information	
Design Soil Load-Bearing Capacity:	2,000 psf
Flood Design Data	
Flood Zone:	X

This table was created using Windload Calculator Plus software (2014 Florida Building Code Edition) available from WindCalcs.com

FASTENING SCHEDULE

CONNECTION	FASTENINGa, m	LOCATION
1. Joist to sill or girder	3 - 8d common (21/2? × 0.131?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	toenail
2. Bridging to joist	2 - 8d common (21/2? × 0.131?) 2 - 3? × 0.131? nails 2 - 3? 14 gage staples	toenail each end
3. 1? × 6? subfloor or less to each joist	2 - 8d common (21/2? × 0.131?)	face nail
4. Wider than 1? × 6? subfloor to each joist	3 - 8d common (21/2? × 0.131?)	face nail
5. 2? subfloor to joist or girder	2 - 16d common (31/2? × 0.162?)	blind and face nail
6. Sole plate to joist or blocking	16d (31/2? × 0.135?) at 16? o.c. 3? × 0.131? nails at 8? o.c. 3? 14 gage staples at 12? o.c.	typical face nail
Sole plate to joist or blocking at braced wall panel	3 - 16d (31/2? × 0.135?) at 16? o.c. 4 - 3? × 0.131? nails at 16? o.c. 4 - 3? 14 gage staples at 16? o.c.	braced wall panels
7. Top plate to stud	2 - 16d common (31/2? × 0.162?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	end nail
8. Stud to sole plate	4 - 8d common (21/2? × 0.131?) 4 - 3? × 0.131? nails 3 - 3? 14 gage staples	toenail
	2 - 16d common (31/2? × 0.162?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	end nail
9. Double studs	16d (31/2? × 0.135?) at 24? o.c. 3? × 0.131? nail at 8? o.c. 3? 14 gage staple at 8? o.c.	face nail
10. Double top plates	16d (31/2? × 0.135?) at 16? o.c. 3? × 0.131? nail at 12? o.c. 3? 14 gage staple at 12? o.c.	typical face nail
Double top plates	8 - 16d common (31/2? × 0.162?) 12 - 3? × 0.131? nails 12 - 3? 14 gage staples	lap splice
11. Blocking between joists or rafters to top plate	3 - 8d common (21/2? × 0.131?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	toenail
12. Rim joist to top plate	8d (21/2? × 0.131?) at 6? o.c. 3? × 0.131? nail at 6? o.c. 3? 14 gage staple at 6? o.c.	toenail
13. Top plates, laps and intersections	2 - 16d common (31/2? × 0.162?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	face nail
14. Continuous header, two pieces	16d common (31/2? × 0.162?)	16? o.c. along edge
15. Ceiling joists to plate	3 - 8d common (21/2? × 0.131?) 5 - 3? × 0.131? nails 5 - 3? 14 gage staples	toenail
16. Continuous header to stud	4 - 8d common (21/2? × 0.131?)	toenail
17. Ceiling joists, laps over partitions (see Section 2308.10.4.1, Table 2308.10.4.1)	3 - 16d common (31/2? × 0.162?) minimum, Table 2308.10.4.1 4 - 3? × 0.131? nails 4 - 3? 14 gage staples	face nail
18. Ceiling joists to parallel rafters (see Section 2308.10.4.1, Table 2308.10.4.1)	3 - 16d common (31/2? × 0.162?) minimum, Table 2308.10.4.1 4 - 3? × 0.131? nails 4 - 3? 14 gage staples	face nail
19. Rafter to plate (see Section 2308.10.1, Table 2308.10.1)	3 - 8d common (21/2? × 0.131?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	toenail
20. 1? diagonal brace to each stud and plate	2 - 8d common (21/2? × 0.131?) 2 - 3? × 0.131? nails 3 - 3? 14 gage staples	face nail
21. 1? × 8? sheathing to each bearing	3 - 8d common (21/2? × 0.131?)	face nail
22. Wider than 1? × 8? sheathing to each bearing	3 - 8d common (21/2? × 0.131?)	face nail
23. Built-up corner studs	16d common (31/2? × 0.162?) 3? × 0.131? nails 3? 14 gage staples	24? o.c. 16? o.c. 16? o.c.
24. Built-up girder and beams	20d common (4? × 0.192?) 32? o.c. 3? × 0.131? nail at 24? o.c. 3? 14 gage staple at 24? o.c.	face nail at top and bottom staggered on opposite sides
	2 - 20d common (4? × 0.192?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	face nail at ends and at each splice
25. 2? planks	16d common (31/2? × 0.162?)	at each bearing
26. Collar tie to rafter	3 - 10d common (3? × 0.148?) 4 - 3? × 0.131? nails 4 - 3? 14 gage staples	face nail
27. Jack rafter to hip	3 - 10d common (3? × 0.148?) 4 - 3? × 0.131? nails 4 - 3? 14 gage staples	toenail
	2 - 16d common (31/2? × 0.162?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	face nail
28. Roof rafter to 2-by ridge beam	2 - 16d common (31/2? × 0.162?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	toenail
	2 -16d common (31/2? × 0.162?) 3 - 3? × 0.131? nails 3 - 3? 14 gage staples	face nail
29. Joist to band joist	3 - 16d common (31/2? × 0.162?) 4 - 3? × 0.131? nails 4 - 3? 14 gage staples	face nail
30. Ledger strip	3 - 16d common (31/2? × 0.162?) 4 - 3? × 0.131? nails 4 - 3? 14 gage staples	face nail at each joist
31. Wood structural panels and particleboard Subfloor, roof and wall sheathing (to framing)	1/2? and less: 6dc, 1 2 3/8? × 0.113? nailn 13/4? 16 gageo 15/32? to 19/32?: 8d common (roofs in 110-140 Vasd mph (Exp. B) 19/32? to 3/4?: 8dd or 6de 23/8? × 0.113? nailp 2? 16 gage 7/8? to 1?: 8dc 11/8? to 11/4?: 10dd or 8dd	6 inch o.c. edges and intermediate, 4 inch o.c. at component and cladding edge strip # Zone 3 [refer to Figure 30.5-1 of ASCE 7]
Single Floor (combination subfloor-to underlayment framing)	3/4? and less: 6de 7/8? to 1?: 8de 11/8? to 11/4?: 10dd or 8de	
32. Panel siding (to framing)	1/2? or less: 6d f 5/8?: 8d f	
33. Fiberboard sheathingg	1/2?: No. 11 gage roofing nailh 6d common nail (2? × 0.113?) No. 16 gage staplei 25/32?: No. 11 gage roofing nails 8d common nail (21/2? × 0.131?) No. 16 gage staplei	
34. Interior paneling	1/4?: 4dj 3/8?: 6dk	

Use this Fastening Schedule as a general guide for all nailing Unless Otherwise Noted specifically. All notes that generally reference a nailing schedule are referencing this nailing schedule.

For SI: 1 inch = 25.4 mm.
a. Common or box nails are permitted to be used except where otherwise stated.
b. Nails spaced at 6 inches on center at edges, 12 inches at intermediate supports except 6 inches at supports where spans are 48 inches or more. For nailing of wood structural panel and particleboard diaphragms and shear walls, refer to Section 2305. Nails for wall sheathing are permitted to be common, box or casing.
c. Common or deformed shank (6d - 2? × 0.113?; 8d - 21/2? × 0.131?; 10d - 3? × 0.148?).
d. Common (6d - 2? × 0.113?; 8d - 21/2? × 0.131?; 10d - 3? × 0.148?).
e. Deformed shank (6d - 2? × 0.113?; 8d - 21/2? × 0.131?; 10d - 3? × 0.148?).
f. Corrosion-resistant siding (6d - 17/8? × 0.106?; 8d - 23/8? × 0.128?) or casing (6d - 2? × 0.099?; 8d - 21/2? × 0.113?) nail.
g. Fasteners spaced 3 inches on center at exterior edges and 6 inches on center at intermediate supports, when used as structural sheathing. Spacing shall be 6 inches on center on the edges and 12 inches on center at intermediate supports for nonstructural applications.
h. Corrosion-resistant roofing nails with 7/16-inch-diameter head and 11/2-inch length for 1/2-inch sheathing and 13/4-inch length for 25/32-inch sheathing.
i. Corrosion-resistant staples with nominal 7/16-inch crown or 1-inch crown and 11/4-inch length for 1/2-inch sheathing and 11/2-inch length for 25/32-inch sheathing. Panel supports at 16 inches (20 inches if strength axis in the long direction of the panel, unless otherwisemarked).
j. Casing (11/2? × 0.080?) or finish (11/2? × 0.072?) nails spaced 6 inches on panel edges, 12 inches at intermediate supports.
k. Panel supports at 24 inches. Casing or finish nails spaced 6 inches on panel edges, 12 inches at intermediate supports.
l. For roof sheathing applications, 8d nails (21/2? × 0.113?) are the minimum required for wood structural panels.
m. Staples shall have a minimum crown width of 7/16 inch.
n. For roof sheathing applications, fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.
o. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports for subfloor and wall sheathing and 3 inches on center at edges, 6 inches at intermediate supports for roof sheathing.
p. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.

GENERAL REQUIREMENTS

BUILDING CODE REQUIREMENTS

All work, materials and installation shall be in strict accordance with all extant ordinances, State and local building codes, OSHA regulations, and codes in force by reference, latest adopted editions, including: The Fifth Edition of the 2014 Florida Building Code; the current National Electric Code; "Building Code Requirements for Reinforcing Concrete" (ACI 318-05); "Specifications for Structural Concrete Buildings" (ACI 301-05); "Building Code Require-ments for Masonry Structures" (ACI 530-05); "Wood-Framed Construction Manual"; and "APA Plywood Design Specification Manual".

NOTICE TO BUILDER AND OWNER

It is the intent of the designer that these plans are accurate and are clear enough for a licensed professional to construct this project. If the owner intends to build this project without the aid of a licensed professional contractor, it is assumed that the owner has the same abilities and knowledge as a fully licensed and experienced professional builder.

Post-permitting consulting fees are established in separate agreements between the designer and the builder or owner, and were not part of the agreement to produce these construction documents. No construction administration or inspection services were included in the agreement to produce these construction documents.

OWNERSHIP OF DOCUMENTS

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DIMENSIONING CONVENTIONS

Written dimensions shall at all times take precedence over scaled dimensions, and no workman shall rely upon the scale of any portion of the drawings in determining dimensions on the job site.

All structural conditions noted as "existing" or of existing structures are based on the best information currently available at the time of preparation of these documents. The Contractor is to verify all conditions prior to commencing work and report any anomalies that may affect the Work. The Contractor is to verify all dimensions prior to construction.

SHORING & OTHER CONSTRUCTION PROCEDURES

The shoring of structural systems and foundation excavations are the responsibilities of the Contractor. Site visits by the designer do not include inspection of construction procedures. Complete shoring plans and calculations (when required by the Building Official) shall be submitted for plan check for the necessary approvals prior to commencing with the work.

LEAD-BASED PAINT POISONING PREVENTION

In the renovation of all residential structures constructed prior to 1978, the contractor shall comply with all provisions of Federal Code of Regulations Title 40, Part 745, "Lead-Based Paint Poisoning Prevention in Certain Residential Structures."

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Office Space Interior Additions
ITEC
SE 147th Circle
Dunnellon, FLorida

All Scales are 1/4" = 1'0" Unless otherwise noted

Sheet Description: Section and Engineering

Issue Date: 21 Oct 2015 A

REVISION TABLE	
NUMBER	DATE

FRAMING WOOD

All structural lumber shall be either Spruce-Pine-Fir (SPF) or Southern Yellow Pine (SYP) No. 2, 1200f. All wood in direct contact with masonry or concrete shall be SYP pressure-treated with an approved preservative.

Plywood. All plywood sheathing shall be marked "CD" By the DFPA, and shall comply w/ US Product standard PS 1-77. All horizontal plywood diaphragms (i.e., roofs & floors) shall be laid face grain perpendicular to joists or rafters & staggered w/ the joists.

Provide 2x solid blocking between joists & rafters @ all supports. Blocking shall be one-piece & the full depth of the joist or rafter. Cross-bridging or solid blocking shall be provided @ 8'-0" O.C. max.

Cutting & Notching of Wood Floor Joists, Beams & Girders: notches on the ends of joists shall not exceed one-fourth the joist depth. Holes bored in joists shall not be within 2" of the top or bottom of the joist, & the diameter of any such hole shall not exceed one-third the depth of the joist. Notches in the top of bottom of joists shall not exceed one-sixth the depth & shall be located in the middle third of the span.

Cutting & Notching of Wood Studs: In exterior walls & bearing partitions, any wood stud may be cut or notched to a depth not exceed\-ing 25 percent of the width of the stud. In on bearing parti\-tions, any wood stud may be cut or notched to a depth not exceeding 40 percent of the width of the stud.

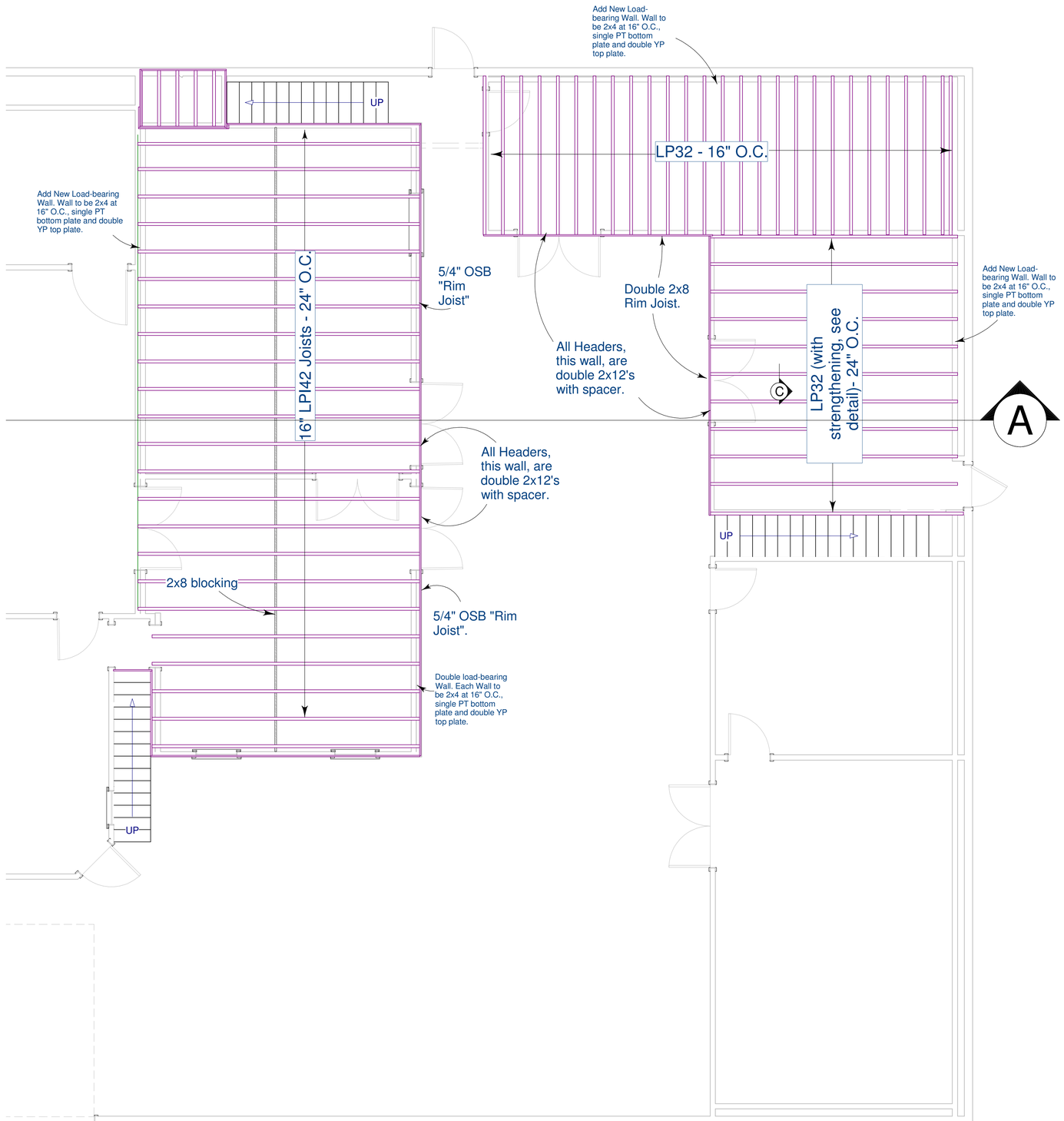
Bored holes in Wood Studs: a hole not greater in diameter than 40 per\-cent of the stud width may be bored in any wood stud. Bored holes not greater than 60 percent of the width of the stud are permitted in non-bearing partitions or in any wall where each bored stud is doubled, provided not more than two such successive doubled stud are so bored. In no case shall the edge of the bored hole be nearer than 5/8" to the edge of the stud. Bored holes shall not be located @ the same sec\-tion of stud as a cut or notch.

Bearing walls (not otherwise braced by plywood) shall be braced w/ not less than 1x6 diagonal let-ins, a minimum of one each end of each exterior wall not to exceed 25'-0" O.C. Each brace shall cover 4 studs & be nailed w/ (2) 8d nails @ each end into the top plate & sill & (2) 8d nails into each intersect\-ing stud.

The enclosed space in stud walls, partitions, & furred walls shall be fire-stopped @ the top, bottom & mid-point in which are more than 10' high. Fire-stops shall consist of wood not less than 2" nominal thickness or of incombustible materials as permitted by the building code. Fire-stopping shall form a complete block across the space to be fire-stopped & the space between them shall not ex\-ceed 10' measured horizontally or vertically. Top & bottom plates which fill all spaces between studs & furring shall be considered fire-stops.

The top plates of all stud walls shall be 2 pieces the same size as the studs, spliced to lap a min. of 4'-0" & nailed as per the schedule.

Glue-lam lumber shall be fabricated as per UBC Standard No. 25-10, sect 2511(f). Exposed structural glue-laminated lumber shall be moisture-resistant, treated wood.



Based upon our engineering evaluation (see supporting data below) the existing slab as it sits will accommodate up to 1628 Pounds per Lineal Foot along the walls. This is sufficient for all loadings that being anticipated for the new usage. No additional footings will be required.
The Mezzanine:
Based upon a light loading of 125 psf the support walls will support 1140 Pounds per Lineal Foot
The Offices:
Based upon the worst case office loading the support walls will be loaded at 950 Pounds per Lineal Foot.

3-3. Stationary live loads.
Floor slabs on grade should have adequate structural live loads. Since floor slabs are designed for moving live loads, the design should be checked for stationary live loading conditions. Table 3-1 lists values for maximum stationary live loads on floor slabs. For very heavy stationary live loads, the floor slab thicknesses listed in table 3-1 will control the design. Table 3-1 was prepared using the equation

$$w = 257.876s\sqrt{\frac{kh}{E}} \qquad \text{(eq 3-1)}$$

where
- w = the maximum allowable distributed stationary live load, pounds per square foot
- s = the allowable extreme fiber stress in tension excluding shrinkage stress and is assumed to be equal to one-half the normal 28-day concrete flexural strength, pounds per square inch
- k = the modulus of subgrade reaction, pounds per cubic inch
- h = the slab thickness, inches
- E= the modulus of elasticity for the slab (assumed to equal 4.0 x 106 pounds per square inch)

The above equation may be used to find allowable loads for combinations of values of s, h, and knot given in table 3-1. Further safety may be obtained by reducing allowable extreme fiber stress to a smaller percentage of the concrete flexural strength have been presented by Grieb and Werner, Waddell, and Hammitt (see Biblio). The selection of the modulus of subgrade reaction for use in table 3-1 is discussed in paragraph 4-2d. The design should be examined for the possibility of differential settlements which could result from nonuniform subgrade support. Also, consideration of the effects of long-term overall settlement for stationary live loads may be necessary for compressible soils (see TM 5-818-1/AFM 88-3, Chap. 7).

Types of Materials	Modulus of Subgrade Reaction, k, in lb/in³ for Moisture Contents of: 1 to 4%	5 to 8%	9 to 12%	13 to 16%	17 to 20%	21 to 24%	25 to 28%	Over 25%
Silts and clays Liquid limit > 50 (OH, CH, MH)	--	175	150	125	100	75	50	25
Silts and clays Liquid limit < 50 (CL, CL, ML)	--	200	175	150	125	100	75	50
Silty and clayey sands (SM & SC)	300	250	225	200	150	–	–	--
Gravelly sands (SW & SP)	300+	300	250	–	–	–	–	--
Silty and clayey gravels (GM & GC)	300+	300+	300	250	–	–	–	--
Gravel and sandy gravels (GW & GP)	300+	300+	–	–	–	–	–	--

calculate w

$$257.876 \cdot 1250 \cdot \left(150 \cdot \frac{6}{2880000}\right)^{.5} = 5698.308 \quad \text{PSF}$$

Calculate PLF along typical wall.
Assuming 3.5' wide area under each wall.

$$\frac{5698.308}{3.5} = 1628.088 \quad \text{PLF}$$

Worst Case under mezzanine at 125 PSF

$$125 \cdot \frac{18}{2} = 1125 \quad \text{PLF} \qquad \text{OKAY}$$

All other loadings are less. Conclusion 6" slab is adequate for the imposed loads. No addiional footings needed.

Second Floor Framing (over first floor)

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Office Space
Interior Additions
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Dunnellon, FLorida

All Scales are 1/4" = 1'0"
Unless otherwise noted

Sheet Description
Floor Framing Plan

Issue Date: 21 Oct 2015 A

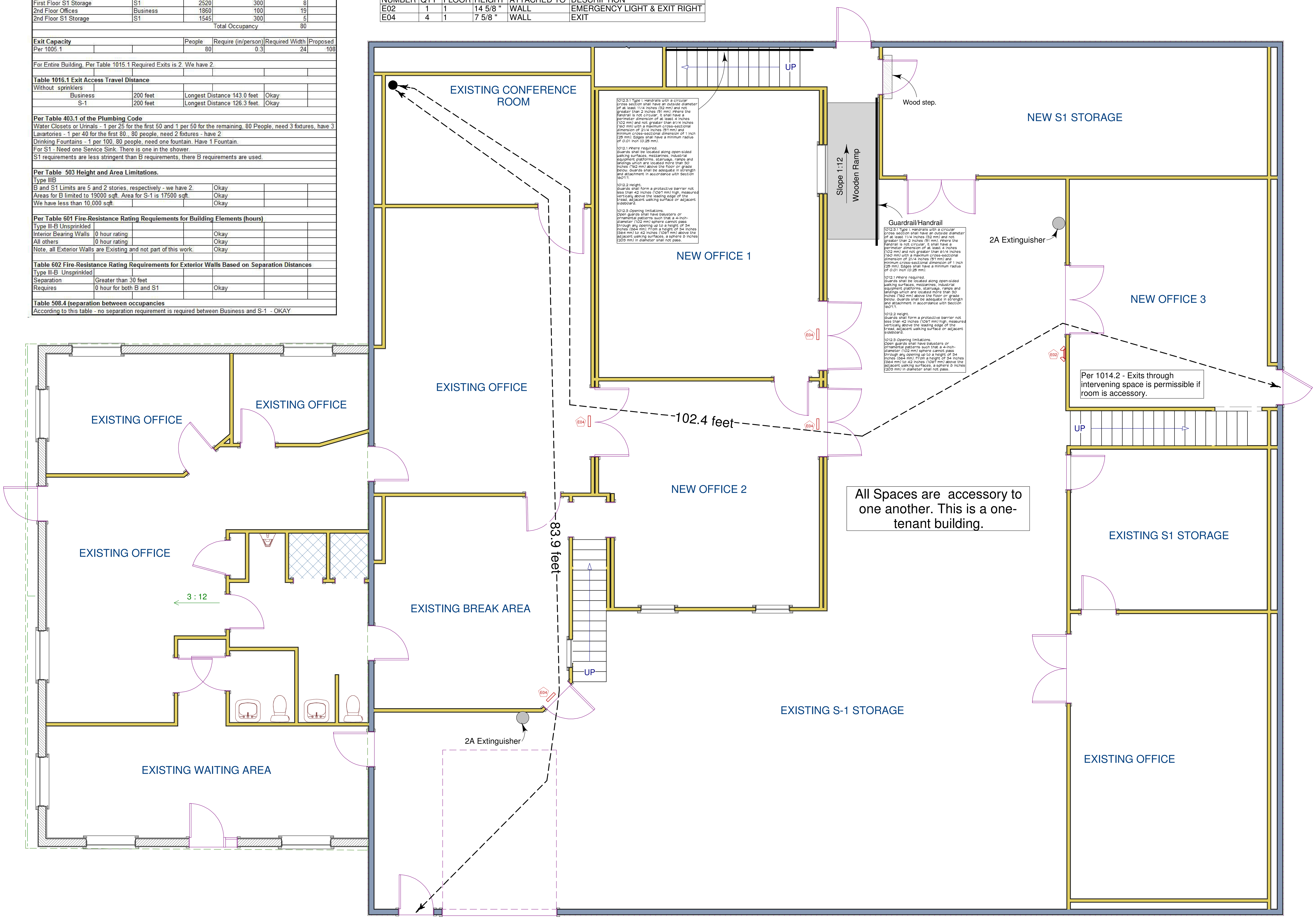
REVISION TABLE	
NUMBER	DATE

S2

OCCUPANCY, EXIT TABLE and Tables Tabulations						
Important Note: This table applies to the entire building for LIFE SAFETY.						
Per Table 1004.1.2						
Area		Occupancy	Area	Required	Capacity	
Existing Office to the South		Business	1283	100	12.83	
First Floor Offices		Business	3503	100	35.03	
First Floor S1 Storage		S1	2520	300	8	
2nd Floor Offices		Business	1860	100	19	
2nd Floor S1 Storage		S1	1545	300	5	
				Total Occupancy	80	
Exit Capacity			People	Require (in/person)	Required Width	Proposed
Per 1005.1			80	0.3	24	108
For Entire Building, Per Table 1015.1 Required Exits is 2. We have 2.						
Table 1016.1 Exit Access Travel Distance						
Without sprinklers						
Business		200 feet	Longest Distance 143.0 feet		Okay	
S-1		200 feet	Longest Distance 126.3 feet.		Okay	
Per Table 403.1 of the Plumbing Code						
Water Closets or Urinals - 1 per 25 for the first 50 and 1 per 50 for the remaining, 80 People, need 3 fixtures, have 3						
Lavartories - 1 per 40 for the first 80., 80 people, need 2 fixtures - have 2						
Drinking Fountains - 1 per 100, 80 people, need one fountain. Have 1 Fountain.						
For S1 - Need one Service Sink. There is one in the shower.						
S1 requirements are less stringent than B requirements, there B requirements are used.						
Per Table 503 Height and Area Limitations.						
Type IIIB						
B and S1 Limits are 5 and 2 stories, respectively - we have 2.				Okay		
Areas for B limited to 19000 sqft. Area for S-1 is 17500 sqft.				Okay		
We have less than 10,000 sqft.				Okay		
Per Table 601 Fire-Resistance Rating Requiements for Building Elements (hours)						
Type III-B Unsprinkled						
Interior Bearing Walls	0 hour rating			Okay		
All others	0 hour rating			Okay		
Note, all Exterior Walls are Existing and not part of this work.				Okay		
Table 602 Fire-Resistance Rating Requirements for Exterior Walls Based on Separation Distances						
Type III-B Unsprinkled						
Separation	Greater than 30 feet					
Requires	0 hour for both B and S1			Okay		
Table 508.4 (separation between occupancies						
According to this table - no separation requirement is required between Business and S-1 - OKAY						

EXIT AND EMERGENCY LIGHTS

NUMBER	QTY	FLOOR	HEIGHT	ATTACHED TO	DESCRIPTION
E02	1	1	14 5/8 "	WALL	EMERGENCY LIGHT & EXIT RIGHT
E04	4	1	7 5/8 "	WALL	EXIT



First Level Life Safety Plan

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Unless otherwise noted

Sheet Description
1st Floor Life Plan

Issue Date: 21 Oct 2015 A

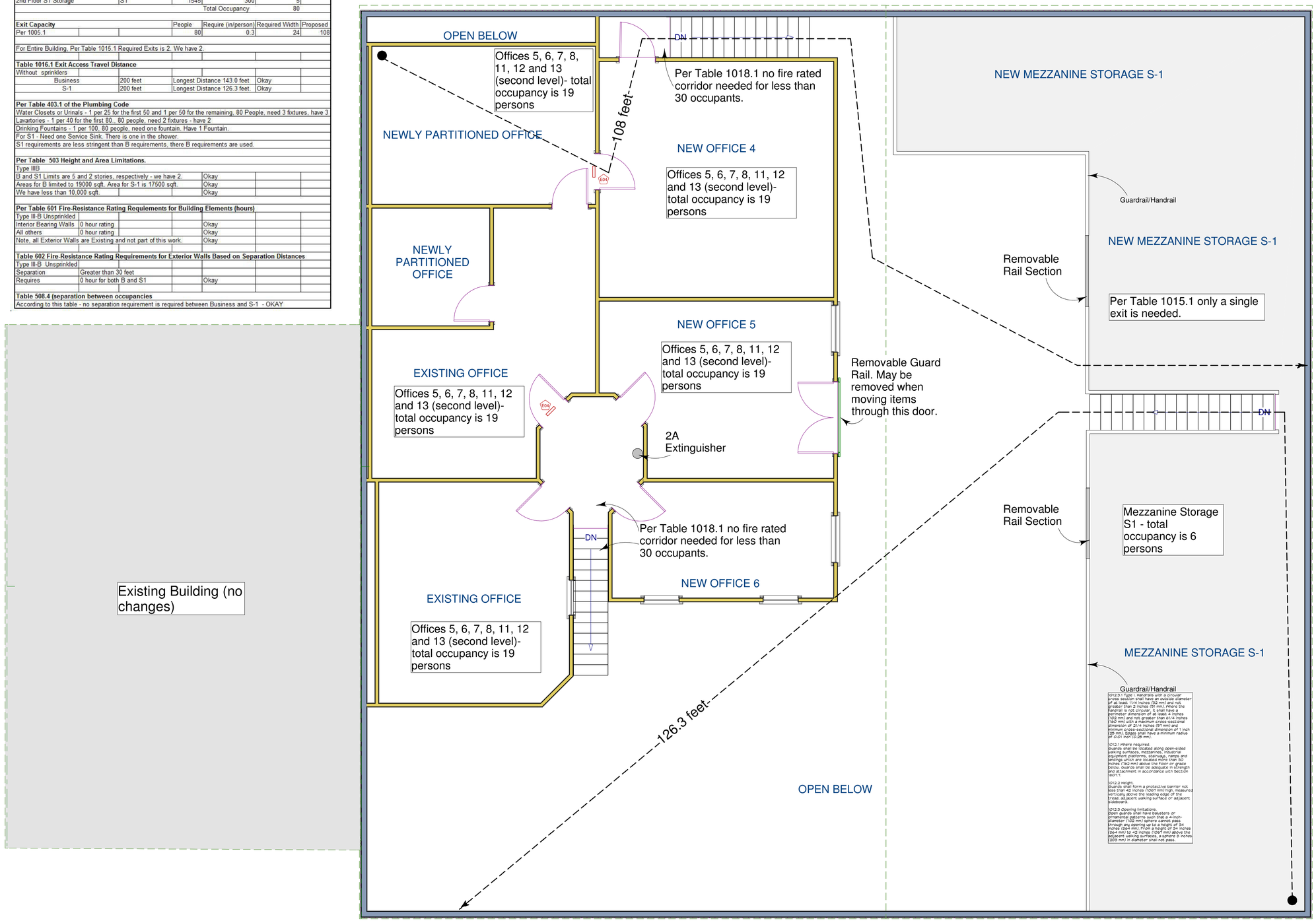
REVISION TABLE	
NUMBER	DATE

L1

OCCUPANCY, EXIT TABLE and Tables Tabulations					
Important Note: This table applies to the entire building for LIFE SAFETY.					
Per Table 1004.1.2					
Area	Occupancy	Area	Required	Capacity	
Existing Office to the South	Business	1283	100	12.83	
First Floor Offices	Business	3503	100	35.03	
First Floor S1 Storage	S1	2520	300	8	
2nd Floor Offices	Business	1860	100	19	
2nd Floor S1 Storage	S1	1545	300	5	
			Total Occupancy	80	
Exit Capacity		People	Require (in/person)	Required Width	Proposed
Per 1005.1		80	0.3	24	108
For Entire Building, Per Table 1015.1 Required Exits is 2. We have 2.					
Table 1016.1 Exit Access Travel Distance					
Without sprinklers					
Business	200 feet	Longest Distance 143.0 feet	Okay		
S-1	200 feet	Longest Distance 126.3 feet.	Okay		
Per Table 403.1 of the Plumbing Code					
Water Closets or Urinals - 1 per 25 for the first 50 and 1 per 50 for the remaining. 80 People, need 3 fixtures, have 3					
Lavartories - 1 per 40 for the first 80.. 80 people, need 2 fixtures - have 2					
Drinking Fountains - 1 per 100. 80 people, need one fountain. Have 1 Fountain.					
For S1 - Need one Service Sink. There is one in the shower.					
S1 requirements are less stringent than B requirements, there B requirements are used.					
Per Table 503 Height and Area Limitations.					
Type IIIB					
B and S1 Limits are 5 and 2 stories, respectively - we have 2.			Okay		
Areas for B limited to 19000 sqft. Area for S-1 is 17500 sqft.			Okay		
We have less than 10,000 sqft.			Okay		
Per Table 601 Fire-Resistance Rating Requiements for Building Elements (hours)					
Type III-B Unsprinkled					
Interior Bearing Walls	0 hour rating		Okay		
All others	0 hour rating		Okay		
Note, all Exterior Walls are Existing and not part of this work.			Okay		
Table 602 Fire-Resistance Rating Requirements for Exterior Walls Based on Separation Distances					
Type III-B Unsprinkled					
Separation	Greater than 30 feet				
Requires	0 hour for both B and S1		Okay		
Table 508.4 (separation between occupancies					
According to this table - no separation requirement is required between Business and S-1 - OKAY					

EXIT AND EMERGENCY LIGHTS					
NUMBER	QTY	FLOOR	HEIGHT	ATTACHED TO	DESCRIPTION
E04	2	2	7 5/8 "	WALL	EXIT

All Spaces are accessory to one another. This is a one-tenant building.



Existing Building (no changes)

Second Level Life Safety Plan

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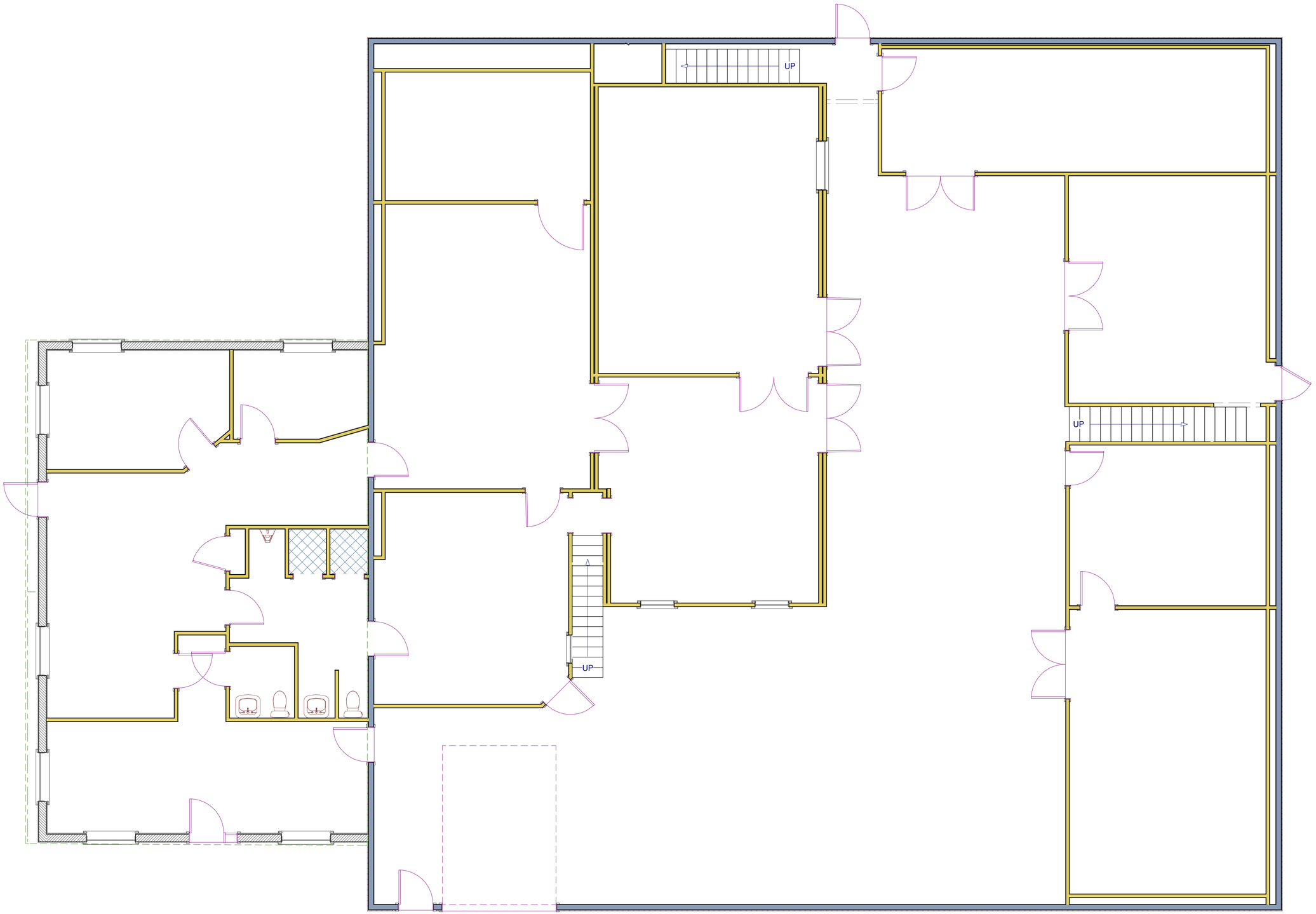
Sheet Description
2nd Floor Life Plan

Issue Date: 21 Oct 2015 A

REVISION TABLE	
NUMBER	DATE

L2



First Floor Electrical and HVAC

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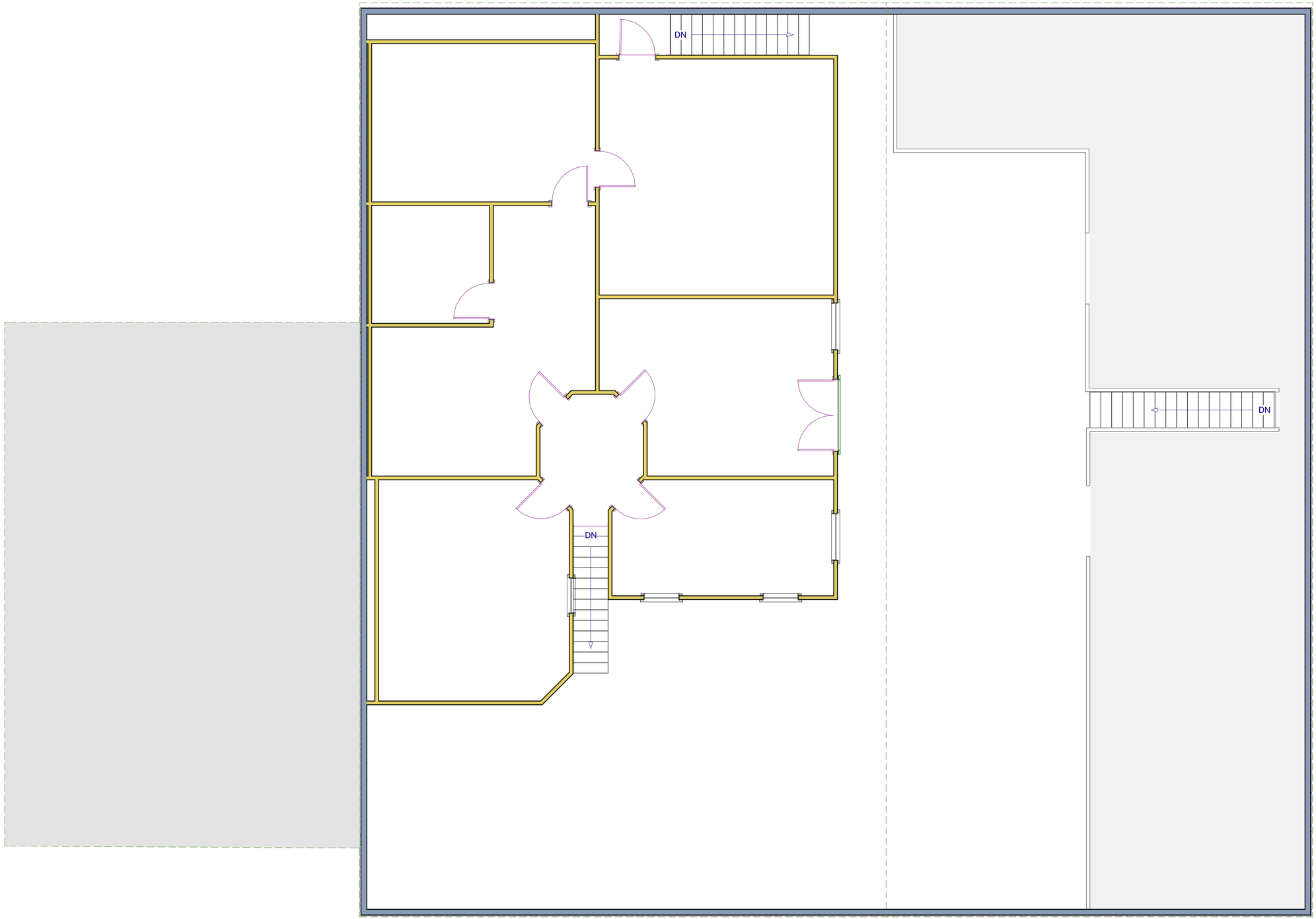
Sheet Description
Mechanical/Electrical

Issue Date: 21 Oct 2015 A

REVISION TABLE

NUMBER	DATE

M1



Second Floor Electrical and HVAC

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Mechanical/Electrical

Issue Date: 21 Oct 2015 A

REVISION TABLE	
NUMBER	DATE

M2