

CODE COMPLIANCE DATA	FBC 2010 REQUIRED	PROVIDED
OCCUPANCY CLASSIFICATION	ASSEMBLY A-2	ASSEMBLY A-2
OCCUPANCY LOAD	SEE CALCULATIONS	111
REQUIRED EXITS	2	2
EXIT CAPACITY	97 X 0.2" = 19.4"	68"
MAIN ENTRY EGRESS	32"	34"
EGRESS CORRIDOR WIDTH	N/A	N/A
DOOR OPENING WIDTH	32"	34"
DEAD END	N/A	N/A
COMMON PATH OF TRAVEL	<100'	<100'
EMERGENCY ILLUMINATION	NONE REQUIRED	PROVIDED
TENANT SEPARATION	2-HOURS	2-HOURS

BUSINESS OCCUPANCY GENERAL CALCULATIONS: PER FBC 2010, 1004.1		
TOTAL AREA OF "A-2" OCCUPANCY:	1990 SQ FT	TOTAL LOADS:
1ST FLOOR DINING AREA (UNCONCENTRATED):	1333 SQ FT / 15 SQ FT PER PERSON	89 PERSONS
1ST FLOOR KITCHEN AREA:	255 SQ FT / 200 SQ FT PER PERSON	2 PERSONS
1ST FLOOR RESTROOMS:	303 SQ FT / 50 SQ FT PER PERSON	6 PERSONS
1ST FLOOR COMPUTED OCCUPANCY LOAD FOR GROUP "A-2":		97 PERSONS
2ND FLOOR OFFICES:	4 FIXED SEATS	4 PERSONS
2ND FLOOR FILE STORAGE:	144 SQ FT / 300 SQ FT PER PERSON	1 PERSONS
TOTAL 2ND FLOOR LOAD:		5 PERSONS
TOTAL COMPUTED OCCUPANCY LOAD FOR GROUP "A-2":		102 PERSONS

	NO.	CLASSIFICATION	OCCUPANCY	WATER CLOSETS		LAVATORIES		BATHTUBS/SHOWERS	DRINKING FOUNTAIN ^{e,f}	OTHER
				MALE	FEMALE	MALE	FEMALE			
REQUIRED PER FBC 2010 403.1, 403.2, AND 403.3	1	A-2 ^d	RESTAURANTS, BANQUET HALLS AND FOOD COURTS	1 PER 75	1 PER 75	1 PER 200			1 PER 500	1 SERVICE SINK
PROVIDED	1	A-2 ^d	RESTAURANTS, BANQUET HALLS AND FOOD COURTS	2 PROVIDED	2 PROVIDED	2 PROVIDED	2 PROVIDED		N/A PER FBC 2010 410.1	1 PROVIDED

GENERAL NOTE:

OCCUPANCY LOAD AS COMPUTED FOR 2ND FLOOR RELIEVES THE OBLIGATION FOR VERTICAL ACCESSIBILITY REQUIREMENTS PER FBC 2014 ACCESSIBILITY 201.1.1.3

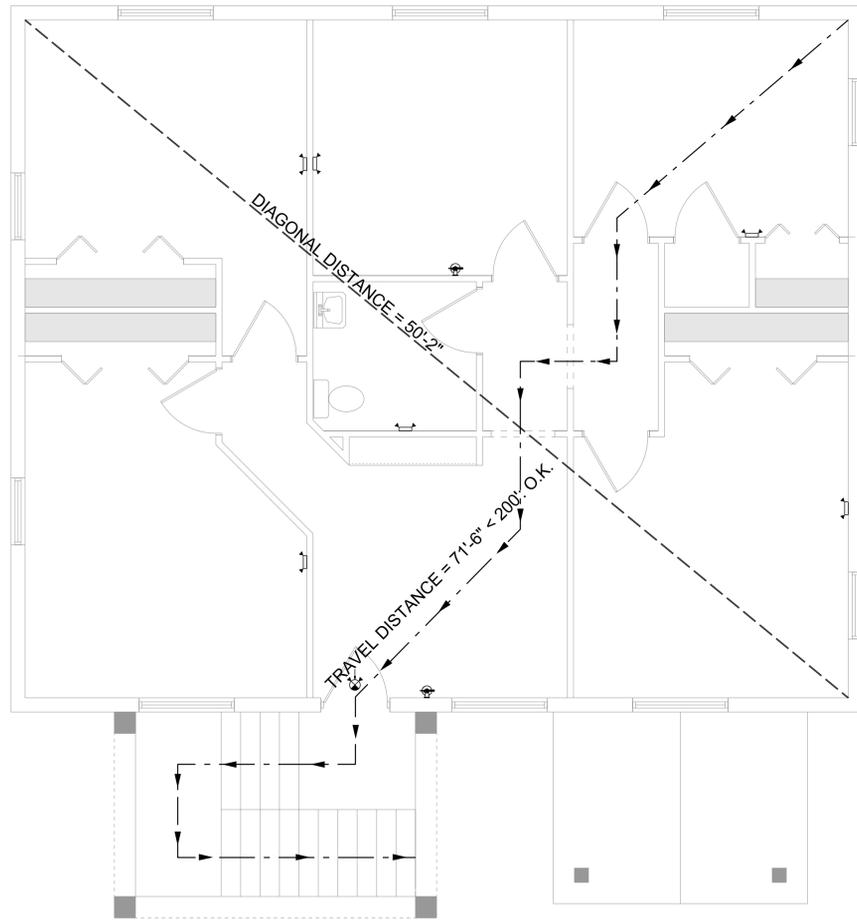
3

LIFE SAFETY LEGEND

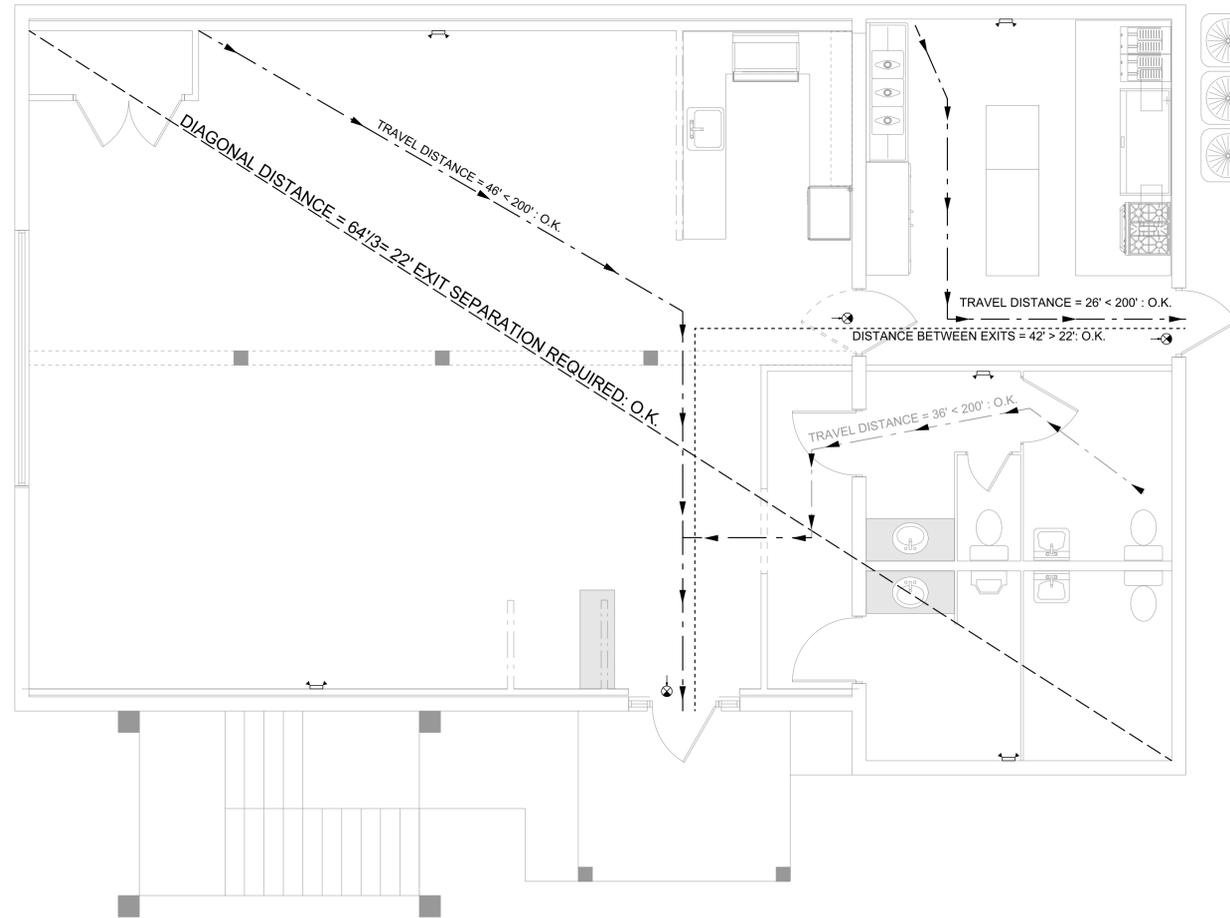
ALL DIMENSION ARE ON CENTER FROM FINISHED INTERIOR

SYMBOL	DESCRIPTION
▶	DIRECTION OF TRAVEL
---	DIAGONAL DISTANCE
---	DISTANCE BETWEEN EXITS
→	TRAVEL PATH
☒	FIRE EXTINGUISHED: TYPE 2A10 BC
⬆	EMERGENCY LIGHTING: WALL MOUNT W/ BATTERY BACK UP
⊗	LIGHTED EXIT SIGN & DUAL EXIT LIGHTING COMBO: SINGLE DIRECTION W/ BATTERY BACKUP
⊗	LIGHTED EXIT SIGN: SINGLE SIDED, FROM SINGLE DIRECTION W/ BATTERY BACKUP
⊗	LIGHTED EXIT SIGN: TWO SIDED, FROM DUAL DIRECTION W/ BATTERY BACKUP

****NOT ALL SYMBOLS MAY APPLY TO THESE PLANS****



2ND FLOOR LIFE SAFETY PLAN
SCALE: 1/4" = 1'-0"



1ST FLOOR LIFE SAFETY PLAN
SCALE: 1/4" = 1'-0"



WRITTEN DIMENSIONS ON THESE DOCUMENTS SHALL TAKE PRECEDENCE OVER ALL SCALED DIMENSIONS. CONTRACTORS ARE TO VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS, SETBACKS & CONDITIONS AT THE JOB SITE. ENGINEERS/DESIGNERS SHALL BE NOTIFIED OF ANY VARIATIONS IN DIMENSIONS, SETBACKS AND/OR CONDITIONS BEFORE SAID CONSTRUCTION IS AUTHORIZED. SHOP DRAWINGS SHALL BE MARKED "REVIEWED BY THE ENGINEER/DESIGNER" BEFORE FABRICATION OF PARTS IS APPROVED.

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SHEET DETAILS

- LIFE SAFETY PLAN A-2
- LIFE SAFETY PLAN B
- OCCUPANCY LOAD CALCS
- PLUMBING FIXTURE CALCS

REVISION DATE:
7/17/2015
10/05/2015

CLIENT:
MCHARDY INVESTMENTS
432 SE MLK BLVD
STUART, FL 34994
PIN# 04-38-41-015-009-00010-0

PROJECT:
M.C.'S GRILLE
&
OFFICE

DRAWN
S.M.K.
CHECKED
JK
DATE
April 18, 2014
SCALE
AS NOTED
JOB NO.
MCH-2013-48

SHEET
LS-1

STRUCTURAL NOTES:

NOTE: NOT ALL NOTES MAY APPLY TO THESE PLANS

GENERAL INFORMATION:

1. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY OF DETERMINING THE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES, BUT IS NOT LIMITED TO, TEMPORARY BRACING, GUYS, FALSE WORK OR CRIBBING THAT MAY BE NECESSARY TO STABILIZE THE STRUCTURE DURING ERECTION, UNDER ALL LOADING CONDITIONS. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.
2. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS FOR INSERTS, SLEEVES, CURBS, PADS, ETC, THAT AFFECT STRUCTURAL WORK.
3. CONTRACTOR SHALL VERIFY ALL EXISTING UTILITIES SHOWN IN THE CIVIL SHEETS PRIOR TO THE EXCAVATION FOR THE BUILDING FOUNDATIONS. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED OF POTENTIAL CONFLICTS BETWEEN THE FOUNDATIONS AND ANY BURIED UTILITY PIPES.
4. CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO ORDERING ANY MATERIALS, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER WHERE CONDITIONS MAY VARY FROM WHAT ARE SHOWN ON THE DRAWINGS.
5. WHERE A DETAIL IS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS, IT SHALL APPLY TO ALL SAME OR SIMILAR CONDITIONS ON THE PROJECT.
6. THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR THE DESIGN OF STEEL STAIRS, HANDRAILS, CURTAIN WALLS, WINDOWS, PRECAST CLADDING, COLD-FORMED METAL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY OTHERS AS REQUIRED IN OTHER PORTIONS OF THE CONTRACT DOCUMENTS.

FOUNDATIONS:

1. FOUNDATIONS ARE DESIGNED FOR AN ASSUMED SOIL BEARING PRESSURE OF 2500 PSF.
2. A GEOTECHNICAL ENGINEER RETAINED BY THE OWNER SHALL INSPECT ALL FOUNDATION WORK TO CONFIRM THAT REQUIRED SOIL BEARING CAPACITIES ARE MET, THE GEOTECHNICAL ENGINEER SHALL FURNISH A SIGNED AND SEALED SUMMARY REPORT STATING THAT THE FOUNDATION PREPARATION PROVIDES THE REQUIRED BEARING CAPACITIES FOR FOUNDATIONS.
3. THE PROJECT SITE SHALL BE IMPROVED WITH VIBRATORY ROLLING COMPACTION FROM THE STRIPPED GRADE PRIOR TO CONSTRUCTION OF THE FOUNDATION PAD. THE TOP 2 FEET BELOW STRIPPED GRADE SHALL BE COMPACTED TO A MINIMUM OF 95% DENSITY PRIOR TO PLACING FILL TO ACHIEVE FINAL GRADE.
4. AREAS UNDER SLABS AND FOOTINGS TO BE BACK FILLED SHALL BE COMPACTED IN LIFTS OF NOT MORE THAN TWELVE INCHES UNCOMPACTED DEPTH (SIX INCHES UNCOMPACTED DEPTH IF HAND OPERATED EQUIPMENT IS USED) TO 95% OF THE MAXIMUM DRY DENSITY AS DEFINED BY ASTM 0698.

SPECIALTY ENGINEERED SYSTEMS:

1. THE DESIGN WIND PRESSURES FOR EXTERIOR WINDOWS AND DOORS ARE SUPPLIED ON THE ELEVATION DRAWINGS. THE CONTRACTOR SHALL SUBMIT PRODUCT CONTROL APPROVALS TO THE ENGINEER SHOWING THAT THE DOORS, WINDOWS AND OTHER CLADDING SYSTEMS MEET THE DESIGN PRESSURES AND DEBRIS IMPACT CRITERIA FOR THE CONSTRUCTION PROJECT PER FBC REQUIREMENTS.

PLYWOOD ROOF SHEATHING:

1. PLYWOOD ROOF SHEATHING SHALL BE MIN 5/8" CDX GRADE, SHEATHING, WITH 48/24 APA SPAN RATING.
2. CONTINUOUS WOOD BLOCKING SHALL BE PROVIDED ALONG RIDGE AND AT ALL EDGE SUPPORTS @ ALL GABLES. BLOCKING SHALL CONSIST OF 2X4 AT PANEL JOINTS WITH (2) 16d TOE NAILS AT EACH END.
3. LAY PANELS, CONTINUOUS OVER TWO OR MORE SPANS AND WITH FACE GRAIN PERPENDICULAR TO PRIMARY FRAMING MEMBERS END JOINTS SHALL OCCUR AT CENTER OF PRIMARY FRAMING MEMBER WITH BOTH PANELS FASTENED TO II END JOINTS SHALL BE STAGGERED.
4. FASTEN PLYWOOD ROOF SHEATHING PANELS TO ALL SUPPORTING MEMBERS USING 8d GALV. RING SHANK NAILS. NAIL SPACING SHALL BE AS FOLLOWS:
ALL ROOF AREAS: 4" O.C. AT PANEL PERIMETER AND 6" O.C. AT INTERMEDIATE SUPPORTS

REINFORCED CONCRETE:

1. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 301, LATEST EDITION, SPECIFICATIONS FOR STRUCTURAL CONCRETE, AS WELL AS ACI 318, LATEST EDITION.
2. UNLESS NOTED OTHERWISE, DETAILS OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 315, MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES, AND CRSI MSP-1, MANUAL OF STANDARD PRACTICE, LATEST EDITIONS.
3. SHOP DRAWINGS SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING OF THE REINFORCING STEEL AND ACCESSORIES SHALL BE SUBMITTED FOR REVIEW TO THE ARCHITECT/ENGINEER. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.
4. SUBMIT CONCRETE MATERIALS, MIX DESIGN AND METHODS OF CURING FOR REVIEW BY THE ENGINEER.
5. TESTING LABORATORY EMPLOYED BY THE OWNER SHALL SUBMIT ONE COPY OF ALL CONCRETE TEST REPORTS DIRECTLY TO THE ENGINEER. SEE SPECIAL INSPECTION SCHEDULE FOR REQUIRED TESTS.
6. UNLESS NOTED OTHERWISE, 28 DAY COMPRESSIVE STRENGTH OF CAST-IN-PLACE CONCRETE FOR EACH PORTION OF THE STRUCTURE SHALL BE AS FOLLOWS:

FOOTINGS, SLAB ON GRADE	=	3,000 PSI
BEAMS COLUMNS AND SLABS ABOVE GRADE	=	4,000 PSI
7. UNLESS NOTED OTHERWISE, MINIMUM CONCRETE COVER FOR REINFORCEMENT SHALL BE AS FOLLOWS:

CONCRETE CAST AGAINST EARTH	3"
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THROUGH #18 BARS	2"
#5 BARS AND SMALLER	1 1/2"
INTERIOR SLABS AND WALLS W/ #11 BAR AND SMALLER	3/4"
INTERIOR BEAMS, AND COLUMNS:	1 1/2"
8. CONCRETE SLUMP SHALL BE 3" MIN. TO 5" MAX.
9. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 U.N.O.
10. ALL SPLICES SHALL CONFORM TO ACI 318, CLASS "B" SPLICES UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS OF 30"x30" FOR TOP AND BOTTOM BARS AT ALL CORNERS OF THE BEAMS AND FOUNDATIONS.
11. CONSTRUCTION JOINTS SHALL FALL AT CONTROL JOINTS AND SHALL BE KEYED.
12. WELDING OR TACK WELDING OF REINFORCING STEEL SHALL NOT BE PERMITTED EXCEPT AS AUTHORIZED OR DIRECTED BY THE STRUCTURAL ENGINEER OR HIS REP.
13. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 AND SHALL BE SUPPLIED IN SHEETS. LAP SHEETS AT LEAST 8" TO CONTAIN AT LEAST ONE CROSS WIRE WITHIN 8" OF CENTRAL JOINT.
14. PROVIDE 2 #4 (X3'-0" LONG) BARS AT ALL RE-ENTRANT CORNERS OF ALL SLABS ON GRADE AND UPPER FLOOR SLABS.
15. PROVIDE CORNER BARS MATCHING THE MAIN REINFORCEMENT AT ALL FOOTING AND THE BEAM INTERSECTIONS. THE LENGTH OF CORNER BARS IN EACH DIRECTION SHALL BE A MINIMUM OF 30 BAR DIAMETERS.
16. IF REQUIRED, HOUSEKEEPING PADS SHALL BE 4" THICK CONCRETE CAST OVER THE FLOOR SLAB. REINFORCE WITH 6X6 W2.1XW2.1 PLACED AT CENTER OF SLAB. COORDINATE SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

MASONRY:

1. MATERIALS, TESTING OF MATERIALS AND STORAGE OF MATERIALS SHALL CONFORM TO ACI STANDARD SPECIFICATIONS FOR MASONRY STRUCTURES AND BUILDING CODE REQUIREMENTS (ACI 530-99 AND 530.1-99).
2. ALL CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90. MORTAR SHALL BE TYPE M OR S COARSE MASONRY GROUT SHALL CONFORM TO ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AND A SLUMP OF ±10". MINIMUM 28 DAY COMPRESSIVE STRENGTH OF THE MASONRY SHALL BE 2,000 PSI.
3. ALL MASONRY CONSTRUCTION SHALL BE RUNNING BOND. MORTAR JOINTS SHALL NOT EXCEED 5/8" THICKNESS IN CONCEALED AREAS AND WITHIN SPECIFIED TOLERANCES IN AREAS EXPOSED TO VIEW. FOUNDATIONS WHERE VERTICAL AND HORIZONTAL ALIGNMENT WOULD CAUSE MORTAR JOINTS TO BE LESS THAN 1/4" OR MORE THAN 1/2" THICK SHALL BE CORRECTED PRIOR TO COMMENCING MASONRY CONSTRUCTION.
4. PIPES OR CONDUITS MAY PENETRATE HORIZONTALLY THROUGH MASONRY WALLS BY MEANS OF A GALVANIZED STEEL SLEEVE NO THINNER THAN STANDARD WEIGHT (SCHEDULE 40), ASTM A53, SOLIDLY GROUTED OR MORTARED IN PLACE. PLACE SLEEVES NOT CLOSER THAN THREE DIAMETERS ON CENTER. MAXIMUM SIZE OF SLEEVE SHALL BE 12".
5. MAXIMUM SIZE OF EMBEDDED VERTICAL PIPE OR CONDUIT IN A BEARING WALL SHALL NOT EXCEED 1/3 OF WALL THICKNESS. PIPES SHALL NOT BE PLACED CLOSER THAN THREE DIAMETERS ON CENTER.
6. HORIZONTAL JOINT REINFORCEMENT IN ALL CMU WALLS SHALL BE 9 GA. DEFORMED GALVANIZED TRUSS TYPE. JOINT REINFORCEMENT SHALL BE FULLY EMBEDDED IN MORTAR WITH A MINIMUM COVER OF 5/8". HORIZONTAL MASONRY REINFORCEMENT SHALL BE PLACED IN EVERY OTHER COURSE UNLESS OTHERWISE NOTED.
7. SEE PLANS FOR VERTICAL REINFORCEMENT IN WALLS. PROVIDE MINIMUM LAP OF 48 BAR DIAMETERS. FILL ALL REINFORCED CELLS WITH GROUT FOR FULL HEIGHT ALL CELLS LOCATED BELOW GRADE SHALL BE FILLED WITH GROUT REINFORCE CELLS ADJACENT TO DOOR, WINDOW AND OPENINGS IN ACCORDANCE WITH CMU OPENING REINFORCING DETAIL. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI.
8. ALIGN VERTICAL CORES EVENLY FOR THE FULL HEIGHT WHERE CELLS ARE TO BE GROUTED. GROUT IN VERTICAL CORES SHALL NOT BE PLACED IN LIFTS GREATER THAN 5 FEET PROVIDE CLEAN OUTS WHEN GROUTING BLOCK CELLS.
9. MASONRY WALLS MUST BE BRACED DURING ERECTION FOR WIND AND CONSTRUCTION LOADS. BRACES MUST BE DESIGNED, CONSTRUCTED AND ERECTED IN A FASHION AS TO PREVENT PERMANENT SCARRING OF EXPOSED MASONRY SURFACES.
10. ALL MASONRY WALLS SHOWN ON STRUCTURAL DRAWINGS SHALL BE IN PLACE PRIOR TO FORMING OR PLACING CONCRETE IN COLUMNS AND BEAMS OVER THE WALLS, EXCEPT AS OTHERWISE SPECIFIED. HORIZONTAL JOINT REINFORCING SHALL EXTEND A MINIMUM OF 6" INTO ALL BUTTING COLUMNS

PREFABRICATED WOOD TRUSSES AND GIRDERS:

1. PREFABRICATED WOOD TRUSSES AND GIRDERS SHALL BE DESIGNED IN ACCORDANCE WITH ASCE 7-10.
2. PREFABRICATED WOOD TRUSSES AND GIRDERS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE TPI AND THE FBC FOR THE LOADS INDICATED ABOVE (LATEST EDITIONS).
3. PREFABRICATED TRUSSES AND GIRDERS SHALL BE DESIGNED BY A FLORIDA REGISTERED SPECIALTY ENGINEER. COMPLETE CALCULATIONS AND SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE SPECIALTY ENGINEER AND SUBMITTED TO THE ENGINEER-OF-RECORD FOR REVIEW. REVIEW BY THE ENGINEER-OF-RECORD WILL BE FOR COMPLIANCE WITH DESIGN CONCEPT ONLY. SPECIALTY ENGINEER SHALL COMPLY WITH CHAPTER 471 F.S.
4. TRUSS MANUFACTURER IS RESPONSIBLE FOR THE DESIGN AND FABRICATION OF TRUSSES, GIRDERS, AND THEIR CONNECTIONS TO EACH OTHER. DESIGN OF BRACING FOR INDIVIDUAL TRUSS OR GIRDER MEMBERS TO RESIST LATERAL BUCKLING UNDER GRAVITY OR WIND LOADS IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER.
5. TRUSS AND GIRDER LAYOUT AS INDICATED ON THE ROOF FRAMING AND/OR FLOOR FRAMING PLAN(S) SHALL NOT BE ALTERED WITHOUT PRIOR WRITTEN AUTHORIZATION FROM THE ENGINEER OF RECORD.
6. SUBMIT SIGNED & SEALED SHOP DRAWINGS FOR REVIEW AND APPROVAL BY THE A/E.
7. WOOD FRAMING SHALL BE SOUTHERN PINE No. 2 UNLESS SPECIFIED OTHERWISE BY TRUSS MANUFACTURER

STRUCTURAL STEEL:

1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, LATEST EDITIONS.
2. SHOP DRAWINGS PREPARED IN ACCORDANCE WITH THE LATEST "STRUCTURAL STEEL DETAILING MANUAL OF THE AISC" SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN FLORIDA, AND SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.
3. UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SECTIONS SHALL CONFORM TO THE FOLLOWING:
 - A. W-SHAPES SHALL CONFORM TO ASTM A992, GRADE 50. (ASTM A 572, GRADE 50 MAY BE SUBSTITUTED FOR ASTM A992.)
 - B. SQUARE/RECTANGULAR HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500, GRADE B
 - C. ROUND HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A501 OR ASTM A53, GRADE B.
 - D. OTHER STEEL SHAPES (CHANNELS, ANGLES, AND PLATES) MAY CONFORM TO ASTM A36
4. BEAMS WITHOUT SPECIFIED CAMBER ARE TO BE FABRICATED SUCH THAT AFTER ERECTION ANY CAMBER DUE TO ROLLING OR SHOP FABRICATION IS UPWARD AT MID SPAN.
5. BOLTS, WELDS, ANCHORS, AND HEADED STUDS:
 - A. WELDS SHALL CONFORM TO "STRUCTURAL WELDING CODE" OF THE AMERICAN WELDING SOCIETY, AWS D11. USE E70XX ELECTRODES. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE. MINIMUM WELD SIZE IS ACCORDING TO AISC, LATEST EDITION.
 - B. UNLESS NOTED OTHERWISE BOLTS SHALL CONFORM TO ASTM A325, AND SHALL BE MINIMUM 3/4" DIAMETER.
 - C. UNLESS NOTED OTHERWISE STUDS SHALL BE 3/4" DIA. 5-3/16" LONG HEADED STUDS, BY NELSON, OR APPROVED EQUAL.
 - D. UNLESS NOTED OTHERWISE ALL EXPANSION ANCHORS ARE TO BE 3/4" DIA. HILTI KWIK - BOLTS WITH 4 3/4" MIN. EMBEDMENT, OR AN APPROVED EQUAL.
 - E. ANCHOR BOLTS SHALL CONFORM TO ASTM A307 AND SHALL BE GALV. IF EXPOSED IN THE FINISHED STRUCTURE.
6. THE FABRICATOR SHALL BE RESPONSIBLE FOR THE DESIGN AND DETAILING OF ALL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE CONTRACT DRAWINGS. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. SHOP DRAWINGS AND CALCULATIONS OF SUCH CONNECTIONS SHALL BE SEALED BY AN ENGINEER LICENSED IN FLORIDA. REVIEW BY THE E.O.R. DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH.
7. UNGALVANIZED PIECES EXCEPT AT AREAS TO BE FIELD WELDED. TOUCH UP FIELD WELDS AND ANY DAMAGED AREAS OF PAINT IN FIELD AFTER WELDING (USE GALVANIZED PAINT FOR TOUCH UP OF GALVANIZED STEEL). GALVANIZED PAINT SHALL BE IN ACCORDANCE WITH THE FDOT QUALIFIED PRODUCTS LIST.

WRITTEN DIMENSIONS ON THESE DOCUMENTS SHALL TAKE PRECEDENCE OVER ALL SCALED DIMENSIONS. CONTRACTORS ARE TO VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONAL SETBACKS & CONDITIONS AT THE JOB SITE. ENGINEER/DESIGNER SHALL BE NOTIFIED OF ANY VARIATIONS IN DIMENSIONAL SETBACKS/AND/OR CONDITIONS BEFORE SAID CONSTRUCTION IS AUTHORIZED. SHOP DRAWINGS SHALL BE MARKED "REVIEWED BY THE ENGINEER/DESIGNER" BEFORE FABRICATION OF PARTS IS APPROVED.



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