**Florida Supplement to the 2012 IRC**

**POST-GLITCH CORRECTION VERSION**

**Note 1**: Throughout the document, change International Building Code to Florida Building Code, Building; change the ICC Electrical Code to Chapter 27 of the Florida Building Code, Building; change the International Energy Conservation Code tothe Florida Building Code, Energy Conservation; change the International Existing Building Code to Florida Building Code, Existing Building; change the International Fire code to Florida Fire Prevention Code; change International Fuel Gas Code to Florida Building Code, Fuel Gas; change the International Mechanical Code to Florida Building Code, Mechanical; change the International Plumbing Code to Florida Building Code, Plumbing; change the International Residential Code to Florida Building Code, Residential.

**PREFACE**

**~~Introduction~~**

**~~Development~~**

**History**

The State of Florida first mandated statewide building codes during the 1970s at the beginning of the modern construction boom. The first law required all municipalities and counties to adopt and enforce one of the four state-recognized model codes known as the “state minimum building codes.” During the early 1990s a series of natural disasters, together with the increasing complexity of building construction regulation in vastly changed markets, led to a comprehensive review of the state building code system. The study revealed that building code adoption and enforcement was inconsistent throughout the state and those local codes thought to be the strongest proved inadequate when tested by major hurricane events. The consequences of the building codes system failure were devastation to lives and economies and a statewide property insurance crisis. The response was a reform of the state building construction regulatory system that placed emphasis on uniformity and accountability.

The 1998 Florida Legislature amended Chapter 553, *Florida Statutes* (FS), Building Construction Standards, to create a single state building code that is enforced by local governments. As of March 1, 2002, the *Florida Building Code*, which is developed and maintained by the Florida Building Commission, supersedes all local building codes. The *Florida Building Code* is updated every three years and may be amended annually to incorporate interpretations and clarifications.

**Scope**

The *Florida Building Code* is based on national model building codes and national consensus standards which are amended where necessary for Florida’s specific needs. However, code requirements that address snow loads and earthquake protection are pervasive; they are left in place but should not be utilized or enforced because Florida has no snow load or earthquake threat. The code incorporates all building construction-related regulations for public and private buildings in the State of Florida other than those specifically exempted by Section 553.73, *Florida Statutes*. It has been harmonized with the *Florida Fire Prevention Code*, which is developed and maintained by the Department of Financial Services, Office of the State Fire Marshal, to establish unified and consistent standards.

The base codes for the Fifth edition (2014) of the *Florida Building Code* include: the International Building Code®, 2012 edition; the International Plumbing Code®, 2012 edition; the International Mechanical Code®, 2012 edition; the International Fuel Gas Code®, 2012 edition; the International Residential Code®, 2012 edition; the International Existing Building Code®, 2012 edition; the International Energy Conservation Code, 2012; the National Electrical Code, 2011 edition; substantive criteria from the American Society of Heating, Refrigerating and Air-conditioning Engineers’ (ASHRAE) Standard 90.1-2010. State and local codes adopted and incorporated into the code include the *Florida Building Code, Accessibility,* and special hurricane protection standards for the High-Velocity Hurricane Zone.

The code is composed of nine main volumes: the *Florida Building Code, Building*, which also includes state regulations for licensed facilities; the *Florida Building Code, Plumbing*; the *Florida Building Code, Mechanical;* the *Florida Building Code, Fuel Gas*; the *Florida Building Code, Existing Building*; the *Florida Building Code, Residential;* the *Florida Building Code, Energy Conservation*; the *Florida Building Code, Accessibility* and the *Florida Building Code, Test Protocols for High-Velocity Hurricane Zones*. Chapter 27 of the *Florida Building Code, Building*, adopts the *National Electrical Code*, NFPA 70, by reference.

Under certain strictly defined conditions, local governments may amend requirements to be more stringent than the code. All local amendments to the *Florida Building Code* must be adopted by local ordinance and reported to the Florida Building Commission then posted on [www.floridabuilding.org](http://www.floridabuilding.org) in Legislative format for a month before being enforced. Local amendments to the *Florida Building Code* and the *Florida Fire Prevention Code* may be obtained from the Florida Building Commission web site, or from the Florida Department of Business and Professional Regulation or the Florida Department of Financial Services, Office of the State Fire Marshal, respectively.

**Adoption and Maintenance**

**[Note to editor: Replace ICC “Adoption” and “Maintenance” with the following text:]**

The *Florida Building Code* is adopted and updated with new editions triennially by the Florida Building Commission. It is amended annually to incorporate interpretations, clarifications and to update standards. Minimum requirements for permitting, plans review and inspections are established by the code, and local jurisdictions may adopt additional administrative requirements that are more stringent. Local technical amendments are subject to strict criteria established by Section 553.73, *F.S.* They are subject to Commission review and adoption into the code or repeal when the code is updated triennially and are subject to appeal to the Commission according to the procedures established by Section 553.73, *F.S*.

Eleven Technical Advisory Committees (TACs), which are constituted consistent with American National Standards Institute (ANSI) Guidelines, review proposed code changes and clarifications of the code and make recommendations to the Commission. These TACs whose membership is constituted consistent with American National Standards Institute (ANSI) Guidelines include: Accessibility; Joint Building Fire (a joint committee of the Commission and the State Fire Marshal); Building Structural; Code Administration/ Enforcement; Electrical; Energy; Mechanical; Plumbing and Fuel Gas; Roofing; Swimming Pool; and Special Occupancy (state agency construction and facility licensing regulations).

The Commission may only issue official code clarifications using procedures of Chapter 120, *Florida Statutes*. To obtain such a clarification, a request for a Declaratory Statement (DEC) must be made to the Florida Building Commission in a manner that establishes a clear set of facts and circumstances and identifies the section of the code in question. Requests are analyzed by staff, reviewed by the appropriate Technical Advisory Committee, and sent to the Florida Building Commission for action. These interpretations establish precedents for situations having similar facts and circumstances and are typically incorporated into the code in the next code amendment cycle. Non-binding opinions are available from the Building Officials Association of Florida’s web site (www.BOAF.net) and a Binding Opinion process is available online at www.floridabuilding.org.

**Code Development Committee Responsibilities (Letter Designations in Front of Section Numbers)**

**[Note to editor: Use paragraphs 1 specific to this code through the code committee descriptors. Delete the remaining text in this section.]**

**Marginal Markings**

Solid vertical lines in the margins within the body of the code indicate a technical change from the requirements of the 2009 edition. Deletion indicators in the form of an arrow (**→**) are provided in the margin where an entire section, paragraph, exception or table has been deleted or an item in a list of items or table has been deleted.

A single asterisk [\*] placed in the margin indicates that text or a table has been relocated within the code. A double asterisk [\*\*] placed in the margin indicates that the text or table immediately following it has been relocated there from elsewhere in the code. ~~The following table indicates such relocations in the 2012 Edition of the~~ *~~International Residential Code~~*. **[Delete table]**

Dotted vertical lines in the margins within the body of the supplement indicate a change from the requirements of the base codes to the 2014 *Florida Building Code* effective TBD.

Sections deleted from the base code are designated “Reserved” in order to maintain the structure of the base code.

**Italicized Terms**

**[No change to I Code text.]**

**Acknowledgments**

The *Florida Building Code* is produced through the efforts and contributions of building designers, contractors, product manufacturers, regulators and other interested parties who participate in the Florida Building Commission’s consensus processes, Commission staff and the participants in the national model code development processes.

**[Note to Editor: Delete the following ICC text in its entirety:]**

**~~Effective Use of the …~~**

**~~Legislation~~**

***CHAPTER 1, SCOPE AND ADMINISTRATION***

***Section R101.2 Scope. Change to read as follows:***

**R101.2 Scope.** The provisions of the *Florida Building* *Code, Residential,* shall apply to the construction, alteration, movement, enlargement, replacement, repair, equipment, use and occupancy, location, removal and demolition of detached one– and two–family dwellings and(townhouses) not more than three stories above grade plane in height with a separate means of egress and their accessory structures.

**Exceptions:**

**1-2 No change**

**3.** Existing buildings undergoing repair, alteration or additions, and change of occupancy shall comply with the *Florida Building Code, Existing Building..*

**R101.2.1** The provisions of Chapter 1, *Florida Building Code, Building,* shall govern the administration and enforcement of the *Florida Building Code, Residential.*

***Section R101.3 Intent. Change to read as follows:***

**R101.3 Intent.** ~~The purpose of this code is to establish minimum requirements to safeguard the public safety, health and general welfare through affordability, structural strength, means of egress facilities, stability, sanitation, light and ventilation, energy conservation and safety to life and property from fire and other hazards attributed to the built environment and to provide safety to fire fighters and emergency responders during emergency operations.~~ Reserved.

***Sections R102 through R114. Change to read as follows:***

**SECTION R102**

**APPLICABILITY**

**RESERVED**

**~~R102.1 General.~~** ~~Where there is a conflict between a general requirement and a specific requirement, the specific requirement shall be applicable. Where, in any specific case, different sections of this code specify different materials, methods of construction or other requirements, the most restrictive shall govern.~~

**~~R102.2 Other laws.~~** ~~The provisions of this code shall not be deemed to nullify any provisions of local, state or federal law.~~

**~~R102.3 Application of references.~~** ~~References to chapter or section numbers, or to provisions not specifically identified by number, shall be construed to refer to such chapter, section or provision of this code.~~

**~~R102.4 Referenced codes and standards.~~** ~~The codes and standards referenced in this code shall be considered part of the requirements of this code to the prescribed extent of each such reference and as further regulated in Sections R102.4.1 and R102.4.2.~~  **~~Exception:~~** ~~Where enforcement of a code provision would violate the conditions of the~~ *~~listing~~* ~~of the~~ *~~equipment~~* ~~or~~ *~~appliance,~~* ~~the conditions of the~~ *~~listing~~* ~~and manufacturer’s instructions shall apply.~~

**~~R102.4.1 Differences.~~** ~~Where differences occur between provisions of this code and referenced codes and standards, the provisions of this code shall apply.~~

**~~R102.4.2 Provisions in referenced codes and standards.~~** ~~Where the extent of the reference to a referenced code or standard includes subject matter that is within the scope of this code, the provisions of this code, as applicable, shall take precedence over the provisions in the referenced code or standard.~~

**~~R102.5 Appendices.~~** ~~Provisions in the appendices shall not apply unless specifically referenced in the adopting ordinance.~~

**~~R102.6 Partial invalidity.~~** ~~In the event any part or provision of this code is held to be illegal or void, this shall not have the effect of making void or illegal any of the other parts or provisions.~~

**~~R102.7 Existing structures.~~** ~~The legal occupancy of any structure existing on the date of adoption of this code shall be permitted to continue without change, except as is specifically covered in this code, the~~ *~~International Property Maintenance Code~~* ~~or the~~ *~~International Fire Code,~~* ~~or as is deemed necessary by the~~ *~~building official~~* ~~for the general safety and welfare of the occupants and the public.~~

**~~R102.7.1 Additions, alterations or repairs.~~** *~~Additions,~~**~~alterations~~* ~~or repairs to any structure shall conform to the requirements for a new structure without requiring the existing structure to comply with all of the requirements of this code, unless otherwise stated.~~ *~~Additions,~~**~~alterations~~* ~~or repairs shall not cause an existing structure to become unsafe or adversely affect the performance of the building.~~

**SECTION R103**

**DEPARTMENT OF BUILDING SAFETY**

**RESERVED**

**~~R103.1 Creation of enforcement agency.~~** ~~The department of building safety is hereby created and the official in charge thereof shall be known as the~~ *~~building official.~~*

**~~R103.2 Appointment.~~** ~~The~~ *~~building official~~* ~~shall be appointed by the chief appointing authority of the~~ *~~jurisdiction.~~*

**~~R103.3 Deputies.~~** ~~In accordance with the prescribed procedures of this~~ *~~jurisdiction~~* ~~and with the concurrence of the appointing authority, the~~ *~~building official~~* ~~shall have the authority to appoint a deputy~~ *~~building official,~~* ~~the related technical officers, inspectors, plan examiners and other employees. Such employees shall have powers as delegated by the~~ *~~building official.~~*

**SECTION R104**

**DUTIES AND POWERS OF THE BUILDING OFFICIAL**

**RESERVED**

**~~R104.1 General.~~** ~~The~~ *~~building official~~* ~~is hereby authorized and directed to enforce the provisions of this code. The~~ *~~building official~~* ~~shall have the authority to render interpretations of this code and to adopt policies and procedures in order to clarify the application of its provisions. Such interpretations, policies and procedures shall be in conformance with the intent and purpose of this code. Such policies and procedures shall not have the effect of waiving requirements specifically provided for in this code.~~

**~~R104.2 Applications and permits.~~** ~~The~~ *~~building official~~* ~~shall receive applications, review~~ *~~construction documents~~* ~~and issue permits for the erection and alteration of buildings and structures, inspect the premises for which such permits have been issued and enforce compliance with the provisions of this code.~~

**~~R104.3 Notices and orders.~~** ~~The~~ *~~building official~~* ~~shall issue all necessary notices or orders to ensure compliance with this code.~~

**~~R104.4 Inspections.~~** ~~The~~ *~~building official~~* ~~is authorized to make all of the required inspections, or the~~ *~~building official~~* ~~shall have the authority to accept reports of inspection by~~ *~~approved agencies~~* ~~or individuals. Reports of such inspections shall be in writing and be certified by a responsible officer of such~~ *~~approved~~* ~~agency or by the responsible individual. The~~ *~~building official~~* ~~is authorized to engage such expert opinion as deemed necessary to report upon unusual technical issues that arise, subject to the approval of the appointing authority.~~

**~~R104.5 Identification.~~** ~~The~~ *~~building official~~* ~~shall carry proper identification when inspecting structures or premises in the performance of duties under this code.~~

**~~R104.6 Right of entry.~~** ~~Where it is necessary to make an inspection to enforce the provisions of this code, or where the~~ *~~building official~~* ~~has reasonable cause to believe that there exists in a structure or upon a premises a condition which is contrary to or in violation of this code which makes the structure or premises unsafe, dangerous or hazardous, the~~ *~~building official~~* ~~or designee is authorized to enter the structure or premises at reasonable times to inspect or to perform the duties imposed by this code, provided that if such structure or premises be occupied that credentials be presented to the occupant and entry requested. If such structure or premises be unoccupied, the~~ *~~building official~~* ~~shall first make a reasonable effort to locate the owner or other person having charge or control of the structure or premises and request entry. If entry is refused, the~~ *~~building official~~* ~~shall have recourse to the remedies provided by law to secure entry.~~

**~~R104.7 Department records.~~** ~~The~~ *~~building official~~* ~~shall keep official records of applications received, permits and certificates issued, fees collected, reports of inspections, and notices and orders issued. Such records shall be retained in the official records for the period required for the retention of public records.~~

**~~R104.8 Liability.~~** ~~The~~ *~~building official,~~* ~~member of the board of appeals or employee charged with the enforcement of this code, while acting for the~~ *~~jurisdiction~~* ~~in good faith and without malice in the discharge of the duties required by this code or other pertinent law or ordinance, shall not thereby be rendered liable personally and is hereby relieved from personal liability for any damage accruing to persons or property as a result of any act or by reason of an act or omission in the discharge of official duties. Any suit instituted against an officer or employee because of an act performed by that officer or employee in the lawful discharge of duties and under the provisions of this code shall be defended by legal representative of the~~ *~~jurisdiction~~* ~~until the final termination of the proceedings. The~~ *~~building official~~* ~~or any subordinate shall not be liable for cost in any action, suit or proceeding that is instituted in pursuance of the provisions of this code.~~

**~~R104.9 Approved materials and equipment.~~** ~~Materials,~~ *~~equipment~~* ~~and devices~~ *~~approved~~* ~~by the~~ *~~building official~~* ~~shall be constructed and installed in accordance with such approval.~~

**~~R104.9.1 Used materials and equipment.~~** ~~Used materials,~~ *~~equipment~~* ~~and devices shall not be reused unless~~ *~~approved~~* ~~by the~~ *~~building official.~~*

**~~R104.10 Modifications.~~** ~~Wherever there are practical difficulties involved in carrying out the provisions of this code, the~~ *~~building official~~* ~~shall have the authority to grant modifications for individual cases, provided the~~ *~~building official~~* ~~shall first find that special individual reason makes the strict letter of this code impractical and the modification is in compliance with the intent and purpose of this code and that such modification does not lessen health, life and fire safety or structural requirements. The details of action granting modifications shall be recorded and entered in the files of the department of building safety.~~

**~~R104.10.1 Flood hazard areas.~~** ~~The~~ *~~building official~~* ~~shall not grant modifications to any provision related to flood hazard areas as established by Table R301.2(1) without the granting of a variance to such provisions by the board of appeals.~~

**~~R104.11 Alternative materials, design and methods of construction and equipment.~~**  ~~The provisions of this code are not intended to prevent the installation of any material or to prohibit any design or method of construction not specifically prescribed by this code, provided that any such alternative has been~~ *~~approved.~~* ~~An alternative material, design or method of construction shall be~~ *~~approved~~* ~~where the~~ *~~building official~~* ~~finds that the proposed design is satisfactory and complies with the intent of the provisions of this code, and that the material, method or work offered is, for the purpose intended, at least the equivalent of that prescribed in this code. Compliance with the specific performance-based provisions of the International Codes in lieu of specific requirements of this code shall also be permitted as an alternate.~~

**~~R104.11.1 Tests.~~** ~~Whenever there is insufficient evidence of compliance with the provisions of this code, or evidence that a material or method does not conform to the requirements of this code, or in order to substantiate claims for alternative materials or methods, the~~ *~~building official~~* ~~shall have the authority to require tests as evidence of compliance to be made at no expense to the~~ *~~jurisdiction.~~* ~~Test methods shall be as specified in this code or by other recognized test standards. In the absence of recognized and accepted test methods, the~~ *~~building official~~* ~~shall approve the testing procedures. Tests shall be performed by an~~ *~~approved~~* ~~agency. Reports of such tests shall be retained by the~~ *~~building official~~* ~~for the period required for retention of public records.~~

**SECTION R105**

**PERMITS**

**RESERVED**

**~~R105.1 Required.~~** ~~Any owner or authorized agent who intends to construct, enlarge, alter, repair, move, demolish or change the occupancy of a building or structure, or to erect, install, enlarge, alter, repair, remove, convert or replace any electrical, gas, mechanical or plumbing system, the installation of which is regulated by this code, or to cause any such work to be done, shall first make application to the~~ *~~building official~~* ~~and obtain the required~~ *~~permit.~~*

**~~R105.2 Work exempt from permit.~~** *~~Permits~~* ~~shall not be required for the following. Exemption from~~ *~~permit~~* ~~requirements of this code shall not be deemed to grant authorization for any work to be done in any manner in violation of the provisions of this code or any other laws or ordinances of this~~ *~~jurisdiction.~~*

**~~Building:~~**

~~1. One-story detached~~ *~~accessory structures~~* ~~used as tool and storage sheds, playhouses and similar uses, provided the floor area does not exceed 200 square feet (18.58 m~~~~2~~~~).~~

~~2. Fences not over 7 feet (2134 mm) high.~~

~~3. Retaining walls that are not over 4 feet (1219 mm) in height measured from the bottom of the footing to the top of the wall, unless supporting a surcharge.~~

~~4. Water tanks supported directly upon~~ *~~grade~~* ~~if the capacity does not exceed 5,000 gallons (18 927 L) and the ratio of height to diameter or width does not exceed 2 to 1.~~

~~5. Sidewalks and driveways.~~

~~6. Painting, papering, tiling, carpeting, cabinets, counter tops and similar finish work.~~

~~7. Prefabricated swimming pools that are less than 24 inches (610 mm) deep.~~

~~8. Swings and other playground equipment.~~

~~9. Window awnings supported by an exterior wall which do not project more than 54 inches (1372 mm) from the exterior wall and do not require additional support.~~

~~10. Decks not exceeding 200 square feet (18.58 m~~~~2~~~~) in area, that are not more than 30 inches (762 mm) above~~ *~~grade~~* ~~at any point, are not attached to a~~ *~~dwelling~~* ~~and do not serve the exit door required by~~ [~~Section R311.4.~~](javascript:Next('./icod_irc_2012_3_par153.htm');)

**~~Electrical:~~**

~~1.~~ *~~Listed~~* ~~cord-and-plug connected temporary decorative lighting.~~

~~2. Reinstallation of attachment plug receptacles but not the outlets therefor.~~

~~3. Replacement of branch circuit overcurrent devices of the required capacity in the same location.~~

~~4. Electrical wiring, devices,~~ *~~appliances,~~* ~~apparatus or~~ *~~equipment~~* ~~operating at less than 25 volts and not capable of supplying more than 50 watts of energy.~~

~~5. Minor repair work, including the replacement of lamps or the connection of~~ *~~approved~~* ~~portable electrical~~ *~~equipment~~* ~~to~~ *~~approved~~* ~~permanently installed receptacles.~~  **~~Gas:~~**

~~1. Portable heating, cooking or clothes drying~~ *~~appliances.~~*

~~2. Replacement of any minor part that does not alter approval of~~ *~~equipment~~* ~~or make such~~ *~~equipment~~* ~~unsafe.~~

~~3. Portable-fuel-cell~~ *~~appliances~~* ~~that are not connected to a fixed piping system and are not interconnected to a power grid.~~  **~~Mechanical:~~**

~~1. Portable heating~~ *~~appliances.~~*

~~2. Portable ventilation~~ *~~appliances.~~*

~~3. Portable cooling units.~~

~~4. Steam, hot- or chilled-water piping within any heating or cooling~~ *~~equipment~~* ~~regulated by this code.~~

~~5. Replacement of any minor part that does not alter approval of~~ *~~equipment~~* ~~or make such~~ *~~equipment~~* ~~unsafe.~~

~~6. Portable evaporative coolers.~~

~~7. Self-contained refrigeration systems containing 10 pounds (4.54 kg) or less of refrigerant or that are actuated by motors of 1 horsepower (746 W) or less.~~

~~8. Portable-fuel-cell~~ *~~appliances~~* ~~that are not connected to a fixed piping system and are not interconnected to a power grid.   
  
The stopping of leaks in drains, water, soil, waste or vent pipe; provided, however, that if any concealed trap, drainpipe, water, soil, waste or vent pipe becomes defective and it becomes necessary to remove and replace the same with new material, such work shall be considered as new work and a~~ *~~permit~~* ~~shall be obtained and inspection made as provided in this code.   
  
The clearing of stoppages or the repairing of leaks in pipes, valves or fixtures, and the removal and reinstallation of water closets, provided such repairs do not involve or require the replacement or rearrangement of valves, pipes or fixtures.~~

**~~R105.2.1 Emergency repairs.~~** ~~Where~~ *~~equipment~~* ~~replacements and repairs must be performed in an emergency situation, the~~ *~~permit~~* ~~application shall be submitted within the next working business day to the~~ *~~building official.~~*

**~~R105.2.2 Repairs.~~** ~~Application or notice to the~~ *~~building official~~* ~~is not required for ordinary repairs to structures, replacement of lamps or the connection of~~ *~~approved~~* ~~portable electrical~~ *~~equipment~~* ~~to~~ *~~approved~~* ~~permanently installed receptacles. Such repairs shall not include the cutting away of any wall, partition or portion thereof, the removal or cutting of any structural beam or load-bearing support, or the removal or change of any required means of egress, or rearrangement of parts of a structure affecting the egress requirements; nor shall ordinary repairs include~~ *~~addition~~* ~~to,~~ *~~alteration~~* ~~of, replacement or relocation of any water supply, sewer, drainage, drain leader, gas, soil, waste, vent or similar piping, electric wiring or mechanical or other work affecting public health or general safety.~~

**~~R105.2.3 Public service agencies.~~** ~~A~~ *~~permit~~* ~~shall not be required for the installation, alteration or repair of generation, transmission, distribution, metering or other related~~ *~~equipment~~* ~~that is under the ownership and control of public service agencies by established right.~~

**~~R105.3 Application for permit.~~** ~~To obtain a~~ *~~permit,~~* ~~the applicant shall first file an application therefor in writing on a form furnished by the department of building safety for that purpose. Such application shall:~~

~~1. Identify and describe the work to be covered by the~~ *~~permit~~* ~~for which application is made.~~

~~2. Describe the land on which the proposed work is to be done by legal description, street address or similar description that will readily identify and definitely locate the proposed building or work.~~

~~3. Indicate the use and occupancy for which the proposed work is intended.~~

~~4. Be accompanied by~~ *~~construction documents~~* ~~and other information as required in Section R106.1.~~

~~5. State the valuation of the proposed work.~~

~~6. Be signed by the applicant or the applicant’s authorized agent.~~

~~7. Give such other data and information as required by the~~ *~~building official.~~*

**~~R105.3.1 Action on application.~~** ~~The~~ *~~building official~~* ~~shall examine or cause to be examined applications for permits and amendments thereto within a reasonable time after filing. If the application or the~~ *~~construction documents~~* ~~do not conform to the requirements of pertinent laws, the~~ *~~building official~~* ~~shall reject such application in writing stating the reasons therefor. If the~~ *~~building official~~* ~~is satisfied that the proposed work conforms to the requirements of this code and laws and ordinances applicable thereto, the~~ *~~building official~~* ~~shall issue a~~ *~~permit~~* ~~therefor as soon as practicable.~~

**~~R105.3.1.1 Determination of substantially improved or substantially damaged existing buildings in flood hazard areas.~~** ~~For applications for reconstruction, rehabilitation,~~ *~~addition~~* ~~or other improvement of existing buildings or structures located in a flood hazard area as established by Table R301.2(1), the~~ *~~building official~~* ~~shall examine or cause to be examined the~~ *~~construction documents~~* ~~and shall prepare a finding with regard to the value of the proposed work. For buildings that have sustained damage of any origin, the value of the proposed work shall include the cost to repair the building or structure to its predamaged condition. If the~~ *~~building official~~* ~~finds that the value of proposed work equals or exceeds 50 percent of the market value of the building or structure before the damage has occurred or the improvement is started, the finding shall be provided to the board of appeals for a determination of substantial improvement or substantial damage. Applications determined by the board of appeals to constitute substantial improvement or substantial damage shall require all existing portions of the entire building or structure to meet the requirements of~~ [~~Section R322~~](javascript:Next('./icod_irc_2012_3_par255.htm');)~~.~~

**~~R105.3.2 Time limitation of application.~~** ~~An application for a~~ *~~permit~~* ~~for any proposed work shall be deemed to have been abandoned 180 days after the date of filing unless such application has been pursued in good faith or a~~ *~~permit~~* ~~has been issued; except that the~~ *~~building official~~* ~~is authorized to grant one or more extensions of time for additional periods not exceeding 180 days each. The extension shall be requested in writing and justifiable cause demonstrated.~~

**~~R105.4 Validity of permit.~~** ~~The issuance or granting of a~~ *~~permit~~* ~~shall not be construed to be a~~ *~~permit~~* ~~for, or an~~ *~~approval~~* ~~of, any violation of any of the provisions of this code or of any other ordinance of the~~ *~~jurisdiction.~~* ~~Permits presuming to give authority to violate or cancel the provisions of this code or other ordinances of the~~ *~~jurisdiction~~* ~~shall not be valid. The issuance of a~~ *~~permit~~* ~~based on~~ *~~construction documents~~* ~~and other data shall not prevent the~~ *~~building official~~* ~~from requiring the correction of errors in the~~ *~~construction documents~~* ~~and other data. The~~ *~~building official~~* ~~is also authorized to prevent occupancy or use of a structure where in violation of this code or of any other ordinances of this~~ *~~jurisdiction.~~*

**~~R105.5 Expiration.~~** ~~Every~~ *~~permit~~* ~~issued shall become invalid unless the work authorized by such~~ *~~permit~~* ~~is commenced within 180 days after its issuance, or if the work authorized by such~~ *~~permit~~* ~~is suspended or abandoned for a period of 180 days after the time the work is commenced. The~~ *~~building official~~* ~~is authorized to grant, in writing, one or more extensions of time, for periods not more than 180 days each. The extension shall be requested in writing and justifiable cause demonstrated.~~

**~~R105.6 Suspension or revocation.~~** ~~The~~ *~~building official~~* ~~is authorized to suspend or revoke a~~ *~~permit~~* ~~issued under the provisions of this code wherever the~~ *~~permit~~* ~~is issued in error or on the basis of incorrect, inaccurate or incomplete information, or in violation of any ordinance or regulation or any of the provisions of this code.~~

**~~R105.7 Placement of permit.~~** ~~The building~~ *~~permit~~* ~~or copy thereof shall be kept on the site of the work until the completion of the project.~~

**~~R105.8 Responsibility.~~** ~~It shall be the duty of every person who performs work for the installation or repair of building, structure, electrical, gas, mechanical or plumbing systems, for which this code is applicable, to comply with this code.~~

**~~R105.9 Preliminary inspection.~~** ~~Before issuing a~~ *~~permit,~~* ~~the~~ *~~building official~~* ~~is authorized to examine or cause to be examined buildings, structures and sites for which an application has been filed.~~

**SECTION R106**

**CONSTRUCTION DOCUMENTS**

RESERVED

**~~R106.1 Submittal documents.~~** ~~Submittal documents consisting of~~ *~~construction documents,~~* ~~and other data shall be submitted in two or more sets with each application for a~~ *~~permit.~~* ~~The~~ *~~construction documents~~* ~~shall be prepared by a registered~~ *~~design professional~~* ~~where required by the statutes of the~~ *~~jurisdiction~~* ~~in which the project is to be constructed. Where special conditions exist, the~~ *~~building official~~* ~~is authorized to require additional~~ *~~construction documents~~* ~~to be prepared by a registered~~ *~~design professional.~~* **~~Exception:~~** ~~The~~ *~~building official~~* ~~is authorized to waive the submission of~~ *~~construction documents~~* ~~and other data not required to be prepared by a registered~~ *~~design professional~~* ~~if it is found that the nature of the work applied for is such that reviewing of~~ *~~construction documents~~* ~~is not necessary to obtain compliance with this code.~~

**~~R106.1.1 Information on construction documents.~~** *~~Construction documents~~* ~~shall be drawn upon suitable material. Electronic media documents are permitted to be submitted when~~ *~~approved~~* ~~by the~~ *~~building official.~~**~~Construction documents~~* ~~shall be of sufficient clarity to indicate the location, nature and extent of the work proposed and show in detail that it will conform to the provisions of this code and relevant laws, ordinances, rules and regulations, as determined by the~~ *~~building official.~~* ~~Where required by the~~ *~~building official,~~* ~~all braced wall lines, shall be identified on the~~ *~~construction documents~~* ~~and all pertinent information including, but not limited to, bracing methods, location and length of braced wall panels, foundation requirements of braced wall panels at top and bottom shall be provided.~~

**~~R106.1.2 Manufacturer’s installation instructions.~~** ~~Manufacturer’s installation instructions, as required by this code, shall be available on the job site at the time of inspection.~~

**~~R106.1.3 Information for construction in flood hazard areas.~~** ~~For buildings and structures located in whole or in part in flood hazard areas as established by Table R301.2(1),~~ *~~construction documents~~* ~~shall include:~~

~~1. Delineation of flood hazard areas, floodway boundaries and flood zones and the design flood elevation, as appropriate;~~

~~2. The elevation of the proposed lowest floor, including~~ *~~basement;~~* ~~in areas of shallow flooding (AO Zones), the height of the proposed lowest floor, including~~ *~~basement,~~* ~~above the highest adjacent~~ *~~grade;~~*

~~3. The elevation of the bottom of the lowest horizontal structural member in coastal high hazard areas (V Zone); and~~

~~4. If design flood elevations are not included on the community’s Flood Insurance Rate Map (FIRM), the~~ *~~building official~~* ~~and the applicant shall obtain and reasonably utilize any design flood elevation and floodway data available from other sources.~~

**~~R106.2 Site plan or plot plan.~~** ~~The~~ *~~construction documents~~* ~~submitted with the application for~~ *~~permit~~* ~~shall be accompanied by a site plan showing the size and location of new construction and existing structures on the site and distances from~~ *~~lot lines.~~* ~~In the case of demolition, the site plan shall show construction to be demolished and the location and size of existing structures and construction that are to remain on the site or plot. The~~ *~~building official~~* ~~is authorized to waive or modify the requirement for a site plan when the application for permit is for alteration or repair or when otherwise warranted.~~

**~~R106.3 Examination of documents.~~** ~~The~~ *~~building official~~* ~~shall examine or cause to be examined~~ *~~construction documents~~* ~~for code compliance.~~

**~~R106.3.1 Approval of construction documents.~~** ~~When the~~ *~~building official~~* ~~issues a~~ *~~permit,~~* ~~the~~ *~~construction documents~~* ~~shall be~~ *~~approved~~* ~~in writing or by a stamp which states "REVIEWED FOR CODE COMPLIANCE.” One set of~~ *~~construction documents~~* ~~so reviewed shall be retained by the~~ *~~building official.~~* ~~The other set shall be returned to the applicant, shall be kept at the site of work and shall be open to inspection by the~~ *~~building official~~* ~~or his or her authorized representative.~~

**~~R106.3.2 Previous approvals.~~** ~~This code shall not require changes in the~~ *~~construction documents,~~* ~~construction or designated occupancy of a structure for which a lawful~~ *~~permit~~* ~~has been heretofore issued or otherwise lawfully authorized, and the construction of which has been pursued in good faith within 180 days after the effective date of this code and has not been abandoned.~~

**~~R106.3.3 Phased approval.~~** ~~The~~ *~~building official~~* ~~is authorized to issue a~~ *~~permit~~* ~~for the construction of foundations or any other part of a building or structure before the~~ *~~construction documents~~* ~~for the whole building or structure have been submitted, provided that adequate information and detailed statements have been filed complying with pertinent requirements of this code. The holder of such~~ *~~permit~~* ~~for the foundation or other parts of a building or structure shall proceed at the holder’s own risk with the building operation and without assurance that a~~ *~~permit~~* ~~for the entire structure will be granted.~~

**~~R106.4 Amended construction documents.~~** ~~Work shall be installed in accordance with the~~ *~~approved construction documents,~~* ~~and any changes made during construction that are not in compliance with the~~ *~~approved construction documents~~* ~~shall be resubmitted for approval as an amended set of~~ *~~construction documents.~~*

**~~R106.5 Retention of construction documents.~~** ~~One set of~~ *~~approved construction documents~~* ~~shall be retained by the~~ *~~building official~~* ~~for a period of not less than 180 days from date of completion of the permitted work, or as required by state or local laws.~~

**SECTION R107**

**TEMPORARY STRUCTURES AND USES**

RESERVED

**~~R107.1 General.~~** ~~The~~ *~~building official~~* ~~is authorized to issue a~~ *~~permit~~* ~~for temporary structures and temporary uses. Such permits shall be limited as to time of service, but shall not be permitted for more than 180 days. The~~ *~~building official~~* ~~is authorized to grant extensions for demonstrated cause.~~

**~~R107.2 Conformance.~~** ~~Temporary structures and uses shall conform to the structural strength, fire safety, means of egress, light, ventilation and sanitary requirements of this code as necessary to ensure the public health, safety and general welfare.~~

**~~R107.3 Temporary power.~~** ~~The~~ *~~building official~~* ~~is authorized to give permission to temporarily supply and use power in part of an electric installation before such installation has been fully completed and the final certificate of completion has been issued. The part covered by the temporary certificate shall comply with the requirements specified for temporary lighting, heat or power in NFPA 70.~~

**~~R107.4 Termination of approval.~~** ~~The~~ *~~building official~~* ~~is authorized to terminate such~~ *~~permit~~* ~~for a temporary structure or use and to order the temporary structure or use to be discontinued.~~

**SECTION R108**

**FEES**

**RESERVED**

**~~R108.1 Payment of fees.~~** ~~A~~ *~~permit~~* ~~shall not be valid until the fees prescribed by law have been paid. Nor shall an amendment to a~~ *~~permit~~* ~~be released until the additional fee, if any, has been paid.~~

**~~R108.2 Schedule of permit fees.~~** ~~On buildings, structures, electrical, gas, mechanical and plumbing systems or~~ *~~alterations~~* ~~requiring a~~ *~~permit,~~* ~~a fee for each~~ *~~permit~~* ~~shall be paid as required, in accordance with the schedule as established by the applicable governing authority.~~

**~~R108.3 Building permit valuations.~~** ~~Building~~ *~~permit~~* ~~valuation shall include total value of the work for which a~~ *~~permit~~* ~~is being issued, such as electrical, gas, mechanical, plumbing equipment and other permanent systems, including materials and labor.~~

**~~R108.4 Related fees.~~** ~~The payment of the fee for the construction, alteration, removal or demolition for work done in connection with or concurrently with the work authorized by a building~~ *~~permit~~* ~~shall not relieve the applicant or holder of the~~ *~~permit~~* ~~from the payment of other fees that are prescribed by law.~~

**~~R108.5 Refunds.~~** ~~The~~ *~~building official~~* ~~is authorized to establish a refund policy.~~

**~~R108.6 Work commencing before permit issuance.~~** ~~Any person who commences work requiring a~~ *~~permit~~* ~~on a building, structure, electrical, gas, mechanical or plumbing system before obtaining the necessary permits shall be subject to a fee established by the applicable governing authority that shall be in addition to the required~~ *~~permit~~* ~~fees.~~

**SECTION R109**

**INSPECTIONS**

RESERVED

**~~R109.1 Types of inspections.~~** ~~For onsite construction, from time to time the~~ *~~building official~~*~~, upon notification from the~~ *~~permit~~* ~~holder or his agent, shall make or cause to be made any necessary inspections and shall either approve that portion of the construction as completed or shall notify the~~ *~~permit~~* ~~holder or his or her agent wherein the same fails to comply with this code.~~

**~~R109.1.1 Foundation inspection.~~** ~~Inspection of the foundation shall be made after poles or piers are set or trenches or~~ *~~basement~~* ~~areas are excavated and any required forms erected and any required reinforcing steel is in place and supported prior to the placing of concrete. The foundation inspection shall include excavations for thickened slabs intended for the support of bearing walls, partitions, structural supports, or~~ *~~equipment~~* ~~and special requirements for wood foundations.~~

**~~R109.1.2 Plumbing, mechanical, gas and electrical systems inspection.~~** ~~Rough inspection of plumbing, mechanical, gas and electrical systems shall be made prior to covering or concealment, before fixtures or~~ *~~appliances~~* ~~are set or installed, and prior to framing inspection.~~  **~~Exception:~~** ~~Backfilling of ground-source heat pump loop systems tested in accordance with~~ [~~Section M2105.1~~](javascript:Next('./icod_irc_2012_21_par032.htm');) ~~prior to inspection shall be permitted.~~

**~~R109.1.3 Floodplain inspections.~~** ~~For construction in flood hazard areas as established by Table R301.2(1), upon placement of the lowest floor, including~~ *~~basement,~~* ~~and prior to further vertical construction, the~~ *~~building official~~* ~~shall require submission of documentation, prepared and sealed by a registered~~ *~~design professional,~~* ~~of the elevation of the lowest floor, including~~ *~~basement,~~* ~~required in~~ [~~Section R322~~](javascript:Next('./icod_irc_2012_3_par255.htm');)~~.~~

**~~R109.1.4 Frame and masonry inspection.~~** ~~Inspection of framing and masonry construction shall be made after the roof, masonry, all framing, firestopping, draftstopping and bracing are in place and after the plumbing, mechanical and electrical rough inspections are~~ *~~approved.~~*

**~~R109.1.5 Other inspections.~~** ~~In addition to the called inspections above, the~~ *~~building official~~* ~~may make or require any other inspections to ascertain compliance with this code and other laws enforced by the~~ *~~building official.~~*

**~~R109.1.5.1 Fire-resistance-rated construction inspection.~~** ~~Where fire-resistance-rated construction is required between~~ *~~dwelling units~~* ~~or due to location on property, the~~ *~~building official~~* ~~shall require an inspection of such construction after all lathing and/or wallboard is in place, but before any plaster is applied, or before wallboard joints and fasteners are taped and finished.~~

**~~R109.1.6 Final inspection.~~** ~~Final inspection shall be made after the permitted work is complete and prior to occupancy.~~

**~~R109.1.6.1 Elevation documentation.~~** ~~If located in a flood hazard area, the documentation of elevations required in~~ [~~Section R322.1.10~~](javascript:Next('./icod_irc_2012_3_par267.htm');) ~~shall be submitted to the~~ *~~building official~~* ~~prior to the final inspection.~~

**~~R109.2 Inspection agencies.~~** ~~The~~ *~~building official~~* ~~is authorized to accept reports of~~ *~~approved~~* ~~agencies, provided such agencies satisfy the requirements as to qualifications and reliability.~~

**~~R109.3 Inspection requests.~~** ~~It shall be the duty of the~~ *~~permit~~* ~~holder or their agent to notify the~~ *~~building official~~* ~~that such work is ready for inspection. It shall be the duty of the person requesting any inspections required by this code to provide access to and means for inspection of such work.~~

**~~R109.4 Approval required.~~** ~~Work shall not be done beyond the point indicated in each successive inspection without first obtaining the approval of the~~ *~~building official.~~* ~~The~~ *~~building official~~* ~~upon notification, shall make the requested inspections and shall either indicate the portion of the construction that is satisfactory as completed, or shall notify the~~ *~~permit~~* ~~holder or an agent of the~~ *~~permit~~* ~~holder wherein the same fails to comply with this code. Any portions that do not comply shall be corrected and such portion shall not be covered or concealed until authorized by the~~ *~~building official.~~*

**SECTION R110**

**CERTIFICATE OF OCCUPANCY**

RESERVED

**~~R110.1 Use and occupancy.~~** ~~No building or structure shall be used or occupied, and no change in the existing occupancy classification of a building or structure or portion thereof shall be made until the~~ *~~building official~~* ~~has issued a certificate of occupancy therefor as provided herein. Issuance of a certificate of occupancy shall not be construed as an approval of a violation of the provisions of this code or of other ordinances of the~~ *~~jurisdiction.~~* ~~Certificates presuming to give authority to violate or cancel the provisions of this code or other ordinances of the~~ *~~jurisdiction~~* ~~shall not be valid.~~  **~~Exceptions:~~**

~~1. Certificates of occupancy are not required for work exempt from permits under Section R105.2.~~

~~2. Accessory buildings or structures.~~

**~~R110.2 Change in use.~~** ~~Changes in the character or use of an existing structure shall not be made except as specified in Sections 3408 and 3409 of the~~ *~~International Building Code~~*~~.~~

**~~R110.3 Certificate issued.~~** ~~After the~~ *~~building official~~* ~~inspects the building or structure and finds no violations of the provisions of this code or other laws that are enforced by the department of building safety, the~~ *~~building official~~* ~~shall issue a certificate of occupancy which shall contain the following:~~

~~1. The building~~ *~~permit~~* ~~number.~~

~~2. The address of the structure.~~

~~3. The name and address of the owner.~~

~~4. A description of that portion of the structure for which the certificate is issued.~~

~~5. A statement that the described portion of the structure has been inspected for compliance with the requirements of this code.~~

~~6. The name of the~~ *~~building official.~~*

~~7. The edition of the code under which the~~ *~~permit~~* ~~was issued.~~

~~8. If an automatic sprinkler system is provided and whether the sprinkler system is required.~~

~~9. Any special stipulations and conditions of the building~~ *~~permit.~~*

**~~R110.4 Temporary occupancy.~~** ~~The~~ *~~building official~~* ~~is authorized to issue a temporary certificate of occupancy before the completion of the entire work covered by the~~ *~~permit~~*~~, provided that such portion or portions shall be occupied safely. The~~ *~~building official~~* ~~shall set a time period during which the temporary certificate of occupancy is valid.~~

**~~R110.5 Revocation.~~** ~~The~~ *~~building official~~* ~~shall, in writing, suspend or revoke a certificate of occupancy issued under the provisions of this code wherever the certificate is issued in error, or on the basis of incorrect information supplied, or where it is determined that the building or structure or portion thereof is in violation of any ordinance or regulation or any of the provisions of this code.~~

**SECTION R111**

**SERVICE UTILITIES**

**RESERVED**

**~~R111.1 Connection of service utilities.~~** ~~No person shall make connections from a utility, source of energy, fuel or power to any building or system that is regulated by this code for which a~~ *~~permit~~* ~~is required, until~~ *~~approved~~* ~~by the~~ *~~building official.~~*

**~~R111.2 Temporary connection.~~** ~~The~~ *~~building official~~* ~~shall have the authority to authorize and approve the temporary connection of the building or system to the utility, source of energy, fuel or power.~~

**~~R111.3 Authority to disconnect service utilities.~~** ~~The~~ *~~building official~~* ~~shall have the authority to authorize disconnection of utility service to the building, structure or system regulated by this code and the referenced codes and standards set forth in Section R102.4 in case of emergency where necessary to eliminate an immediate hazard to life or property or when such utility connection has been made without the approval required by Section R111.1 or R111.2. The~~ *~~building official~~* ~~shall notify the serving utility and whenever possible the owner and occupant of the building, structure or service system of the decision to disconnect prior to taking such action if not notified prior to disconnection. The owner or occupant of the building, structure or service system shall be notified in writing as soon as practical thereafter.~~

**SECTION R112**

**BOARD OF APPEALS**

**RESERVED**

**~~R112.1 General.~~** ~~In order to hear and decide appeals of orders, decisions or determinations made by the~~ *~~building official~~* ~~relative to the application and interpretation of this code, there shall be and is hereby created a board of appeals. The~~ *~~building official~~* ~~shall be an ex officio member of said board but shall have no vote on any matter before the board. The board of appeals shall be appointed by the governing body and shall hold office at its pleasure. The board shall adopt rules of procedure for conducting its business, and shall render all decisions and findings in writing to the appellant with a duplicate copy to the~~ *~~building official.~~*

**~~R112.2 Limitations on authority.~~** ~~An application for appeal shall be based on a claim that the true intent of this code or the rules legally adopted thereunder have been incorrectly interpreted, the provisions of this code do not fully apply, or an equally good or better form of construction is proposed. The board shall have no authority to waive requirements of this code.~~

**~~R112.2.1 Determination of substantial improvement in flood hazard areas.~~** ~~When the~~ *~~building official~~* ~~provides a finding required in Section R105.3.1.1, the board of appeals shall determine whether the value of the proposed work constitutes a substantial improvement. A substantial improvement means any repair, reconstruction, rehabilitation,~~ *~~addition~~* ~~or improvement of a building or structure, the cost of which equals or exceeds 50 percent of the market value of the building or structure before the improvement or repair is started. If the building or structure has sustained substantial damage, all repairs are considered substantial improvement regardless of the actual repair work performed. The term does not include:~~

~~1. Improvements of a building or structure required to correct existing health, sanitary or safety code violations identified by the~~ *~~building official~~* ~~and which are the minimum necessary to assure safe living conditions; or~~

~~2. Any alteration of an historic building or structure, provided that the alteration will not preclude the continued designation as an historic building or structure. For the purpose of this exclusion, an historic building is:~~

~~2.1.~~ *~~Listed~~* ~~or preliminarily determined to be eligible for~~ *~~listing~~* ~~in the National Register of Historic Places; or~~

~~2.2. Determined by the Secretary of the U.S. Department of Interior as contributing to the historical significance of a registered historic district or a district preliminarily determined to qualify as an historic district; or~~

~~2.3. Designated as historic under a state or local historic preservation program that is~~ *~~approved~~* ~~by the Department of Interior.~~

**~~R112.2.2 Criteria for issuance of a variance for flood hazard areas.~~**  ~~A variance shall be issued only upon:~~

~~1. A showing of good and sufficient cause that the unique characteristics of the size, configuration or topography of the site render the elevation standards in~~ [~~Section R322~~](javascript:Next('./icod_irc_2012_3_par255.htm');) ~~inappropriate.~~

~~2. A determination that failure to grant the variance would result in exceptional hardship by rendering the~~ *~~lot~~* ~~undevelopable.~~

~~3. A determination that the granting of a variance will not result in increased flood heights, additional threats to public safety, extraordinary public expense, cause fraud on or victimization of the public, or conflict with existing local laws or ordinances.~~

~~4. A determination that the variance is the minimum necessary to afford relief, considering the flood hazard.~~

~~5. Submission to the applicant of written notice specifying the difference between the design flood elevation and the elevation to which the building is to be built, stating that the cost of flood insurance will be commensurate with the increased risk resulting from the reduced floor elevation, and stating that construction below the design flood elevation increases risks to life and property.~~

**~~R112.3 Qualifications.~~** ~~The board of appeals shall consist of members who are qualified by experience and training to pass on matters pertaining to building construction and are not employees of the~~ *~~jurisdiction.~~*

**~~R112.4 Administration.~~** ~~The~~ *~~building official~~* ~~shall take immediate action in accordance with the decision of the board.~~

**SECTION R113**

**VIOLATIONS**

**RESERVED**

**~~R113.1 Unlawful acts.~~** ~~It shall be unlawful for any person, firm or corporation to erect, construct, alter, extend, repair, move, remove, demolish or occupy any building, structure or~~ *~~equipment~~* ~~regulated by this code, or cause same to be done, in conflict with or in violation of any of the provisions of this code.~~

**~~R113.2 Notice of violation.~~** ~~The~~ *~~building official~~* ~~is authorized to serve a notice of violation or order on the person responsible for the erection, construction, alteration, extension, repair, moving, removal, demolition or occupancy of a building or structure in violation of the provisions of this code, or in violation of a detail statement or a plan~~ *~~approved~~* ~~thereunder, or in violation of a~~ *~~permit~~* ~~or certificate issued under the provisions of this code. Such order shall direct the discontinuance of the illegal action or condition and the abatement of the violation.~~

**~~R113.3 Prosecution of violation.~~** ~~If the notice of violation is not complied with in the time prescribed by such notice, the~~ *~~building official~~* ~~is authorized to request the legal counsel of the~~ *~~jurisdiction~~* ~~to institute the appropriate proceeding at law or in equity to restrain, correct or abate such violation, or to require the removal or termination of the unlawful occupancy of the building or structure in violation of the provisions of this code or of the order or direction made pursuant thereto.~~

**~~R113.4 Violation penalties.~~** ~~Any person who violates a provision of this code or fails to comply with any of the requirements thereof or who erects, constructs, alters or repairs a building or structure in violation of the~~ *~~approved construction documents~~* ~~or directive of the~~ *~~building official,~~* ~~or of a~~ *~~permit~~* ~~or certificate issued under the provisions of this code, shall be subject to penalties as prescribed by law.~~

**SECTION R114**

**STOP WORK ORDER**

**RESERVED**

**~~R114.1 Notice to owner.~~** ~~Upon notice from the~~ *~~building official~~* ~~that work on any building or structure is being prosecuted contrary to the provisions of this code or in an unsafe and dangerous manner, such work shall be immediately stopped. The stop work order shall be in writing and shall be given to the owner of the property involved, or to the owner’s agent or to the person doing the work and shall state the conditions under which work will be permitted to resume.~~

**~~R114.2 Unlawful continuance.~~** ~~Any person who shall continue any work in or about the structure after having been served with a stop work order, except such work as that person is directed to perform to remove a violation or unsafe condition, shall be subject to penalties as prescribed by law.~~

**[CA5422 AM]**

***CHAPTER 2, DEFINITIONS***

***Add or revise the following definitions as shown:***

**ADDITION**. An extension or increase in floor area, number of stories or height of a building or structure. **[F5424 AS]**

**APPLICABLE GOVERNING BODY.** A city, county, state, state agency or other political government subdivision or entity authorized to administer and enforce the provisions of this code, as adopted or amended. Also applies to administrative authority. **[CA5423 AS]**

**ARCHITECT.** A Florida-registered architect. **[CA5423 AS]**

**CARBON MONOXIDE ALARM.** A device for the purpose of detecting carbon monoxide, that produces a distinct audible alarm, and is listed or labeled with the appropriate standard, either ANSI/UL 2034 ~~- 96,~~ *Standard for Single and Multiple Station CO Alarms*, or UL 2075 ~~- 04~~, *Gas and Vapor Detector Sensor*, in accordance with its application. **[5424 AS]**

**COMMISSION**. Means the Florida Building Commission created by this part.

**DECORATIVE CEMENTITIOUS FINISH**. A skim coat, as defined in ASTM C 926, of Portland cement-based plaster applied to concrete or masonry surfaces intended for cosmetic purposes. **[SP5416 AS]**

**ENFORCEMENT AGENCY.**

**Local enforcement agency.** Means an agency of local government with authority to make inspections of buildings and to enforce the codes which establish standards for design, construction, erection, alteration, repair, modification or demolition of public or private buildings, structures or facilities.

**State Enforcement Agency.** Means the agency of state government with authority to make inspections of buildings and to enforce the codes, as required by this part, which establish standards for design, construction, erection, alteration, repair, modification or demolition of public or private buildings, structures or facilities. **[CA5423 AS]**

**ENGINEER**. A Florida-registered engineer. **[CA5423 AS]**

**FIRE SEPARATION DISTANCE.** The distance measured from the building face to one of the following:

1. To the closest interior lot line; or

2. To the centerline of a street, an alley, easement or public way; or

3. To an imaginary line between two buildings on the lot.

The distance shall be measured at a right angle from the face of the wall.

**FOSSIL FUEL.** Coal, kerosene, oil, fuel gases, or other petroleum or hydrocarbon product that emits carbon monoxide as a by-product of combustion. **[5424 AS]**

**GARAGE DOOR MANUFACTURER:**The party responsible for the completed assembly of the garage door components. **[S5329 AS]**

**HABITABLE SPACE.**A space in a building for living, sleeping, eating or cooking. Bathrooms, toilet rooms, closets, halls, screen enclosures, sunroom Categories I, II and III as defined in the AAMA/NPEA/NSA 2100, storage or utility spaces and similar areas, are not considered habitable spaces**.**

**(S6233GlitchAS)**

**HIGH VELOCITY HURRICANE ZONE (HVHZ)**. This zone consists of Broward and Dade counties.

**LANDSCAPE ARCHITECT.** A Florida registered Landscape Architect. **[CA5423 AS]**

**LOCAL FLOODPLAIN MANAGEMENT ORDINANCE.**An ordinance or regulation adopted pursuant to the authority granted to local governments by Title 44 Code of Federal Regulations, Sections 59 and 60 for participation in the National Flood Insurance Program. **[SP 5290 AS]**

**MATERIAL CODE VIOLATION.** A material code violation is a violation that exists within a completed building, structure or facility which may reasonably result, or has resulted, in physical harm to a person or significant damage to the performance of a building or its systems. **[CA5423 AS]**

**MATERIAL VIOLATION.** As defined in Florida Statutes. **[CA5423 AS]**

**MEANS OF ESCAPE**. A way out of a building or structure that does not conform to the strict definition of means of egress but does provide an alternate way out. A means of escape consists of a door, stairway, passage or hall providing a way of unobstructed travel to the outside at street or ground level. It may also consist of a passage through an adjacent nonlockable space, independent of and remotely located from the means of egress, to any approved exit. **[F5716 AM]**

**MODULAR HOME.** Any residential unit, constructed to standards promulgated by the Florida Building Commission, away from the installation site, and which bears a Department of Business and Professional Regulation Insignia. **[SP 5428 AS]**

**REGISTERED TERMITICIDE**. Product listed as registered for use as a preventative treatment for termites for new construction by the Florida Department of Agriculture and Consumer Services under authority of Chapter 487, *Florida Statutes*.

|  |
| --- |
| **ROOF SECTION.** A separating or division of a roof area by existing expansion joints, parapet walls, flashing (excluding valley), difference of elevation (excluding hips and ridges), roof type or legal description; not including the roof area required for a proper tie-off with an existing system. **(5937 AM)** |
|  |

**SEPARATE ATMOSPHERE.** The atmosphere that exists between rooms, spaces or areas that are separated by an approved smoke barrier. (F5424 AS)

**SITE BUILT SINGLE-FAMILY RESIDENTIAL STRUCTURES.** This term shall mean site built single family detached residential structures. **(5937 AS G3)**

**SCREEN ENCLOSURE.** A building or part thereof, in whole or in part self-supporting, and having walls of insect screening with or without removable vinyl or acrylic wind break panels and a roof of insect screening, plastic, aluminum or similar lightweight material, or other materials and assemblies such as a patio, deck, or roof of a structure. (S6243GlitchAM)

**SUNROOM.**

1. A one-story structure attached to a *dwelling* with a *glazing area* in excess of 40 percent of the gross area of the structure’s *exterior walls* and roof. ~~For definition applicable in Chapter 11, see Section N1101.9.~~

2. A one-story structure added to a dwelling with solid roof panels without sloped glazing. The sunroom walls may have any configuration, provided the open areas with operable or fixed glass or windows or side hinged or sliding glass doors of the longer wall and one additional wall is equal to at least 65 percent of the area below 6 foot 8 inches (2032 mm) of each wall, measured from the floor. For the purposes of this code the term sunroom as used herein shall include conservatories, sunspaces, solariums, and porch or patio covers or enclosures. **[CA5793 AS]**

**TOWNHOUSE**. A single-family dwelling unit constructed in a group of three or more attached units with property lines separating each unit in which each unit extends from foundation to roof and with a yard or public way on at least two sides. (F5424 AS)

**VALUE**. The estimated current replacement cost of the building in kind. **[CA5938 AS] [CA5423 AS]**

**WIND-BORNE DEBRIS REGION.** Areas within *hurricane-prone regions* ~~as designated in accordance with Figure R302.1(4).~~ located:

1.       Within 1 mile (1.61 km) of the coastal mean high water line where the ultimate design wind speed Vult is 130 mph (58 m/s) or greater; or

1. In areas where the ultimate design wind speed Vult is 140 mph (63 m/s) or greater.

**[S5678 AS]**

**WIND SPEED, Vult.** Ultimate design wind speeds (3-sec gust), miles per hour (mph) (km/hr) determined from Figure R301.2(4). **[S5678 AS]**

**WIND SPEED, Vasd.** Nominal design wind speeds (3-sec gust), miles per hour (mph) (km/hr) where applicable. **[S5678 AS]**

***CHAPTER 3, BUILDING PLANNING***

***Section R301.1 Application. Change to read as follows:***

**R301.1 Application.** Buildings and structures, and all parts thereof, shall be constructed to safely support all loads, including dead loads, live loads, roof loads, flood loads, and wind loads as prescribed by this code. The construction of buildings and structures in accordance with the provisions of this code shall result in a system that provides a complete load path that meets all requirements for the transfer of all loads from their point of origin through the load-resisting elements to the foundation. Buildings and structures constructed as prescribed by this code are deemed to comply with the requirements of this section.

**Exception**: Buildings and structures located within the High Velocity Hurricane Zone shall comply with Sections R302 to R324, inclusive and the provisions of Chapter R44 and section R406. In addition, buildings and structures located in flood hazard areas established in Table R301.2(1) shall comply with Sections R301.2.4 and R322.  **[SP 5290 AS]**

***Table R301.2(1). Change to read as shown:***

**TABLE R301.2(1)** **CLIMATIC AND GEOGRAPHIC DESIGN CRITERIA**

|  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **GROUND SNOW LOAD** | **WIND** **DESIGN** | | **SEISMIC DESIGN CATEGORYf** | **SUBJECT TO DAMAGE FROM** | | | **WINTER DESIGN TEMPe** | **ICE BARRIER UNDERLAYMENT REQUIREDh** | **FLOOD HAZARDSg** | **AIR FREEZING INDEXi** | **MEAN ANNUAL TEMPj** |
| **Speedd**  **(mph)** | **Topographic effectsk** **(NA)** | **Weatheringa** | **Frost line depthb** | **Termitec** |
| NA | See Fig. R301.2(4) | | NA | Negligible | NA | Very Heavy |  | NA |  | NA | NA |

For SI: 1 pound per square foot = 0.0479 kN/m2, 1 mile per hour =1.609 km/h.

a. Weathering is “negligible” for concrete as determined from the Weathering Probability Map [Figure 301.2(3)]. The grade of masonry units shall be determined from ASTM C 34, C 55, C 62, C 73, C 90, C 129, C 145, C 216 or C 652.

b. ~~The frost line depth may require deeper footings than indicated in Figure R403.1(1). The jurisdiction shall fill in the frost line depth column with the minimum depth of footing below finish grade.~~ Reserved.

c. Termite infestation per Figure R301.2(6) is “very heavy.”

d. Wind speed shall be from the basic wind speed map [Figure R301.2(4). Wind exposure category shall be determined on a site-specific basis in accordance with Section R301.2.1.4.

e. The outdoor design dry-bulb temperature shall be selected from the columns of 971/2-

percent values for winter from Appendix D of the *Florida Building Code, Plumbing*. Deviations from the Appendix D temperatures shall be permitted to reflect local climates or local weather experience as determined by the building official.

f. ~~The jurisdiction shall fill in this part of the table with the seismic design category determined from Section R301.2.2.1.~~ Reserved.

g. The applicable governing **body** shall, by local floodplain management ordinance, specify ~~jurisdiction shall fill in this part of the table with~~ (a) the date of the jurisdiction’s entry into the National Flood Insurance Program (date of adoption of the first code or ordinance for management of flood hazard areas), (b) the date(s) of the Flood Insurance Study and (c) the panel numbers and dates of all currently effective FIRM and FBFM, or other flood hazard map adopted by the authority having jurisdiction, as amended.  **[SP5290 AS]**

h. ~~In accordance with Sections R905.2.7.1, R905.4.3.1, R905.5.3.1, R905.6.3.1, R905.7.3.1 and R905.8.3.1, where there has been a history of local damage from the effects of ice damming, the jurisdiction shall fill in this part of the table with “YES.” Otherwise, the jurisdiction shall fill in this part of the table with “NO.”~~ Reserved.

i. ~~The jurisdiction shall fill in this part of the table with the 100-year return period air freezing index (BF-days) from Figure R403.3(2) or from the 100-year (99 percent) value on the National Climatic Data Center data table “Air Freezing Index-USA Method (Base 32°F)” at~~ [~~www.ncdc.noaa.gov/fpsf.html.~~](http://www.ncdc.noaa.gov/fpsf.html) Reserved.

j. ~~The jurisdiction shall fill in this part of the table with the mean annual temperature from the National Climatic Data Center data table “Air Freezing Index- USA Method (Base 32°F)” at~~ [~~www.ncdc.noaa.gov/fpsf.html.~~](http://www.ncdc.noaa.gov/fpsf.html) Reserved.

k. ~~In accordance with Section R301.2.1.5, where there is local historical data documenting structural damage to buildings due to topographic wind speed-up effects, the jurisdiction shall fill in this part of the table with “YES.” Otherwise, the jurisdiction shall indicate “NO” in this part of the table.~~ Reserved.

**(S6081GlitchAS)**

***Table R301.2(2). Delete and replace with the following:***

**TABLE R301.2(2)**

**COMPONENT AND CLADDING LOADS FOR A BUILDING WITH A MEAN ROOF HEIGHT OF 30 FEET LOCATED IN EXPOSURE B (psf) a,b,c,d,e,f,g**

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
|  | **ZONE** | **EFFECTIVE WIND AREA (feet2)** | **Ultimate Design Wind Speed** **V*ult*** **(mph)** | | | | | | | | | | | | | | | | | |
| **110** | | **115** | | **120** | | **130** | | **140** | | **150** | | **160** | | **180** | | **200** | |
| **Roof > 0 to 7 degrees** | 1 | 10 | 8.9 | -21.8 | 9.7 | -23.8 | 10.5 | -25.9 | 12.4 | -30.4 | 14.3 | -35.3 | 16.5 | -40.5 | 18.7 | -46.1 | 23.7 | -58.3 | 29.3 | -72.0 |
| 1 | 20 | 8.3 | -21.2 | 9.1 | -23.2 | 9.9 | -25.2 | 11.6 | -29.6 | 13.4 | -34.4 | 15.4 | -39.4 | 17.6 | -44.9 | 22.2 | -56.8 | 27.4 | -70.1 |
| 1 | 50 | 7.6 | -20.5 | 8.3 | -22.4 | 9.0 | -24.4 | 10.6 | -28.6 | 12.3 | -33.2 | 14.1 | -38.1 | 16.0 | -43.3 | 20.3 | -54.8 | 25.0 | -67.7 |
| 1 | 100 | 7.0 | -19.9 | 7.7 | -21.8 | 8.3 | -23.7 | 9.8 | -27.8 | 11.4 | -32.3 | 13.0 | -37.0 | 14.8 | -42.1 | 18.8 | -53.3 | 23.2 | -65.9 |
| 2 | 10 | 8.9 | -36.5 | 9.7 | -39.9 | 10.5 | -43.5 | 12.4 | -51.0 | 14.3 | -59.2 | 16.5 | -67.9 | 18.7 | -77.3 | 23.7 | -97.8 | 29.3 | -120.7 |
| 2 | 20 | 8.3 | -32.6 | 9.1 | -35.7 | 9.9 | -38.8 | 11.6 | -45.6 | 13.4 | -52.9 | 15.4 | -60.7 | 17.6 | -69.0 | 22.2 | -87.4 | 27.4 | -107.9 |
| 2 | 50 | 7.6 | -27.5 | 8.3 | -30.1 | 9.0 | -32.7 | 10.6 | -38.4 | 12.3 | -44.5 | 14.1 | -51.1 | 16.0 | -58.2 | 20.3 | -73.6 | 25.0 | -90.9 |
| 2 | 100 | 7.0 | -23.6 | 7.7 | -25.8 | 8.3 | -28.1 | 9.8 | -33.0 | 11.4 | -38.2 | 13.0 | -43.9 | 14.8 | -50.0 | 18.8 | -63.2 | 23.2 | -78.1 |
| 3 | 10 | 8.9 | -55.0 | 9.7 | -21.8 | 10.5 | -65.4 | 12.4 | -76.8 | 14.3 | -89.0 | 16.5 | -102.2 | 18.7 | -116.3 | 23.7 | -147.2 | 29.3 | -181.7 |
| 3 | 20 | 8.3 | -45.5 | 9.1 | -21.2 | 9.9 | -54.2 | 11.6 | -63.6 | 13.4 | -73.8 | 15.4 | -84.7 | 17.6 | -96.3 | 222 | -121.9 | 27.4 | -150.5 |
| 3 | 50 | 7.6 | -33.1 | 8.3 | -20.4 | 9.0 | -39.3 | 10.6 | -46.2 | 12.3 | -53.5 | 14.1 | -61.5 | 16.0 | -69.9 | 20.3 | -88.5 | 25.0 | -109.3 |
| 3 | 100 | 7.0 | -23.6 | 7.7 | -19.8 | 8.3 | -28.1 | 9.8 | -33.0 | 11.4 | -38.2 | 13.0 | -43.9 | 14.8 | -50.0 | 18.8 | -63.2 | 23.2 | -78.1 |
| **Roof > 7** **to 27** **degrees** | 1 | 10 | 12.5 | -19.9 | 13.7 | -37.9 | 14.9 | -23.7 | 17.5 | -27.8 | 20.3 | -32.3 | 23.3 | -37.0 | 28.5 | -42.1 | 33.6 | -53.3 | 41.5 | -65.9 |
| 1 | 20 | 11.4 | -19.4 | 12.5 | -34.9 | 13.6 | -23.0 | 16.0 | -27.0 | 18.5 | -31.4 | 21.3 | -36.0 | 24.2 | -41.0 | 30.6 | -51.9 | 37.8 | -64.0 |
| 1 | 50 | 10.0 | -18.6 | 10.9 | -30.9 | 11.9 | -22.2 | 13.9 | -26.0 | 16.1 | -30.2 | 18.5 | -34.6 | 21.1 | -39.4 | 26.7 | -49.9 | 32.9 | -61.6 |
| 1 | 100 | 8.9 | -18.1 | 9.7 | -27.8 | 10.5 | -21.5 | 12.4 | -25.2 | 14.3 | -29.3 | 16.5 | -33.6 | 18.7 | -38.2 | 23.7 | -48.4 | 29.3 | -59.8 |
| 2 | 10 | 12.5 | -34.7 | 13.7 | -56.0 | 14.9 | -41.3 | 17.5 | -48.4 | 20.3 | -56.2 | 23.3 | -64.5 | 26.5 | -73.4 | 33.6 | -92.9 | 41.5 | -114.6 |
| 2 | 20 | 11.4 | -31.9 | 12.5 | -52.4 | 13.6 | -38.0 | 16.0 | -44.6 | 18.5 | -51.7 | 21.3 | -59.3 | 24.2 | -67.5 | 30.6 | -85.4 | 37.8 | -105.5 |
| 2 | 50 | 10.0 | -28.2 | 10.9 | -47.6 | 11.9 | -33.6 | 13.9 | -39.4 | 16.1 | -45.7 | 18.5 | -52.5 | 21.1 | -59.7 | 26.7 | -75.6 | 32.9 | -93.3 |
| 2 | 100 | 8.9 | -25.5 | 9.7 | -44.0 | 10.5 | -30.3 | 12.4 | -35.6 | 14.3 | -41.2 | 16.5 | -47.3 | 18.7 | -53.9 | 23.7 | -68.2 | 29.3 | -84.2 |
| 3 | 10 | 12.5 | -51.3 | 13.7 | -23.8 | 14.9 | -61.0 | 17.5 | -71.6 | 20.3 | -83.1 | 23.3 | -95.4 | 26.5 | -108.5 | 33.6 | -137.3 | 41.5 | -169.5 |
| 3 | 20 | 11.4 | -47.9 | 12.5 | -22.6 | 13.6 | -57.1 | 16.0 | -67.0 | 18.5 | -77.7 | 21.3 | -89.2 | 24.2 | -101.4 | 30.6 | -128.4 | 37.8 | -158.5 |
| 3 | 50 | 10.0 | -43.5 | 10.9 | -21.0 | 11.9 | -51.8 | 13.9 | -60.8 | 16.1 | -70.5 | 18.5 | -81.0 | 21.1 | -92.1 | 26.7 | -116.6 | 32.9 | -143.9 |
| 3 | 100 | 8.9 | -40.2 | 9.7 | -19.8 | 10.5 | -47.9 | 12.4 | -56.2 | 14.3 | -65.1 | 16.5 | -74.8 | 18.7 | -85.1 | 23.7 | -107.7 | 29.3 | -132.9 |
| **Roof > 27 to 45 degrees** | 1 | 10 | 19.9 | -21.8 | 21.8 | -27.6 | 23.7 | -25.9 | 27.8 | -30.4 | 32.3 | -35.3 | 37.0 | -40.5 | 42.1 | -46.1 | 53.3 | -58.3 | 65.9 | -72.0 |
| 1 | 20 | 19.4 | -20.7 | 21.2 | -26.6 | 23.0 | -24.6 | 27.0 | -28.9 | 31.4 | -33.5 | 36.0 | -38.4 | 41.0 | -43.7 | 51.9 | -55.3 | 64.0 | -68.3 |
| 1 | 50 | 18.6 | -19.2 | 20.4 | -25.0 | 22.2 | -22.8 | 26.0 | -26.8 | 30.2 | -31.1 | 34.6 | -35.7 | 39.4 | -40.6 | 49.9 | -51.4 | 61.6 | -63.4 |
| 1 | 100 | 18.1 | -18.1 | 19.8 | -23.8 | 21.5 | -21.5 | 25.2 | -25.2 | 29.3 | -29.3 | 33.6 | -33.6 | 38.2 | -38.2 | 48.4 | -48.4 | 59.8 | -59.8 |
| 2 | 10 | 19.9 | -25.5 | 21.8 | -27.8 | 23.7 | -30.3 | 27.8 | -35.6 | 32.3 | -41.2 | 37.0 | -47.3 | 42.1 | -53.9 | 53.3 | -68.2 | 65.9 | -84.2 |
| 2 | 20 | 19.4 | -24.3 | 21.2 | -26.6 | 23.0 | -29.0 | 27.0 | -34.0 | 31.4 | -39.4 | 36.0 | -45.3 | 41.0 | -51.5 | 51.9 | -65.2 | 64.0 | -80.5 |
| 2 | 50 | 18.6 | -22.9 | 20.4 | -25.0 | 22.2 | -27.2 | 26.0 | -32.0 | 30.2 | -37.1 | 34.6 | -42.5 | 39.4 | -48.4 | 49.9 | -61.3 | 61.6 | -75.6 |
| 2 | 100 | 18.1 | -21.8 | 19.8 | -23.8 | 21.5 | -25.9 | 25.2 | -30.4 | 29.3 | -35.3 | 33.6 | -40.5 | 38.2 | -46.1 | 48.4 | -58.3 | 59.8 | -72.0 |
| 3 | 10 | 19.9 | -25.5 | 21.8 | -27.8 | 23.7 | -30.3 | 27.8 | -35.6 | 32.3 | -41.2 | 37.0 | -47.3 | 42.1 | -53.9 | 53.3 | -68.2 | 65.9 | -84.2 |
| 3 | 20 | 19.4 | -24.3 | 21.2 | -26.6 | 23.0 | -29.0 | 27.0 | -34.0 | 31.4 | -39.4 | 36.0 | -45.3 | 41.0 | -51.5 | 51.9 | -65.2 | 64.0 | -80.5 |
| 3 | 50 | 18.6 | -22.9 | 20.4 | -25.0 | 22.2 | -27.2 | 26.0 | -32.0 | 30.2 | -37.1 | 34.6 | -42.5 | 39.4 | -48.4 | 49.9 | -61.3 | 61.6 | -75.6 |
| 3 | 100 | 18.1 | -21.8 | 19.8 | -23.6 | 21.5 | -25.9 | 25.2 | -30.4 | 29.3 | -35.3 | 33.6 | -40.5 | 38.2 | -46.1 | 48.4 | -58.3 | 59.8 | -72.0 |
| **Wall** | 4 | 10 | 21.8 | -23.6 | 23.8 | -25.8 | 25.9 | -28.1 | 30.4 | -33.0 | 35.3 | -38.2 | 40.5 | -43.9 | 46.1 | -50.0 | 58.3 | -63.2 | 72.0 | -78.1 |
| 4 | 20 | 20.8 | -22.6 | 22.7 | -24.7 | 24.7 | -26.9 | 29.0 | -31.6 | 33.7 | -36.7 | 38.7 | -42.1 | 44.0 | -47.9 | 55.7 | -60.6 | 68.7 | -74.8 |
| 4 | 50 | 19.5 | -21.3 | 21.3 | -23.3 | 23.2 | -25.4 | 27.2 | -29.8 | 31.6 | -34.6 | 36.2 | -39.7 | 41.2 | -45.1 | 52.2 | -57.1 | 64.4 | -70.6 |
| 4 | 100 | 18.5 | -20.4 | 20.2 | -22.2 | 22.0 | -24.2 | 25.9 | -28.4 | 30.0 | -33.0 | 34.4 | -37.8 | 39.2 | -43.1 | 49.6 | -54.5 | 61.2 | -67.3 |
| 4 | 500 | 16.2 | -18.1 | 17.7 | -19.8 | 19.3 | -21.5 | 22.7 | -25.2 | 26.3 | -29.3 | 30.2 | -33.6 | 34.3 | -38.2 | 43.5 | -48.4 | 53.7 | -59.8 |
| 5 | 10 | 21.8 | -29.1 | 23.8 | -31.9 | 25.9 | -34.7 | 30.4 | -40.7 | 35.3 | -47.2 | 40.5 | -54.2 | 46.1 | -61.7 | 58.3 | -78.0 | 72.0 | -96.3 |
| 5 | 20 | 20.8 | -27.2 | 22.7 | -29.7 | 24.7 | -32.4 | 29.0 | -38.0 | 33.7 | -44.0 | 38.7 | -50.5 | 44.0 | -57.5 | 55.7 | -72.8 | 68.7 | -89.9 |
| 5 | 50 | 19.5 | -24.6 | 21.3 | -26.9 | 23.2 | -29.3 | 27.2 | -34.3 | 31.6 | -39.8 | 36.2 | -45.7 | 41.2 | -52.0 | 52.2 | -65.8 | 64.4 | -81.3 |
| 5 | 100 | 18.5 | -22.6 | 20.2 | -24.7 | 22.0 | -26.9 | 25.9 | -31.6 | 30.0 | -36.7 | 34.4 | -42.1 | 39.2 | -47.9 | 49.6 | -60.6 | 61.2 | -74.8 |
| 5 | 500 | 16.2 | -18.1 | 17.7 | -19.8 | 19.3 | -21.5 | 22.7 | -25.2 | 26.3 | -29.3 | 30.2 | -33.6 | 34.3 | -38.2 | 43.5 | -48.4 | 53.7 | -59.8 |

 a. The effective wind area shall be equal to the span length multiplied by an effective width. This width shall be permitted to be not be less than one-third the span length. For cladding fasteners, the effective wind area shall not be greater than the area that is tributary to an individual fastener.

b. For effective areas between those given above, the load may be interpolated; otherwise, use the load associated with the lower effective area.

c. Table values shall be adjusted for height and exposure by multiplying by the adjustment coefficient in Table R301.2(3).

d. See Figure R301.2(7) for location of zones.

e. Plus and minus signs signify pressures acting toward and away from the building surfaces.

f. Positive design wind pressures shall not be less than +16 psf and negative design wind pressures shall not be less than -16 psf.

g.  For allowable stress design and for testing as specified in Section R301.2.1.6, component and cladding loads are permitted to be multiplied by 0.6.

**[S5702 AS](S6274GlitchAS)**

***Section R301.2.1 Wind design criteria. Change to read as shown:***

**R301.2.1 Wind design criteria.** Buildings and portions thereof shall be constructed in accordance with the wind provisions of this code using the basic wind speed in Table R301.2(1) as determined from Figure R301.2(4)~~A~~. ~~The structural provisions of this code for wind loads are not permitted where wind design is required as specified in Section R301.2.1.1.~~ Where different construction methods and structural materials are used for various portions of a building, the applicable requirements of this section for each portion shall apply. Where not otherwise specified, the wind loads listed in Table R301.2(2) adjusted for height and exposure using Table R301.2(3) shall be used to determine design load performance requirements for wall coverings, curtain walls, roof coverings, exterior windows, skylights~~, garage doors~~ and exterior doors (other than garage doors). Where loads for garage doors are not otherwise specified, the loads listed in Table R301.2(4) adjusted for height and exposure using Table R301.2(3) shall be used to determine design load performance requirements. Asphalt shingles shall be designed for wind speeds in accordance with Section R905.2.4. A continuous load path shall be provided to transmit the applicable uplift forces ~~in Section R802.11.1~~ from the roof assembly to the foundation.

**(S5328AM) (S5681 AS)**

***Add the following table:***

***[NOTE: numbers are crammed in. They should be read from the top line down, e.g.: -27.5]***

**TABLE R301.2(4)**

**NOMINAL (ASD) GARAGE DOOR LOADS FOR A BUILDING WITH A**

**MEAN ROOF HEIGHT OF 30 FEET LOCATED IN EXPOSURE B (PSF) 1,2,3,4,5**

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| Roof Angle  >10 degrees | | **Ultimate Design Wind Speed (Vult) Determined in Accordance with Section R301.2.1 (mph - 3 Second Gust)** | | | | | | | | | | | | | | | | | | | | | |
| W Width  (ft) | Height  (ft) | 100 mph | | 110 mph | | 120 mph | | 130 mph | | 140 mph | | 150 mph | | 160 mph | | 170 mph | | 180 180 mph | | 190 mph | | 200 200 mph | |
| 9 | 7 | 9.6 | -10.9 | 11.4 | -12.9 | 13.7 | -15.5 | 16.1 | -18.2 | 18.5 | -20.9 | 21.3 | -24.1 | 24.3 | -27.5 | 27.6 | -31.2 | 30.6 | -34.6 | 34.2 | -38.6 | 38.0 | -43.0 |
| 16 | 7 | 9.2 | -10.3 | 10.9 | -12.2 | 13.1 | -14.6 | 15.5 | -17.2 | 17.7 | -19.7 | 20.4 | -22.7 | 23.3 | -26.0 | 26.4 | -29.4 | 29.3 | -32.6 | 32.7 | -36.5 | 36.4 | -40.6 |
|  | | 78 mph | | 85 mph | | 93 mph | | 101 mph | | 108 mph | | 116 mph | | 124 mph | | 132 mph | | 139 mph | | 147 mph | | 155 mph | |
|  | | **Nominal Design Wind Speed (Vasd) converted from Vult per Section R301.2.1.3** | | | | | | | | | | | | | | | | | | | | | |

For SI: 1 foot = 304.8 mm, 1 mile per hour = 1.609 km/h, 1 psf = 47.88 N/m2

1. For effective areas or wind speeds between those given above the load may be interpolated, otherwise use the load associated with the lower effective area.

2. Table values shall be adjusted for height and exposure by multiplying by the adjustment coefficient in Table R301.2(3)

3. Plus and minus signs signify pressures acting toward and away from the building surfaces.

4. Negative pressures assume door has 2 feet of width in building's end zone.

5.Table values include the 0.6 load reduction factor.

**(S5328 AM R1)**

***Section R301.2.1.1 Wind limitations and wind design required. Change to read as shown:***

**R301.2.1.1 Wind limitations and wind design required.** The wind provisions of this code shall not apply to the design of buildings where ~~wind design is required in accordance with Figure R301.2(4)B or where~~ the ~~basic~~ ultimate design wind speed, V*ult*, from Figure R301.2(4)~~A~~ equals or exceeds 115 ~~110~~ miles per hour (51 ~~49~~ m/s).

**Exceptions:**

1. For concrete construction, the wind provisions of this code shall apply in accordance with the limitations of Sections R404 and R611.

2. For structural insulated panels, the wind provisions of this code shall apply in accordance with the limitations of Section R613.

In regions where ~~wind design is required in accordance with Figure R301.2(4)B or where~~ the basic ultimate design wind speed, V*ult*, shown on Figure R301.2(4)~~A~~ equals or exceeds 115 ~~110~~ miles per hour (51 ~~49~~ m/s), the design of buildings for wind loads shall be in accordance with one or more of the following methods:

1. AF&PA *Wood Frame Construction Manual* (WFCM); or

2. ICC *Standard for Residential Construction in High-Wind Regions* (ICC 600); or

3. ASCE *Minimum Design Loads for Buildings and Other Structures* (ASCE 7); or

4. AISI *Standard for Cold-Formed Steel Framing—Prescriptive Method For One- and Two-Family Dwellings* (AISI S230); or

5. *~~International~~ Florida Building Code.*

6. Concrete masonry construction shall be designed in accordance with the provisions of this code or in accordance with TMS 402/ACI 530/ASCE 5 and TMS 602/ACI 530.1/ASCE 6; or

7. The MAF Guide to Concrete Masonry Residential Construction in High Wind Areas shall be permitted for applicable concrete masonry buildings for a basic wind speed of 130 mph (58 m/s) or less in Exposure B and 110 mph (49 m/s) or less in Exposure C in accordance with Figure R301.2(4) as converted in accordance with R301.2.1.3.

The wind speeds in Figure R301.2(4) shall be converted to nominal wind speeds, Vasd, in accordance with Section R301.2.1.3 when the provisions of the standards referenced in Items 2, 4 and 7  ~~2 through 4~~ are used unless the wind provisions in the standards are based on Ultimate Wind Speeds as specified in Figure R301.2(4) or Chapter 26 of ASCE 7.

(S6115GlitchAS)

The elements of design not addressed by the methods in Items 1 through ~~5~~ 7 shall be in accordance with the provisions of this code. ~~When ASCE 7 or the~~ *~~International Florida Building Code~~* ~~is used for the design of the building, the wind speed map and exposure category requirements as specified in ASCE 7 and the~~ *~~International Florida Building Code~~* ~~shall be used.~~

**[S5681 AS] [S5998 AS]**

***Delete Figures R301.2(4)A, R301.2(4)B, and R301.2(4)C as shown:***

**~~FIGURE R301.2(4)A~~**

**~~BASIC WIND SPEEDS~~**

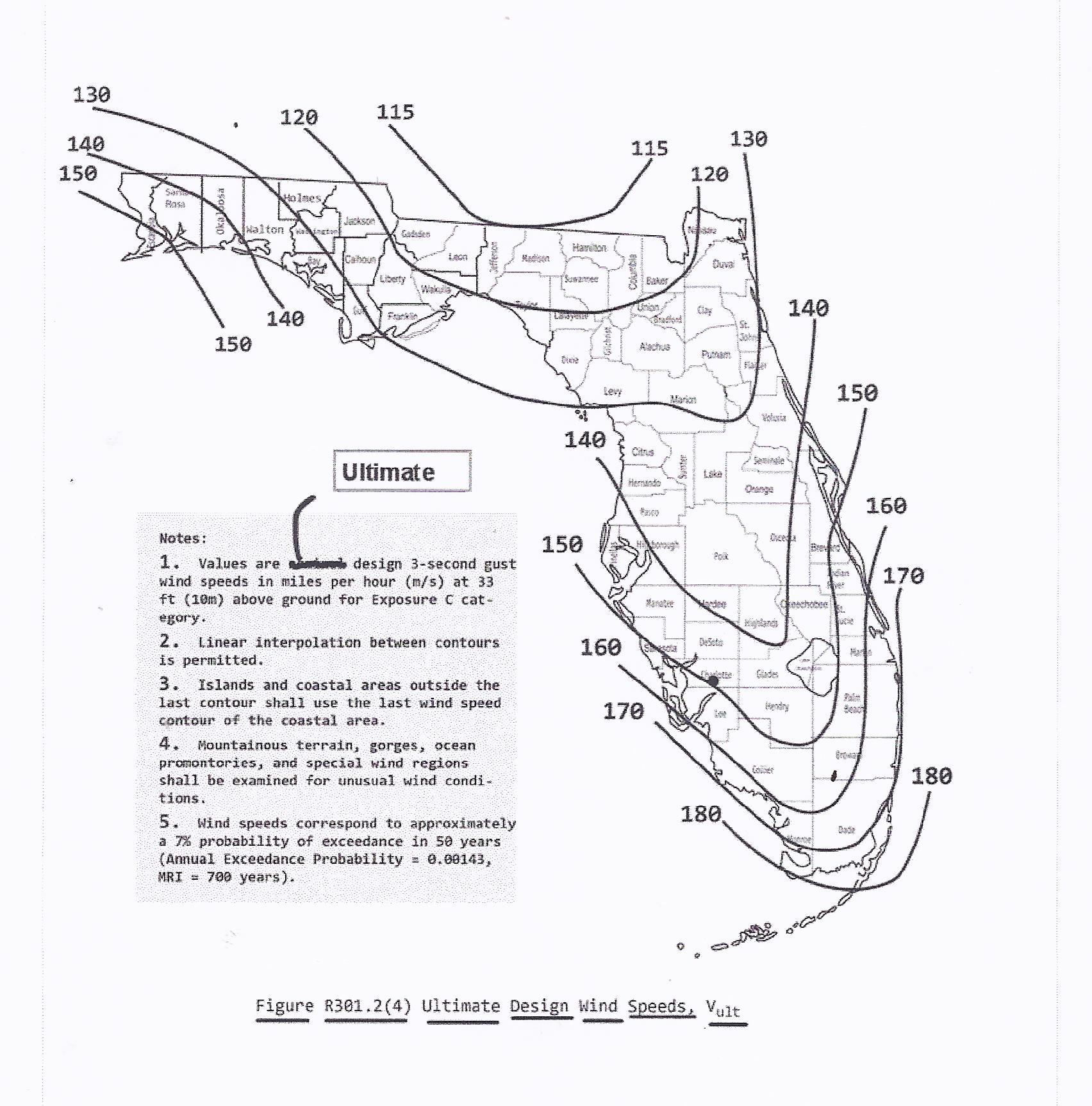
**~~FIGURE R301.2(4)B~~**

**~~REGIONS WHERE WIND DESIGN IS REQUIRED~~**

**~~FIGURE R301.2(4)C~~**

**~~WIND-BORNE DEBRIS REGIONS~~**

***Insert Figure R301.2(4) as follows:***



**[S5681 AS]**

**Mo*dify Figure R301.2(4)A - add footnote 6 as follows:***

6~~5~~. This map is accurate to the county. Local governments establish specific wind speed/wind-borne debris lines using physical landmarks such as major roads, canals, rivers, and shorelines. **(S5430AS) (S6116GlitchAS)**

***Section R301.2.1.1.1 Add a new section as shown:***

**R301.2.1.1.1 Aluminum structure design.** The AAF Guide to Aluminum Construction in High-Wind Areas shall be permitted for the construction of the aluminum structures therein addressed. Screen enclosures shall be permitted to be designed in accordance with the Florida Building Code Section 2002 and shall be permitted to be designed in accordance with Figure 1609C. Vinyl and acrylic panels shall be permitted and shall be removable. Removable panels shall be identified as removable by a decal. The identification decal shall essentially state: “Removable panel SHALL be removed when wind speeds exceed 75 mph (34 m/s).” Decals shall be placed such that the decal is visible when the panel is installed. **[S5790 AS](S6234GlitchAM)**

***R301.2.1.1.2 Add new section as shown:***

**R301.2.1.1.2 Sunroom design.**

**R301.2.1.1.2.1** Sunrooms shall comply with AAMA/NPEA/NSA 2100 as modified below.

AAMA 2100, Section 5.2.1 Wind Loads: Modify Section 5.2.1 as follows:

5.2.1 Wind Loads: Basic wind speed (Vasd) in miles per hour (mph) shall be determined in accordance with Section R301.2.1. Sunrooms including exposed structures, components, cladding, and roof covering shall be designed to resist the wind loads as established in Section R301.2.1.

**(S6117GlitchAS)**

**R301.2.1.1.2.2** For the purpose of applying the criteria of the AAMA/NPEA/NSA 2100, sunrooms shall be categorized in one of the following categories by the permit applicant, design professional or the property owner where the sunroom is being constructed.

**Category I: A Thermally Isolated Sunroom with walls that are either open or enclosed with insect screening or 0.5 mm (20 mil) maximum thickness plastic film.** The space is defined as a non-habitable, non-conditioned sunroom.

**Category II: A Thermally Isolated Sunroom with enclosed walls.** The openings are permitted to be enclosed with translucent or transparent plastic or glass. The space is defined as a non-habitable, non-conditioned sunroom.

**Category III: A Thermally Isolated Sunroom with enclosed walls.** The openings are permitted to be enclosed with translucent or transparent plastic or glass. The sunroom fenestration complies with additional requirements for air infiltration resistance and water penetration resistance. The space is defined as a non-habitable, non-conditioned sunroom.

**Category IV: A Thermally Isolated Sunroom with enclosed walls.** The sunroom is designed to be heated and or cooled by a separate temperature control or system and is thermally isolated from the primary structure. The sunroom fenestration complies with additional requirements for air infiltration resistance, water penetration resistance, and thermal performance. The space is defined as a non-habitable and conditioned sunroom.

**Category V: A Sunroom with enclosed walls.** The sunroom is designed to be heated and or cooled and is open to the main structure. The sunroom fenestration complies with additional requirements for air infiltration resistance, water penetration resistance, and thermal performance. The space is defined as a habitable and conditioned sunroom.

**[S5791 AS]**

***Add Section R301.2.1.1.3 to read as follows:***

**R301.2.1.1.3 Alternative design method for screen enclosure.**

(1) The purpose of this Section is to provide an alternate method for designing aluminum screen enclosures as defined by the Florida Building Code, permitting the loads of the structural frame to be based on portions of the screen in the screen walls removed, retracted, moved to the open position, or cut. The use of framing materials other than aluminum is allowed in accordance with Section 104.11 of the *Florida Building Code, Building*. The method applies only to walls and roofs with 100% screen.

(a) Screen enclosure frames designed in accordance with the screen removal alternates of this Section, shall be designed using signed and sealed site-specific engineering and shall be designed in accordance with the wind load provisions of the *Florida Building Code, Building*, Section 1609.1.1,

(b) Designs that consider these screen alternates shall comply with *Florida Building Code, Building,* Section 2002.4 and Table 2002.4, using the 110 mph column as modified by Table 2002.4A with all screen panels in place.

(c) Designs using strength design or load and resistance factor design in accordance with the *Florida Building Code, Building*, Section 1605.2 or allowable stress design methods of the *Florida Building Code, Building*, Section 1605.3.1 shall be permitted.

(d) The design shall be by rational analysis or by 3D Finite Element Analysis. Either method will be acceptable.

(2) Where screen enclosures are designed in accordance with the screen removal alternates of this Section, removable screen may consist of removable panels, retractable panels, or by designating specific screen panels in the design in which the screen is to be removed by cutting the screen. Removable panels shall be removed; retractable panels shall be placed in the retracted position without increasing the load on the affected area. Screen designated in the design to be cut shall be completely cut when wind speeds are forecast to exceed 75 mph.

(3) Where screen enclosures designed in accordance with the screen removal alternates of this Section serve as the barrier required by R4201.17.1, the required minimum height of the barrier shall be maintained when screen panels are retracted, removed, moved to the open position, or cut.

(4) Where screen enclosures are designed in accordance with the screen removal alternates of this Section, retractable screen panels, removable screen panels, and screen panels identified to be cut shall be clearly identified on adjacent structural members with highly visible permanent labels, at each panel, or by other means approved by the local building department.

(5) Where screen enclosures are designed in accordance with the screen removal alternates of this Section, the retraction of screen panels, removal of screen panels, or cutting of screen panels shall not require the use of ladders or scaffolding.

(6) Engineering documents submitted with building permit applications shall identify the panels to be removed, retracted, opened, or cut.

(7) Where screen enclosures are designed in accordance with the screen removal alternates of this Section based on removing screen panels by cutting the screen, the contractor shall provide replacement screen for a one-time replacement of all screen and spline designated by the design to be cut.

(8) Where screen enclosures are designed in accordance with the screen removal alternates of this Section, the contractor shall provide written notice to the owner and the local building code enforcement department that the owner must retract, remove, or cut a panel or panels of the screen enclosure in accordance with the project engineering design or the manufacturer’s instructions when wind speeds are expected to exceed 75 mph.

**(S6082GlitchAS)**

***Section R301.2.1.2 Protection of openings. Change to read as follows:***

**R301.2.1.2 Protection of openings.** ~~Exterior glazing~~ Glazed openings in buildings located in windborne debris regions shall be protected from windborne debris. Glazed opening protection for windborne debris shall meet the requirements of the Large Missile Test of ASTM E 1996 and ASTM E 1886 referenced therein, SSTD 12, TAS 201,202, and 203 or AAMA 506, as applicable. The applicable wind zones for establishing missile types in ASTM E 1996 are shown in Section R301.2.1.2.1 ~~on Figure R301.2 (4)~~ . Garage door glazed opening protection for windborne debris shall meet the requirements of an approved impact-resisting standard or ANSI/DASMA 115

**[S5663 AM R1] [S5718 AS] [S5681 AS]**

1. Opening in sunrooms, balconies or enclosed porches constructed under existing roofs or decks are not required to be protected provided the spaces are separated from the building interior by a wall and all openings in the separating wall are protected in accordance with this section. Such space shall be permitted to be designed as either partially enclosed or enclosed structures. **[S5432 AS]**

2. Storage sheds that are not designed for human habitation and that have a floor area of 720 square feet (67 m2) or less are not required to comply with the mandatory wind-borne debris impact standard of this code. **[S5432 AS]**

**Exception:** Wood structural panels with a minimum thickness of 7/16 inch (11 mm) and a maximum span of 8 feet (2438 mm) shall be permitted for opening protection in one- and two-story buildings. Panels shall be precut and attached to the framing surrounding the opening containing the product with the glazed opening. Panels shall be predrilled as required for the anchorage method and shall be secured with the attachment hardware provided. Attachments shall be designed to resist the component and cladding loads determined in accordance with either Table R301.2(2) or ASCE 7, with the permanent corrosion-resistant attachment hardware provided and anchors permanently installed on the building. Attachment in accordance with Table R301.2.1.2 is permitted for buildings with a mean roof height of 33 feet (10 058 mm) or less where Vasd determined in accordance with Section R301.2.3, does not exceed 130 miles per hour (58 m/s) ~~located in Wind Zones 1 and 2 in accordance with Figure R301.2(4)C~~.

**R301.2.1.2.1 Modifications to ASTM E 1996.** Section 6.2.2 of ASTM E 1996 shall be modified as follows:

**6.2.2 Unless otherwise specified, select the wind zone based on the basic wind speed as follows:**

**6.2.2.1WindZone 1** - 130 mph < basic wind speed < 140 mph, and Hawaii.

**6.2.2.2 Wind Zone 2 -** 140 mph < basic wind speed < 150 mph at greater than 1.6 km (one mile) from the coastline. The coastline shall be measured from the mean high water mark.

**6.2.2.3 Wind Zone 3 -** 150 mph (58 m/s) < basic wind speed < 170 mph (63 m/s), or 140 mph (54 m/s) < basic wind speed < 170 mph (63 m/s) and within 1.6 km (one mile) of the coastline.The coastline shall be measured from the mean high water mark.

**6.2.2.4 Wind Zone 4-**basic wind speed >  170 mph (63 m/s).

**(S5721 AS) (S5681 AS)**

**R301.2.1.2.1.1 Modifications to ASTM E 1886 and ASTM E 1996.**

**Table 1 of ASTM E 1886 and ASTM E 1996 –** add column and notes to read as follows:

**Air Pressure Cycles**

0.2 to 0.5 Ppos1

0.0 to 0.6 Ppos

0.5 to 0.8 Ppos

0.3 to 1.0 Ppos

0.3 to 1.0 Pneg2

0.5 to 0.8 Pneg

0.0 to 0.6 Pneg

0.2 to 0.5 Pneg

Notes:

1.      Ppos = 0.6 x positive ultimate design load in accordance with ASCE 7.

2.     Pneg = 0.6 x negative ultimate design load in accordance with ASCE 7.

**[S5681 AS]**

***Section R301.2.1.3 Wind speed conversion. Change to read as follows:***

**R301.2.1.3 Wind speed conversion.**When required ~~referenced documents are based on fastest mile wind speeds, the three-second gust basic~~ ultimate design wind speeds, Vult,  ~~wind speeds,~~ *~~V3s~~*~~,~~ of Figure R301.2(4) shall be converted to ~~fastest mile~~ nominal design wind speeds, Vasd *~~V~~~~fm~~*~~,~~ using Table R301.2.1.3.

***Delete existing Table R301.2.1.3 and replace with the following:***

**TABLE R301.2.1.3**

**WIND SPEED CONVERSIONSabc**

|  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| Vult | 100 | 110 | 120 | 130 | 140 | 150 | 160 | 170 | 180 | 190 | 200 |
| Vasd | 78 | 85 | 93 | 101 | 108 | 116 | 124 | 132 | 139 | 147 | 155 |

a.   Linear interpolation is permitted

b.   Vasd = nominal design wind speed

c.   Vult = ultimate design wind speed determined from Figure R301.2(4).

**[S5681 AS]**

***Section R301.2.1.4. Exposure category. Change to read as follows:***

**R301.2.1.4 Exposure category.** For each wind direction considered, an exposure category that adequately reflects the characteristics of ground surface irregularities shall be determined for the site at which the building or structure is to be constructed. For a site located in the transition zone between categories, the category resulting in the largest wind forces shall apply. Account shall be taken of variations in ground surface roughness that arise from natural topography and vegetation as well as from constructed features. For any given wind direction, the exposure in which a specific building or other structure is sited shall be assessed as being one of the following categories. ~~. For a site where multiple detached one- and two-family dwellings,~~ *~~townhouses~~* ~~or other structures are to be constructed as part of a subdivision, master-planned community, or otherwise designated as a developed area by the authority having jurisdiction, the exposure category for an individual structure shall be based upon the site conditions that will exist at the time when all adjacent structures on the site have been constructed, provided their construction is expected to begin within one year of the start of construction for the structure for which the exposure category is determined. For any given wind direction, the exposure in which a specific building or other structure is sited shall be assessed as being one of the following categories:~~

~~1. Exposure A. Large city centers with at least 50 percent of the buildings having a height in excess of 70 feet (21 336 mm). Use of this exposure category shall be limited to those areas for which terrain representative of Exposure A prevails in the upwind direction for a distance of at least 0.5 mile (0.8 km) or 10 times the height of the building or other structure, whichever is greater. Possible channeling effects or increased velocity pressures due to the building or structure being located in the wake of adjacent buildings shall be taken into account.~~

~~2. Exposure B. Urban and suburban areas, wooded areas, or other terrain with numerous closely spaced obstructions having the size of single family dwellings or larger. Exposure B shall be assumed unless the site meets the definition of another type exposure.~~

~~3. Exposure C. Open terrain with scattered obstructions, including surface undulations or other irregularities, having heights generally less than 30 feet (9144 mm) extending more than 1,500 feet (457 m) from the building site in any quadrant. This exposure shall also apply to any building located within Exposure B type terrain where the building is directly adjacent to open areas of Exposure C type terrain in any quadrant for a distance of more than 600 feet (183 m). This category includes flat, open country and grasslands.~~

~~4. Exposure D. Flat, unobstructed areas exposed to wind flowing over open water for a distance of at least 1 mile (1.61 km). Shorelines in Exposure D include inland waterways, the Great Lakes, and coastal areas of California, Oregon, Washington and Alaska. This exposure shall apply only to those buildings and other structures exposed to the wind coming from over the water. Exposure D extends inland from the shoreline a distance of 1500 feet (457 m) or 10 times the height of the building or structure, whichever is greater.~~

**Exception**: An intermediate exposure between the exposure categories defined is permitted in a transition zone provided that it is determined by a rational analysis method.

**R301.2.1.4.1 Wind directions and sectors.** For each selected wind direction at which the wind loads are to be evaluated, the exposure of the building or structure shall be determined for the two upwind sectors extending 45 degrees (0.79 rad) either side of the selected wind direction. The exposures in these two sectors shall be determined in accordance with Sections R301.2.4.2 and R301.2.1.4.3 and the exposure resulting in the highest wind loads shall be used to represent winds from that direction.

**R301.2.1.4.2 Surface roughness categories.** A ground surface roughness within each 45-degree 4(0.79 rad) sector shall be determined for a distance upwind of the site as defined in Section R301.2.1.4.3 from the categories defined below, for the purpose of assigning an exposure category as defined in Section R301.2.1.4.3.

**Surface Roughness B.** Urban and suburban areas, wooded areas or other terrain with numerous closely spaced obstructions having the size of single-family dwellings or larger.

**Surface Roughness C.** Open terrain with scattered obstructions having heights generally less than 30 feet (9144 mm). This category includes flat open country and grasslands. This surface roughness shall also apply to any building located within surface roughness B-type terrain where the building is within 100 feet horizontally in any direction of open areas of surface roughness C or D-type terrain that extends more than 600 feet (182.9 m) and width greater than 150 ft. in the upwind direction. Short-term (less than two year) changes in the pre-existing terrain exposure, for the purposes of development, shall not be considered surface roughness C. Where development buildout will occur within three years and the resultant condition will meet the definition of surface roughness B, surface roughness B shall be regulating for the purpose of permitting. This category includes flat open country and grasslands and shall extend downwind for a distance of 1500 feet.

**Surface Roughness D.** Flat, unobstructed areas and water surfaces. This category includes smooth mud flats, salt flats and unbroken ice.

**R301.2.1.4.3 Exposure categories.** An exposure category shall be determined in accordance with the following:

**Exposure B.** For buildings with a mean roof height of less than or equal to 30 feet, Exposure B shall apply where the ground surface roughness, as defined by Surface Roughness B, prevails in the upwind direction for a distance of at least 1,500 ft (457 m). For buildings with a mean roof height greater than 30 ft, Exposure B shall apply where Surface Roughness B prevails in the upwind direction for a distance of at least 2,600 feet (792 m) or 20 times the height of the building, whichever is greater.

**Exposure C.** Exposure C shall apply for all cases where Exposures B or D do not apply.

**Exposure D.** Exposure D shall apply where the ground surface roughness, as defined by Surface Roughness D, prevails in the upwind direction for a distance of at least 5,000 feet (1524 m) or 20 times the height of the building, whichever is greater. Exposure D shall also apply where the ground surface roughness immediately upwind of the site is B or C, and the site is within a distance of 600ft (183 m) or 20 times the building height, whichever is greater, from an exposure D condition as defined in the previous sentence.

**[S5694 AS]**

***Section R301.2.1.6. Add the following section to read as follows:***

**R301.2.1.6 Testing to allowable or nominal loads.** Where testing for wind load resistance is based on allowable or nominal wind loads, the design wind loads determined in accordance with ASCE 7 or Section 1609 of the *Florida Building Code, Building* are permitted to be multiplied by 0.6 for the purposes of the wind load resistance testing. **[S5700 AS]**

**R301.2.1.7 Basic wind speed.** The ultimate design wind speed, Vult, in miles per hour, for the development of windloads, shall be determined from Figure R301.2(4). The exact location of wind speed lines shall be established by local ordinance using recognized physical landmarks such as major roads, canals, rivers and lake shores whenever possible.

**(S6180GlitchAM)**

***Section R301.2.2.3.1 Height limitations. Change to read as follows:***

**R301.2.2.3.1 Height limitations.** Wood-framed buildings shall be limited to three stories above *grade* plane ~~or the limits given in Table R602.10.3(3)~~. Cold-formed, steel-framed buildings shall be limited to less than or equal to three stories above *grade* plane in accordance with AISI S230. Mezzanines as defined in Section R202 shall not be considered as stories. Structural insulated panel buildings shall be limited to two stories above *grade* plane.

**(S6119GlitchAS)**

***Section R301.2.4.1. Change to read as follows:***

**R301.2.4.1 Alternative provisions**. As an alternative to the requirements in Section R322, ~~R322.3 for buildings and structures located in whole or in part in coastal high-hazard areas (V Zones) and Coastal A Zones, if delineated,~~ ASCE 24 is permitted subject to the limitations of this code and the limitations therein. **[SP 5292 AS][F5902 AS]**

***R301.2.5 Add to read as follows:***

**R301.2.5 Structures seaward of a coastal control construction line.** Structures located seaward of the coastal construction control line shall be designed to resist the predicted forces of a 100-year storm event in accordance with Section 3109 of the *Florida Building Code, Building*.

***Section R301.3 Story height. Change to read as follows:***

|  |
| --- |
| **R301.3 Story height.** The wind and seismic provisions of this code shall apply to buildings with story heights not exceeding the following:   1. For wood wall framing, the laterally unsupported bearing wall stud height permitted by Section R602 ~~Table R602.3(5)~~ plus a height of floor framing not to exceed 16 inches (406 mm).  **~~Exception:~~** ~~For wood-framed wall buildings with bracing in accordance with Section R602 Tables R602.10.3(1) and R602.10.3(3), the wall stud clear height used to determine the maximum permitted~~ *~~story height~~* ~~may be increased to 12 feet (3658 mm) without requiring an engineered design for the building wind and seismic force-resisting systems provided that the length of bracing required by Section R602 Table R602.10.3(1) is increased by multiplying by a factor of 1.10 and the length of bracing required by Table R602.10.3(3) is increased by multiplying by a factor of 1.20. Wall studs are still subject to the requirements of this section.~~   (no change to remainder of text) |
| **(S6120GlitchAS) (Post-Glitch Comment AM)** |

***Section R301.9. Add to read as follows:***

**R301.9** All exterior wall coverings and soffits shall be capable of resisting the design pressures specified for walls for components and cladding loads in accordance with Table R301.2(2) as modified by Table R301.2(3), Manufactured soffits shall be tested at 1.5 times the design pressure.

**[S5743 AS]**

***Section R302.1 Exterior walls. Change to read as follows:***

**R302.1 Exterior walls.** Construction, projections, openings and penetrations of exterior walls of dwellings and accessory buildings shall comply with Table R302.1(1); or dwellings equipped throughout with an automatic sprinkler system installed in accordance with Section P2904 shall comply with Table R302.1(2).

**Exceptions:**

1.-5. [No change]

6. Screen enclosure walls of insectscreening with a maximum of 25% solid flexible finishes.

**[F5902 AS]**

***Section R302.2. Townhouses. Change to read as shown:***

**R302.2 Townhouses.** Each *townhouse* shall be considered a separate building and shall be separated by fire-resistance- rated wall assemblies meeting the requirements of Section R302.1 for exterior walls.

**Exception:** A common ~~1-hour~~ 2-hour fire-resistance-rated wall assembly tested in accordance with ASTME 119 or UL 263 is permitted for townhouses if such walls do not contain plumbing or mechanical equipment, ducts or vents in the cavity of the common wall, unless such materials and methods of penetration comply with Section R302.4. The wall shall be rated for fire exposure from both sides and shall extend to and be tight against exterior walls and the underside of the roof sheathing. Electrical installations shall be installed in accordance with Chapter~~s~~ 34 ~~through 43~~. Penetrations of electrical outlet boxes shall be in accordance with Section R302.4.

**[F6011 AS]**

***Section R302.3. Revise to read as shown:***

**R302.3 Two-family dwellings.** *Dwelling units* in two-family dwellings shall be separated from each other by wall and/or floor assemblies having not less than a 1-hour fire-resistance rating when tested in accordance with ASTM E 119 or UL 263. Fire-resistance-rated floor/ceiling and wall assemblies shall extend to and be tight against the *exterior wall*, and wall assemblies shall extend from the foundation to the underside of the roof sheathing.

**Exceptions:**

1. – 2. [No change]

3. Screen enclosure walls of insect screening with a maximum of 25 percent solid flexible finishes.

**[F5904 AS]**

***R302.5.2 Duct penetration. Change to read as shown:***

**R302.5.2 Duct penetration**. Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel, 1 inch minimum rigid nonmetallic Class 0 or Class 1 duct board, or other approved material and shall have no openings into the garage,

**[F5417 AS] [M5868 AS ]**

***Section R302.13Combustible insulation clearance. Change to read as shown:***

**R302.13 Combustible insulation clearance.** Combustible insulation shall be separated a minimum of 3 inches (76 mm) from recessed luminaires, fan motors and other heat-producing devices.

**Exception:** Where heat-producing devices are listed for lesser clearances, combustible insulation complying with the listing requirements shall be separated in accordance with the conditions stipulated in the listing.

Recessed luminaires installed in the building thermal envelope shall meet the requirements of Section R402.4.4 of the *Florida Building Code, Energy Conservation* ~~N1102.4.4 of this code~~.

**(Code correlation)**

***Section R306.3 Sewage disposal. Change to read as shown:***

**R306.3 Sewage disposal.** All plumbing fixtures shall be connected to a sanitary sewer or to an approved private sewage disposal system in accordance with Chapter 64E-6, Florida Administrative Code, Standards for Onsite Sewage Treatment and Disposal Systems.

**[SP5434 AS]**

***Section R308.1 Identification. Change to read as shown:***

**R308.1 Identification.** Each pane shall bear the manufacturer's label designating the type and thickness of glass or glazing material. Except as indicated in [Section R308.1.1](javascript:Next('./icod_irc_2012_3_sec008_par001.htm');) each pane of glazing installed in hazardous locations as defined in [Section R308.4](javascript:Next('./icod_irc_2012_3_sec008_par006.htm');) shall be provided with a manufacturer’s or installer's label, ~~designation specifying who applied the designation,~~ designating the type of glass and the safety glazing standard with which it complies, which is visible in the final installation. The ~~designation~~ safety glazing label shall be acid etched, sandblasted, ceramic-fired, laser etched, embossed, or be of a type which once applied cannot be removed without being destroyed. ~~A~~ *~~label~~* ~~shall be permitted in lieu of the manufacturer’s designation.~~ Laminated glass, other than used for safety glazing, shall be permanently identified as per this section, designating laminator, overall glass thickness and trade name of interlayer.

**Exceptions:**

1. For other than tempered glass, manufacturer’s designations are not required provided the *building official* approves the use of a certificate, affidavit or other evidence furnished by the gazing contractor certifying that each light id glazed in accordance with approved construction documents that comply with the provisions of this chapter confirming compliance with this code.

2. Tempered spandrel glass is permitted to be identified by the manufacturer with a removable paper ~~designation~~ label

**[S5822 AS]**

***R310.1 Emergency escape and rescue required. Change to read as follows:***

**R310.1 Emergency escape and rescue required. [No change to text; exceptions only]**

**Exceptions: 7**

**1.** Basements used only to house mechanical equipment and not exceeding total floor area of 200 square feet (18.58 m2).

**2.** The emergency escape and rescue opening shall be permitted to open into a screen enclosure, open to the atmosphere, where a screen door is provided leading away from the residence.

**[F5419 AS]**

***R310.4 Bars, grills, covers and screens. Change to read as shown:***

**R310.4 Bars, grills, covers and screens.** Bars, grills, covers, screens or similar devices are permitted to be placed over emergency escape and rescue openings, bulkhead enclosures, or window wells that serve such openings, provided the minimum net clear opening size complies with Sections R310.1.1 to R310.1.3, and such devices shall be releasable or removable from the inside without the use of a key, tool, special knowledge or force greater than that which is required for normal operation of the escape and rescue opening. The temporary installation or closure of storm shutters, panels, and other approved hurricane protection devices shall be permitted on emergency escape and rescue openings during the threat of a storm. Such devices shall not be required to comply with the operational constraints of Section R310.1.4. While such protection is provided, at least one means of escape from the dwelling or dwelling unit shall be provided. The means of escape shall be within the first floor of the dwelling or dwelling unit and shall not be located within a garage without a side-hinged door leading directly to the exterior.

Occupants in any part of the dwelling or dwelling unit shall be able to access the means of escape without passing through a lockable door not under their control.

**[F5717 AS]**

***Section R311.2 Egress door. Add exception to read as follows:***

**R311.2 Egress door.** At least one egress door shall be provided for each dwelling unit. The egress door shall be side-hinged, and shall provide a minimum clear width of 32 inches (813 mm) when measured between the face of the door and the stop, with the door open 90 degrees (1.57 rad). The minimum clear height of the door opening shall not be less than 78 inches (1981 mm) in height measured from the top of the threshold to the bottom of the stop. Other doors shall not be required to comply with these minimum dimensions. Egress doors shall be readily openable from inside the dwelling without the use of a key or special knowledge or effort.

**Exception:** Buildings that are 400 square feet or less and that are intended for use in conjunction with one- and two-family residences are not subject to the door height and width requirements of this code.

**[SP5436 AS]**

***Section R311.3 Floors and landings at exterior doors. Change to read as shown:***

**R311.3 Floors and landings at exterior doors.** There shall be a landing or floor on each side of each exterior door. The width of each landing shall not be less than the door served. Every landing shall have a minimum dimension of 36 inches (914 mm) measured in the direction of travel. Exterior landings shall be permitted to have a slope not to exceed 1/4 unit vertical in 12 units horizontal (2-percent).

**Exception:** Exterior balconies less than 60 square feet (5.6m2) and only accessible from a door are permitted to have a landing less than 36 inches (914 mm) measured in the direction of travel.

**R311.3.1 Floor elevations at the required egress doors.** Landings or finished floors at the required egress door shall not be more than l ½ inches (38 mm) lower than the top of the threshold.

**Exception:** The landing or floor on the exterior side shall not be more than 7-3/4 inches (196 mm) below the top of the threshold ~~provided the door does not swing over the landing or floor~~.

Where exterior landings of floors serving the required egress door are not at grade, they shall be provided with access to grade by means of a ramp in accordance with Section R311.8 or a stairway in accordance with Section R311.7.

**R311.3.2 Floor elevations for other exterior doors.** Doors other than the required egress door shall be provided with landings or floors not more than 7-3/4, inches (196 mm) below the top of the threshold.

**Exception:** A landing is not required where a stairway of two or fewer risers is located on the exterior side of the door. ~~, provided the door does not swing over the stairway.~~

**[F5415 AS]**

***Section R311.7.6, Landings for stairways. Add exception 2 as shown:***

**R311.7.6 Landings for stairways.** There shall be a floor or landing at the top and bottom of each stairway. The minimum width perpendicular to the direction of travel shall be no less than the width of the flight served. Landings of shapes other than square or rectangular shall be permitted provided the depth at the walk line and the total area is not less than that of a quarter circle with a radius equal to the required landing width. Where the stairway has a straight run, the minimum depth in the direction of travel shall be not less than 36 inches (914 mm).

**Exceptions:**

1.   A floor or landing is not required at the top of an interior flight of stairs, including stairs in an enclosed garage, provided a door does not swing over the stairs.

2.   See Section R311.3 from exterior doors where a step down is provided.

**[F5415 AS]**

***Section R313 Automatic Fire Sprinkler. Change to read as follows:***

**SECTION R313**

**AUTOMATIC FIRE SPRINKLER**

**RESERVED**

**~~R313.1 Townhouse automatic fire sprinkler systems.~~** ~~An automatic residential fire sprinkler system shall be installed in~~ *~~townhouses~~*~~.~~

**~~Exception:~~** ~~An automatic residential fire sprinkler system shall not be required when~~ *~~additions~~* ~~or~~ *~~alterations~~* ~~are made to existing~~ *~~townhouses~~* ~~that do not have an automatic residential fire sprinkler system installed.~~

**~~R313.1.1 Design and installation.~~** ~~Automatic residential fire sprinkler systems for~~ *~~townhouses~~* ~~shall be designed and installed in accordance with~~ [~~Section P2904~~](javascript:Next('./icod_irc_2012_29_sec004.htm');)~~.~~

**~~R313.2 One- and two-family dwellings automatic fire systems.~~** ~~An automatic residential fire sprinkler system shall be installed in one- and two-family~~ *~~dwellings~~*~~.~~

**~~Exception:~~** ~~An automatic residential fire sprinkler system shall not be required for~~ *~~additions~~* ~~or~~ *~~alterations~~* ~~to existing buildings that are not already provided with an automatic residential sprinkler system.~~

**~~R313.2.1 Design and installation.~~** ~~Automatic residential fire sprinkler systems shall be designed and installed in accordance with Section P2904 or NFPA 13D.~~

**[F5437 AS]**

***R315.1. Carbon monoxide alarms. Replace Section R315.1 to read as shown:***

**SECTION R315**

**CARBON MONOXIDE ALARMS**

**R315.1 Carbon monoxide protection ~~alarms~~.** Every separate building or an addition to an existing building for which a permit for new construction is issued and having a fossil-fuel-burning heater or appliance, a fireplace, an attached garage, or other feature, fixture, or element that emits carbon monoxide as byproduct of combustion shall have an operational carbon monoxide alarm installed within 10 feet of each room used for sleeping purposes. ~~For new construction, an approved carbon monoxide alarm shall be installed outside of each separate sleeping area in the immediate vicinity of the bedrooms in~~ *~~dwelling units~~* ~~within which fuel-fired~~ *~~appliances~~* ~~are installed and in dwelling units that have attached garages.~~

**Exception:** This section shall not apply to existing buildings that are undergoing alterations or repair unless the alteration is an addition as defined in Section R315.1.3.

**R315.1.1 Carbon monoxide alarm.** The requirements of Section R315.1 shall be satisfied by providing for one of the following alarm installations:

(1) A hard-wired carbon monoxide alarm.

(2) A battery-powered carbon monoxide alarm.

(3) A hard-wired combination carbon monoxide and smoke alarm.

(4) A battery-powered combination carbon monoxide and smoke alarm.

**R315.1.2 Combination alarms.** Combination smoke/carbon monoxide alarms shall be listed and labeled by a Nationally Recognized Testing Laboratory.

**R315.1.3** **Addition shall mean**: An extension or increase in floor area, number of stories or height of a building or structure.

**315.2 Carbon monoxide detection systems. Reserved.** ~~Carbon monoxide detection systems that include carbon monoxide detectors and audible notification appliances, installed and maintained in accordance with this section for carbon monoxide alarms and NFPA 720, shall be permitted. The carbon monoxide detectors shall be listed as complying with UL 2075. Where a household carbon monoxide detection system is installed, it shall become a permanent fixture of the occupancy, owned by the homeowner and shall be monitored by an approved supervising station.~~

**~~Exception:~~** ~~Where carbon monoxide alarms are installed meeting the requirements of~~ [~~Section R315.1~~](javascript:Next('./icod_irc_2012_3_sec015.htm');)~~, compliance with Section 315.2 is not required.~~

**R315.3 Where required in existing dwellings**. Reserved . **~~Where required in existing dwellings.~~** ~~Where work requiring a~~ *~~permit~~* ~~occurs in existing~~ *~~dwellings~~* ~~that have attached garages or in existing dwellings within which fuel-fired~~ *~~appliances~~* ~~exist, carbon monoxide alarms shall be provided in accordance with~~ [~~Section R315.1.~~](javascript:Next('./icod_irc_2012_3_sec015.htm');)

**R315.4 Alarm requirements.** Reserved. ~~Single-station carbon monoxide alarms shall be listed as complying with UL 2034 and shall be installed in accordance with this code and the manufacturer’s installation instructions.~~

**[F5438 AM R1]**

***Section R318 Protection against subterrannean termites . Replace with Florida specific criteria to read as shown:***

**SECTION R318**

**PROTECTION AGAINST TERMITES**

**R318.1** **Termite Protection.** Termite protection shall be provided by registered termiticides, including soil applied pesticides, baiting systems, and pesticides applied to wood, or other approved methods of termite protection labeled for use as a preventative treatment to new construction. See §202, REGISTERED TERMITICIDE. Upon completion of the application of the termite protective treatment, a Certificate of Compliance shall be issued to the building department by the licensed pest control company that contains the following statement: "The building has received a complete treatment for the prevention of subterranean termites. Treatment is in accordance with rules and laws established by the Florida Department of Agriculture and Consumer Services."

**R318.1.1** If soil treatment used for subterranean termite prevention, the initial chemical soil treatment inside the foundation perimeter shall be done after all excavation, backfilling and compaction is complete.

**R318.1.2**If soil treatment is used for subterranean termite prevention, soil area disturbed after initial chemical soil treatment shall be retreated with a chemical soil treatment, including spaces boxed or formed.

**R318.1.3** If soil treatment is used for subterranean termite prevention, space in concrete floors boxed out or formed for the subsequent installation of plumbing traps, drains or any other purpose shall be created by using plastic or metal permanently placed forms of sufficient depth to eliminate any planned soil disturbance after initial chemical soil treatment.

**R318.1.4** If soil treatment is used for subterranean termite prevention, chemically treated soil shall be protected with a minimum 6 mil vapor retarder to protect against rainfall dilution. If rainfall occurs before vapor retarder placement, retreatment is required. Any work, including placement of reinforcing steel, done after chemical treatment until the concrete floor is poured, shall be done in such manner as to avoid penetrating or disturbing treated soil.

**R318.1.5** If soil treatment is used for subterranean termite prevention, concrete overpour or mortar accumulated along the exterior foundation perimeter shall be removed prior to exterior chemical soil treatment, to enhance vertical penetration of the chemicals.

**R318.1.6** If soil treatment is used for subterranean termite prevention, chemical soil treatments shall also be applied under all exterior concrete or grade within 1 foot (305 mm) of the primary structure sidewalls. Also, a vertical chemical barrier shall be applied promptly after construction is completed, including initial landscaping and irrigation/sprinkler installation. Any soil disturbed after the chemical vertical barrier is applied shall be promptly retreated.

**R318.1.7** If a registered termiticide formulated and registered as a bait system is used for subterranean termite prevention, §R318.1.1 through §R318.1.6 do not apply; however, a signed contract assuring the installation, maintenance and monitoring of the baiting system that is in compliance with the requirements of Chapter 482, F.S. ~~for a minimum of five years from the issue of the Certificate of Occupancy~~ shall be provided to the building official prior to the pouring of the slab, and the system must be installed prior to final building approval.

If the baiting system directions for use require a monitoring phase prior to installation of the pesticide active ingredient, the installation of the monitoring phase components shall be deemed to constitute installation of the system.

**(SP6044GlitchAS)**

**R318.1.8** If a registered termiticide formulated and registered as a wood treatment is used for subterranean termite prevention, Sections R318.1.1 through R318.1.6 do not apply. Application of the wood treatment termiticide shall be as required by label directions for use, and must be completed prior to final building approval.

**R318.2 Penetration.** Protective sleeves around piping penetrating concrete slab-on-grade floors shall not be of cellulose-containing materials. If soil treatment is used for subterranean termite protection, the sleeve shall have a maximum wall thickness of 0.010 inch (0.25 mm), and be sealed within the slab using a non-corrosive clamping device to eliminate the annular space between the pipe and the sleeve. No termiticides shall be applied inside the sleeve.

**R318.3 Cleaning.** Cells and cavities in masonry units and air gaps between brick, stone or masonry veneers and the structure shall be cleaned of all non-preservative treated or non-naturally durable wood, or other cellulose-containing material prior to concrete placement.

**Exception:**  Inorganic material manufactured for closing cells in foundation concrete masonry unit construction or clean earth fill placed in concrete masonry unit voids below slab level before termite treatment is performed.

**R318.4 Concrete bearing ledge.** Brick, stone or other veneer shall be supported by a concrete bearing ledge at least equal to the total thickness of the brick, stone or other veneer which is poured integrally with the concrete foundation. No supplemental concrete foundation pours which will create a hidden cold joint shall be used without supplemental treatment in the foundation unless there is an approved physical barrier. An approved physical barrier shall also be installed from below the wall sill plate or first block course horizontally to embed in a mortar joint. If masonry veneer extends below grade, a termite protective treatment must be applied to the cavity created between the veneer and the foundation, in lieu of a physical barrier.

**Exception:** Veneer supported by a structural member secured to the foundation sidewall in accordance with ACI 530/ASCE 5/TMS 402, provided at least a 6 inch (152 mm) clear inspection space of the foundation sidewall exterior exist between the veneer and the top of any soil, sod, mulch or other organic landscaping component, deck, apron, porch, walk or any other work immediately adjacent to or adjoining the structure.

**R318.5 Protection against decay and termites.** Condensate Lines, irrigation/sprinkler system risers for spray heads, and roof downspouts shall discharge at least 1 foot (305 mm) away from the structure sidewall, whether by underground piping, tail extensions or splash blocks. Gutters with downspouts are required on all buildings with eaves of less than 6 inches (152 mm) horizontal projection except for gable end rakes or on a roof above another roof.

**R318.6 Preparation of building site and removal of debris.**

**R318.6.1** All building sites shall be graded to provide drainage under all portions of the building not occupied by basements.

**R318.6.2** The foundation and the area encompassed within 1 foot (305 mm) therein shall have all vegetation, stumps, dead roots, cardboard, trash and foreign material removed and the fill material shall be free of vegetation and foreign material. The fill shall be compacted to assure adequate support of the foundation.

**R318.6.3** After all work is completed, loose wood and debris shall be completely removed from under the building and within 1 foot (305 mm) thereof. All wood forms and supports shall be completely removed. This includes, but is not limited to: wooden grade stakes, forms, contraction spacers, tub trap boxes, plumbing supports, bracing, shoring, forms or other cellulose-containing material placed in any location where such materials are not clearly visible and readily removable prior to completion of the work. Wood shall not be stored in contact with the ground under any building.

***[Florida specific]***

***Add new Section R318.7 as shown:***

**R318.7 Inspection for termites.** In order to provide for inspection for termite infestation, clearance between exterior wall coverings and final earth grade on the exterior of a building shall not be less than 6 inches (152 mm).

**Exceptions:**

l. Paint or *decorative cementitious finish* less than 5/8 inch (17.1 mm) thick adhered directly to the masonry foundation sidewall.

2. Access or vehicle ramps which rise to the interior finish floor elevation for the width of such ramps only.

3. A 4-inch (102 mm) inspection space above patio and garage slabs and entry areas.

4. If the patio has been soil treated for termites, the finish elevation may match the building interior finish floor elevations on masonry construction only.

5. Masonry veneers constructed in accordance with Section R318.4.

**[SP5416 AS]**

***Section R318.8. Add section to read as shown:***

**R318.8 Foam plastic protection.**   
In areas where the probability of termite infestation is "very heavy” as indicated in Figure R301.2(6), extruded and expanded polystyrene, polyisocyanurate and other foam plastics shall not be installed on the exterior face or under interior or exterior foundation walls or slab foundations located below grade. The clearance between foam plastics installed above grade and exposed earth shall be at least 6 inches (152 mm).

**Exceptions:**

1. Buildings where the structural members of walls, floors, ceilings and roofs are entirely of noncombustible materials or pressure-preservative-treated wood.

2. When in addition to the requirements of Section R318.1, an approved method of protecting the foam plastic and structure from subterranean termite damage is used.

3. On the interior side of basement walls.

**(S6181GlitchAS)**

**SECTION R320**

**ACCESSIBILITY**

***Section R320.1 Scope. Revise to read as shown:***

**R320.1 Scope.** Shall be in accordance with the provisions of the *Florida Building Code, Accessibility.*~~Where there are four or more~~ *~~dwelling~~* ~~units or sleeping units in a single structure, the provisions of Chapter 11 of the~~ *~~International Building Code~~* ~~for Group R-3 shall apply.~~

**R320.1.1** All new single-family houses, duplexes, triplexes, condominiums and townhouses shall provide at least one bathroom, located with maximum possible privacy, where bathrooms are provided on habitable grade levels, with a door that has a 29-inch (737 mm) clear opening. However, if only a toilet room is provided at grade level, such toilet rooms shall have a clear opening of not less than 29 inches (737 mm).

**[A5439 AS]**

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| ***Section R321.3 Accessibility. Revise to read as shown****:*  **R321.3 Accessibility.** Reserved. ~~Elevators or platform lifts that are part of an accessible route required by Chapter 11 of the~~ *~~International Building Code~~*~~, shall comply with ICC A117.1.~~ |
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**[A5440 AS]**

***R322.1.1 Alternative provisions. Revise to read as shown:***

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| **R322.1.1 Alternative provisions**. As an alternative to the requirements in Section R322, ~~R322.3 for buildings and structures located in whole or in part in coastal high-hazard areas (V Zones) and Coastal A Zones, if delineated,~~ ASCE 24 is permitted subject to the limitations of this code and the limitations therein. **[SP 5292 AS]** |

***Section R322.1.7 Protection of water supply and sanitary sewage systems. Revise to read as shown:***

**R322.1.7 Protection of water supply and sanitary sewage systems.** New and replacement water supply systems shall be designed to minimize or eliminate infiltration of flood waters into the systems in accordance with the plumbing provisions of this code. New and replacement sanitary sewage systems shall be designed to minimize or eliminate infiltration of floodwaters into systems and discharges from systems into floodwaters in accordance with the plumbing provisions of this code and in accordance with Chapter 64E-6, Florida Administrative Code, Standards for Onsite Sewage Treatment and Disposal Systems. ~~and Chapter 3 of the~~ *~~International Private Sewage Disposal Code~~*~~.~~

***Section R322.1.9 Manufactures homes. Revise to read as shown:***

**R322.1.9 Manufactured homes.**  In addition to the applicable requirements of the state agency with jurisdiction over installation of manufactured homes, installation of manufactured homes in flood hazard areas is subject to the applicable provisions of the local floodplain management ordinance.

***Section R322.1.11 Structural seaward of a coastal construction line. Add section to read as shown:***

**R322.1.11 Structures seaward of a coastal construction line.** In addition to the requirements of this section, structures located in flood hazard areas and seaward of the coastal construction line shall be designed to resist the predicted forces of a 100-year storm event in accordance with Section R3109 of the *Florida Building Code, Building,* and the more restrictive provisions shall govern.

***Section R322.2.4 Pools in flood hazard areas. Add to read as shown:***

**R322.2.4 Pools in flood hazard areas.** Pools that are located in flood hazard areas established by Table R301.2(1), including above-ground pools, on-ground pools, and in-ground pools that involve placement of fill, shall comply with Sections R322.2.4.1 or RB322.2.4.2.

**Exception:** Pools located in riverine flood hazard areas which are outside of designated floodways.

**R322.2.4.1 Pools located in designated floodways.** Where pools are located in designated floodways, documentation shall be submitted to the building official, which demonstrates that the construction of the pool will not increase the design flood elevation at any point within the jurisdiction.

**R322.2.4.2 Pools located where floodways have not been designated.** Where pools are located in riverine flood hazard areas where design flood elevations are specified but floodways have not been designated, the applicant shall provide a floodway analysis that demonstrates that the proposed pool will not increase the design flood elevation more than 1 foot (305 mm) at any point within the jurisdiction.

**[SP5294 AS]**

***Section R322.3.2 Elevation requirements. Revise to read as shown:***

**R322.3.2 Elevation requirements.**

1. All buildings and structures erected within coastal high-hazard areas shall be elevated so that the lowest portion of all structural members supporting the lowest floor, with the exception of piling, pile caps, columns, grade beams and bracing, is elevated to or above the design flood elevation. ~~:~~

~~1.1. Located at or above the design flood elevation, if the lowest horizontal structural member is oriented parallel to the direction of wave approach, where parallel shall mean less than or equal to 20 degrees (0.35 rad) from the direction of approach, or~~

~~1.2. Located at the base flood elevation plus 1 foot (305 mm), or the design flood elevation, whichever is higher, if the lowest horizontal structural member is oriented perpendicular to the direction of wave approach, where perpendicular shall mean greater than 20 degrees (0.35 rad) from the direction of approach.~~

2. – 4. [No change]

**[SP 5295 AS]**

***Section R322.3.3.1 Pools. Add new section to read as shown:***

**R322.3.3.1 Pools.** Pools in coastal high-hazard areas shall be designed and constructed in conformance with ASCE 24.

**[SP5294 AS]**

***Section R322.3.4 Walls below design flood elevation. Revise to read as shown:***

**R322.3.4 Walls below design flood elevation.** Walls and partitions are permitted below the elevated floor, provided that such walls and partitions are not part of the structural support of the building or structure and:

1. Electrical, mechanical, and plumbing system components are not to be mounted on or penetrate through walls that are designed to break away under flood loads; and

2. Are constructed with insect screening or open lattice; or

3. Are designed to break away or collapse without causing collapse, displacement or other structural damage to the elevated portion of the building or supporting foundation system. Such walls, framing and connections shall have a ~~design safe loading~~ resistance of not less than 10 (479 Pa) and no more than 20 pounds per square foot (958 Pa) determined using allowable stress design; or

4. Where wind loading values of this code exceed 20 pounds per square foot (958 Pa) determined using allowable stress design, the *construction documents* shall include documentation prepared and sealed by a registered design professional that:

4.1 – 4.2 [No change]

**[SP5296 AS]**

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| ***Section R324. Add a new section to read as shown:***  **SECTION R324**  **AIRPORT NOISE**  **R324.1 Airport noise study guidelines**. The Aviation Safety and Noise Abatement Act of 1979 14 CFR Part 150 (US Department of Transportation) including revisions through January 2005 and hereby adopted as a guideline for establishing airport noise control. When required by a local government by local ordinance to provide noise attenuation in a new structure or addition to an existing structure near an airport in the area of the local government, the applicant must provide either:  1.  A testing certificate from an accredited noise testing lab that a new structure or addition to existing structure built to the submitted engineering plans will achieve an average minimum dBA reduction equal to or greater than the reduction required,  2.  An engineering judgment signed and sealed by an engineer licensed in the State of Florida that the structure or addition built to the submitted engineering plans will achieve an average minimum dBA reduction equal to or greater than the reduction required, or  3.  Plans using the standards contained in "Guidelines for Sound Insulation of Residences Exposed to Aircraft Operations" prepared for the Department of the Navy by Wyle Research and Consulting, Arlington, Virginia, April 2005 on file with the Florida Building Commission. **[S5441 AS]** |
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***CHAPTER 4, FOUNDATIONS***

***Section R401.1 Application. Revise text to add exception 3 to read as shown:***

**R401.1 Application**. The provisions of this chapter shall control the design and construction of the foundation and foundation spaces for all buildings. In addition to the provisions of this chapter, the design and construction of foundations in flood hazard areas as established by Table R301.2(1) shall meet the provisions of Section R322. Wood foundations shall be designed and installed in accordance with AF&PA PWF.

**Exception:** The provisions of this chapter shall be permitted to be used for wood foundations only in the following situations:

1. – 2. [No change]

3. Buildings and structures located within the High-Velocity Hurricane Zone shall comply with the provisions of Chapter 44 and, as applicable, Section R322 in flood hazard areas.

**[Florida specific]**

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***Section R401.2 Requirements. Revise to read as shown:***

**R401.2 Requirements.** Foundations shall be capable of resisting all loads from roof uplift and building overturn. Foundation uplift for light-frame wood or steel buildings shall be calculated or determined from Table R401.1. Masonry buildings within the dimensional scope of Table R401.1 shall be assumed to be of adequate weight so as not to require uplift resistance greater than that provided by the structure and any normal foundation.   Foundation construction shall also be capable of accommodating all gravity loads according to Section R301 and of transmitting the resulting loads to the supporting soil. Fill soils that support footings and foundations shall be designed, installed and tested in accordance with accepted engineering practice. Gravel fill used as footings for wood and precast concrete foundations shall comply with Section R403.

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**TABLE R401.1**

**FOUNDATION UPLIFT LIGHT STEEL & WOOD FRAME BUILDINGS IN EXPOSURE B (plf)5, 6**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| Roof Angle |  | Bldg Width | Minimum Building Length | **Vasd as determined in accordance with Section R301.2.1.3** / Velocity Pressure | | | | | |
|  | 100 | 110 | 120 | 130 | 140 | 150 |
|  | 15 | 18 | 22 | 26 | 30 | 34 |
| 45 | 3 Stories3 | 20 | 12 | 173 | 276 | 389 | 512 | 644 | 787 |
| 25 | 14 | 73 | 165 | 265 | 374 | 492 | 618 |
| 30 | 19 | 0 | 81 | 174 | 275 | 384 | 502 |
| 35 | 25 | 17 | 48 | 104 | 200 | 305 | 417 |
| 40 | 35 | 33 | 69 | 109 | 152 | 240 | 349 |
| 2 Stories2 | 20 | 12 | 64 | 126 | 195 | 270 | 350 | 437 |
| 25 | 15 | 17 | 66 | 129 | 198 | 272 | 352 |
| 30 | 22 | 38 | 62 | 90 | 146 | 217 | 294 |
| 35 | 35 | 56 | 86 | 118 | 154 | 192 | 252 |
| 40 | 40 | 74 | 108 | 146 | 186 | 230 | 277 |
| 1 Story1 | 20 | 12 | 33 | 46 | 61 | 94 | 132 | 173 |
| 25 | 22 | 57 | 75 | 96 | 118 | 142 | 167 |
| 30 | 18 | 79 | 103 | 129 | 157 | 187 | 219 |
| 35 | 16 | 100 | 128 | 159 | 192 | 229 | 267 |
| 40 | 16 | 120 | 152 | 188 | 226 | 268 | 312 |
| *30* | *3 Stories3* | *20* | *12* | *92* | *177* | *271* | *373* | *483* | *601* |
| 25 | 17 | 0 | 63 | 143 | 230 | 324 | 425 |
| 30 | 25 | 0 | 23 | 52 | 125 | 209 | 300 |
| 35 | 35 | 13 | 44 | 78 | 115 | 154 | 206 |
| 40 | 40 | 28 | 64 | 102 | 145 | 190 | 239 |
| 2 Stories2 | 20 | 13 | 1 | 50 | 103 | 162 | 224 | 292 |
| 25 | 23 | 16 | 35 | 57 | 84 | 139 | 199 |
| 30 | 30 | 36 | 60 | 87 | 116 | 148 | 181 |
| 35 | 35 | 54 | 83 | 115 | 150 | 187 | 227 |
| 40 | 36 | 71 | 104 | 141 | 181 | 224 | 270 |
|  | 20 | 20 | 32 | 46 | 60 | 76 | 93 | 112 |
| 25 | 15 | 56 | 74 | 95 | 117 | 140 | 166 |
| 30 | 13 | 78 | 102 | 127 | 155 | 185 | 217 |
| 35 | 14 | 99 | 127 | 157 | 190 | 226 | 264 |
| 1Story140 | 16 | 118 | 150 | 185 | 223 | 264 | 308 |
| 20 | 12 | 113 | 203 | 301 | 408 | 523 | 647 |
| 25 | 14 | 45 | 130 | 222 | 322 | 431 | 547 |
| 30 | 17 | 4 | 85 | 177 | 277 | 385 | 501 |
| 35 | 19 | 20 | 58 | 154 | 257 | 369 | 489 |
| 3Story140 | 21 | 35 | 72 | 141 | 249 | 367 | 493 |
| 20 | 12 | 43 | 100 | 163 | 231 | 304 | 384 |
| 25 | 13 | 22 | 79 | 143 | 214 | 289 | 371 |
| 30 | 15 | 42 | 72 | 141 | 217 | 298 | 386 |
| 35 | 15 | 61 | 92 | 150 | 232 | 321 | 417 |
| 2Story140 | 16 | 78 | 114 | 164 | 254 | 352 | 457 |
| 20 | 12 | 38 | 57 | 94 | 135 | 179 | 226 |
| 25 | 12 | 62 | 82 | 122 | 171 | 223 | 280 |
| 30 | 12 | 85 | 110 | 154 | 212 | 275 | 342 |
| 35 | 14 | 107 | 136 | 190 | 257 | 330 | 409 |
| 1Story140 | 16 | 126 | 160 | 227 | 304 | 388 | 478 |

**TABLE R401.1**

**FOUNDATION UPLIFT LIGHT STEEL & WOOD FRAME BUILDINGS IN EXPOSURE C (plf)5, 6**

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| Roof Angle | |  | | Bldg Width | | Minimum Building Length4 | | **Vasd as determined in accordance with Section R301.2.1.3** / Velocity Pressure | | | | | | | | | | | |
|  | | 100 | | 110 | | 120 | | 130 | | 140 | | 150 | |
|  | | 21 | | 26 | | 31 | | 36 | | 42 | | 48 | |
| 45 | | 3 Stories3 | | 20 | | 12 | | 370 | | 515 | | 673 | | 845 | | 1031 | | 1231 | |
| 25 | | 13 | | 249 | | 377 | | 518 | | 670 | | 836 | | 1013 | |
| 30 | | 17 | | 159 | | 278 | | 408 | | 550 | | 703 | | 867 | |
| 35 | | 21 | | 89 | | 203 | | 328 | | 463 | | 610 | | 767 | |
| 40 | | 26 | | 102 | | 153 | | 262 | | 394 | | 537 | | 691 | |
| 2 Stories2 | | 20 | | 12 | | 184 | | 271 | | 368 | | 472 | | 585 | | 706 | |
| 25 | | 13 | | 119 | | 200 | | 288 | | 385 | | 489 | | 601 | |
| 30 | | 18 | | 85 | | 147 | | 233 | | 326 | | 426 | | 533 | |
| 35 | | 24 | | 113 | | 155 | | 200 | | 284 | | 383 | | 489 | |
| 40 | | 36 | | 139 | | 187 | | 240 | | 297 | | 358 | | 457 | |
| 1 Story1 | | 20 | | 12 | | 58 | | 95 | | 140 | | 189 | | 243 | | 300 | |
| 25 | | 16 | | 92 | | 118 | | 147 | | 178 | | 224 | | 281 | |
| 30 | | 19 | | 124 | | 157 | | 193 | | 233 | | 275 | | 321 | |
| 35 | | 17 | | 154 | | 193 | | 236 | | 283 | | 334 | | 388 | |
| 40 | | 16 | | 182 | | 227 | | 277 | | 331 | | 389 | | 452 | |
| 30 | | 3 Stories3 | | 20 | | 12 | | 256 | | 376 | | 507 | | 650 | | 804 | | 970 | |
| 25 | | 15 | | 130 | | 232 | | 344 | | 466 | | 598 | | 740 | |
| 30 | | 21 | | 47 | | 127 | | 228 | | 337 | | 455 | | 582 | |
| 35 | | 31 | | 72 | | 116 | | 163 | | 241 | | 351 | | 469 | |
| 40 | | 40 | | 96 | | 146 | | 200 | | 259 | | 323 | | 392 | |
| 2 Stories2 | | 20 | | 12 | | 95 | | 163 | | 238 | | 320 | | 408 | | 502 | |
| 25 | | 18 | | 53 | | 85 | | 151 | | 223 | | 301 | | 385 | |
| 30 | | 30 | | 83 | | 117 | | 154 | | 195 | | 239 | | 304 | |
| 35 | | 35 | | 110 | | 150 | | 195 | | 244 | | 296 | | 353 | |
| 40 | | 37 | | 135 | | 182 | | 233 | | 289 | | 350 | | 415 | |
| 1 Story1 | | 20 | | 20 | | 58 | | 76 | | 97 | | 119 | | 143 | | 169 | |
| 25 | | 16 | | 91 | | 117 | | 145 | | 176 | | 210 | | 245 | |
| 30 | | 13 | | 123 | | 156 | | 191 | | 230 | | 272 | | 317 | |
| 35 | | 14 | | 152 | | 191 | | 234 | | 280 | | 330 | | 384 | |
| 40 | | 16 | | 179 | | 224 | | 273 | | 327 | | 384 | | 446 | |
| 20 | 3 Stories3 | | 20 | | 12 | | 285 | | 411 | | 549 | | 698 | | 860 | | 1034 | |
| 25 | | 13 | | 207 | | 325 | | 455 | | 595 | | 748 | | 911 | |
| 30 | | 15 | | 162 | | 280 | | 409 | | 549 | | 701 | | 863 | |
| 35 | | 17 | | 138 | | 260 | | 393 | | 538 | | 695 | | 863 | |
| 40 | | 18 | | 124 | | 252 | | 392 | | 545 | | 709 | | 886 | |
| 2 Stories2 | | 20 | | 12 | | 152 | | 233 | | 320 | | 416 | | 519 | | 630 | |
| 25 | | 12 | | 133 | | 215 | | 306 | | 404 | | 511 | | 625 | |
| 30 | | 13 | | 130 | | 219 | | 316 | | 422 | | 536 | | 658 | |
| 35 | | 14 | | 138 | | 235 | | 341 | | 456 | | 581 | | 715 | |
| 40 | | 16 | | 150 | | 257 | | 373 | | 500 | | 636 | | 783 | |
| 1 Story1 | | 20 | | 12 | | 88 | | 136 | | 188 | | 245 | | 307 | | 373 | |
| 25 | | 12 | | 114 | | 172 | | 235 | | 303 | | 377 | | 457 | |
| 30 | | 12 | | 146 | | 214 | | 288 | | 370 | | 457 | | 552 | |
| 35 | | 14 | | 180 | | 259 | | 346 | | 441 | | 543 | | 653 | |
| 40 | | 16 | | 215 | | 306 | | 406 | | 515 | | 632 | | 758 | |

Notes:

1. Based on 1st floor height = 10 ft. or 11 ft. floor to floor in multi-story.
2. Based on 2nd floor height = 8 ft. or 9 ft. floor to floor in multi-story.
3. Based on 3rd floor height = 8 ft.
4. Building length shall be equal to or greater than that shown in tables.
5. Roof and floor framing shall span in the same direction.
6. Includes provision for 2 foot roof overhang

**(S5780 AS)**

***Section R403.1 General. Revise to read as shown:***

**R403.1 General.** [No change to text]

**R403.1.1 Minimum size.** Minimum sizes for concrete and masonry footings shall be as set forth in Table R403.1 and Figure R403.1(1). Minimum sizes for concrete and masonry footings shall also be as required to provide adequate resistance to uplift and overturn of the building as determined from Table 401.1 or as calculated using engineered design in accordance with the *Florida Building Code, Building*. The footing width, W, shall be based on the load-bearing value of the soil in accordance with Table R401.4.1. Spread footings shall be at least 8 inches (203 mm) in thickness T. Footing projections, P, shall be at least 2 inches (51 mm) and shall not exceed the thickness of the footing. The size of footings supporting piers and columns shall be based on the tributary load and allowable soil pressure in accordance with Table R401.4.1. Footings for wood foundations shall be in accordance with the details set forth in Section R403.2, and Figures R403.1(2) and R403.1(3).

**R403.1.2** **Resistance to uplift.** Uplift resistance of common foundations are given in Table R403.1.2. Uplift resistance of these foundations may be increased by increasing the size of the concrete footing. When determining the modified uplift resistance the added weight shall be reduced by multiplying by a factor of 0.6. Other foundation systems shall be engineered in accordance with the *Florida Building Code, Building*.

**TABLE R403.1.2**

**FOUNDATION UPLIFT DESIGN DETAILS**

|  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- |
| **FOOTING** | **TYPE** | **T** | **W** | **SLAB/WALL1** | **RESISTANCE** | **NOTES** |
| A | Mono | 20 | 12 | 6 | 502 | 3 |
|  | Mono | 20 | 16 | 6 | 585 | 3 |
| B | Mono Interior | 20 | 12 | 13 | 796 | 3 |
|  | Mono Interior | 20 | 16 | 13 | 879 | 3 |
| C | 12” Stem/Joist | 10 | 20 | 228 | 436 | 1,2,3 |
| D | Mono | 20 | 12 | 6 | 502 | -- |
|  | Mono | 20 | 16 | 6 | 585 | -- |
| E | Mono Interior | 20 | 12 | 13 | 796 | -- |
|  | Mono Interior | 20 | 16 | 13 | 879 | -- |
| F | Stem/Joist | 10 | 20 | -- | 208 | 2,3 |
| G | Stem/Slab | 10 | 20 | 6 | 460 | 3 |
| H | Stem/Slab | 10 | 12 | 6 | 377 | 3 |
|  | Stem/Slab | 10 | 20 | 6 | 460 | 3 |

For SI: 1 inch = 25.4 mm.

1. Tributary width of 3½" slab or weight of stemwall and bond beam

2. 1st floor dead load multiplied by 0.6 may also be included.

3. All footing dowel bars shall be same size as wall steel, shall have a standard 90-degree hook, and shall be embedded a minimum of 6 inches. Dowel bars shall lap vertical wall reinforcement a minimum of 25 inches.

***Delete Figure R403.1(1) and replace with the following:***

|  |
| --- |
|  |

|  |
| --- |
|  |

**FOOTING A MONOLITHIC SLAB-ON-GRADE EXTERIOR WALL**

|  |
| --- |
|  |

**FOOTING B MONOLITHIC SLAB-ON-GRADE INTERIOR WALL**  **FIGURE 403.1(1) CONCRETE AND MASONRY FOUNDATION DETAILS**

|  |
| --- |
|  |

**FOOTING C STEM WALL WOOD JOIST FLOOR**  **FIGURE R403.1(1)—continued**

|  |
| --- |
|  |

**FOOTING D MONOLITHIC EXTERIOR FOOTING**

|  |
| --- |
|  |

**FOOTING E MONOLITHIC INTERIOR FOOTING**  **FIGURE R403.1(1)—continued**

|  |
| --- |
|  |

**FOOTING F WOOD FLOOR TO CONCRETE OR MASONRY STEMWALL**  **FIGURE R403.1(1)—continued**

|  |
| --- |
|  |

**FOOTING G STEMWALL FOUNDATION WITH SLAB ON GRADE**  **FIGURE R403.1(1)—continued**

|  |
| --- |
|  |

**FOOTING H   
  
FIGURE R403.1(1)—continued**

**[S5780 AS]**

***Section R403.1.6 Foundation anchorage. Change to read as follows:***

**R403.1.6 Foundation anchorage.** (no change).

**Exceptions:**

1. [No change]

2. Walls 24 inches (610 mm) total length or shorter connecting offset *braced wall panels* shall be anchored to the foundation with a minimum of one anchor bolt located in the center third of the plate section and shall be attached to adjacent *braced wall panels* at corners as required in Section R602 ~~shown in item 8 of Table R602.3(1)~~.

3. Connection of walls 12 inches (305 mm) total length or shorter connecting offset *braced wall panels* to the foundation without anchor bolts shall be permitted. The wall shall be attached to adjacent *braced wall panels* at corners as required in Section R602 ~~shown in item 8 of Table R602.3(1)~~.

**(S6122GlitchAS)**

***Section R404.2.6 Fastening. Change to read as follows:***

**R404.2.6 Fastening.** Wood structural panel foundation wall sheathing shall be attached to framing in accordance with Section R602 ~~Table R602.3(1)~~ and Section R402.1.1.

**(S6123GlitchAS)**

|  |
| --- |
| ***Section R404.3 Wood sill plates. Change to read as follows:***  **R404.3 Wood sill plates.** Wood sill plates shall be a minimum of 2-inch by 4-inch (51 mm by 102 mm) nominal lumber. Sill plate anchorage shall be in accordance with Sections R403.1.6 and R602 ~~R602.11~~. |
| **(S6124GlitchAS)** |

***Section R404.5.1 Design. Change to read as follows:***

**R404.5.1 Design.** Precast concrete foundation walls shall be designed in accordance with accepted engineering practice. The design and manufacture of precast concrete foundation wall panels shall comply with the materials requirements of Section R402.3 or ACI 318. The panel design drawings shall be prepared by a registered design professional where required by the statutes of the *jurisdiction* in which the project is to be constructed in accordance with Section ~~R106.1~~ 107 of the *Florida Building Code, Building*.

**(S6125GlitchAS)**

***Section R404.5.2 Precast concrete foundation design drawings. Change to read as follows:***

**R404.5.2 Precast concrete foundation design drawings.** (no change)

1-6. (No change)

7. ~~Basic~~Ultimate design wind speed, Vult , from Figure R301.2(4).

**(S6126GlitchAS)**

***Section R408.3Unvented crawl space. Change to read as follows:***

**R408.3 Unvented crawl space.** Ventilation openings in under-floor spaces specified in Sections R408.1and R408.2 shall not be required where:

1. [No change]
2. One of the following is provided for the under-floor space:
   1. Continuously operated mechanical exhaust ventilation at a rate equal to 1 cubic foot per minute (0.47 L/s) for each 50 square feet (4.7 m2) of crawl space floor area, including an air pathway to the common area (such as a duct or transfer grille), and perimeter walls insulated in accordance with Section *R403.2.1 of the Florida Building Code, Energy Conservation* ~~N1103.2.1 of this code~~;
   2. -2.3 [No change]

**(Code correlation)**

***CHAPTER 5, FLOORS***

***R501.1 Application. Change to read as follows:***

**R501.1 Application.** The provisions of this chapter shall control the design and construction of the floors for all buildings including the floors of attic spaces used to house mechanical and/or plumbing fixtures and equipment.

**Exception:** Buildings and structures located within the High-Velocity Hurricane Zone shall comply with the provisions of Chapter 44.

***Section 502.1. Add the following sections R502.1.8 through R502.1.11 as shown using IRC sections identified in brackets:***

**R502.1.8 [IRC502.12] Draftstopping required.**Draftstopping shall be provided in accordance with Section R302.12.

**R502.1.9 [IRC502.13] Fireblocking required.** Fireblocking shall be provided in accordance with Section R302.11.

**R502.1.10 [IRC502.11] Wood trusses**

**R502.1.10.1 [IRC502.11.1] Design.** Wood trusses shall be designed in accordance with approved engineering practice. The design and manufacture of metal plate connected wood trusses shall comply with ANSI/TPI 1. The truss design drawings shall be prepared by a registered professional where required by *Florida Statutes*. ~~the statutes of the jurisdiction in which the project is to be constructed in accordance with Section R106.1.~~

**R502.1.10.2 [IRC502.11.2] Bracing.** Trusses shall be braced to prevent rotation and provide lateral stability in accordance with the requirements specified in the construction documents for the building and on the individual truss design drawings. In the absence of specific bracing requirements, trusses shall be braced in accordance with TPI/WTCA BCSI ~~accepted industry practices , such as the~~ *~~Building Component Safety Information (BCSI) Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses~~.*

**R502.1.10.3 [IRC502.11.3] Alterations to trusses.** [No change to text].

**R502.1.10.4 [IRC502.11.4] Truss design drawings.** Truss design drawings, prepared in compliance with Section R502.1.3.1, shall be provided to the building official and approved prior to installation. Truss design drawing shall be provided with the shipment of trusses delivered to the job site. Truss design drawings shall include, at a minimum, the information specified below: [No change to remainder of text]

**R502.1.11 [IRC502.8] Cutting, drilling and notching.** Structural floor members shall not be cut, bored or notched in excess of the limitations specified in this section. See Figure R502.1.11 ~~R502.8.~~

**FIGURE R502.1.11 [IRC502.8]**

**CUTTING, NOTCHING and DRILLING.**

[No other change to IRC Figure R502.8]

**R502.1.11.1  [IRC502.8.1] Sawn lumber.**  [No change to IRC R502.8.1 text]

**R502.1.11.2  [IRC502.8.2] Engineered wood products.** [No change to IRC R502.8.2 text]

***Section R502.2 Design and construction. Revise to read as shown:***

**R502.2 Design and construction.** Floor framing of light-frame wood construction shall be designed and constructed in accordance with the provisions of Section R301.2.1.1 or in accordance with the AF&PA NDS. Floor framing of light-frame wood construction shall also comply with Sections R317, R318 ~~R319,  R320~~, and R502.1. ~~Floors shall be designed and constructed in accordance with the provisions of this Chapter, Figure R502.2 and Sections R317 and R318 or in accordance with AF&PA/ NDS.~~

[The remainder of Section R502.2 is to be deleted and "Reserved."]

**[S5984 AS] (S6127GlitchAS)**

***Section R505 Steel Floor Framing. Reserve as shown*:**

**SECTION R505**

**STEEL FLOOR FRAMING**

**RESERVED**

**[S6000 AS]**

***CHAPTER 6, WALL CONSTRUCTION***

***Section R601.1 Application. Revise to read as shown:***

**R601.1 Application.** The provisions of this chapter shall control the design and construction of all walls and partitions for all buildings.

**Exception:** Buildings and structures located within the High-Velocity Hurricane Zone shall comply with the provisions of Chapter 44.

**[S4972 AS]**

***Section 602.2. Revise Section R602.2 and add the following sections R602.2.2.1 through R602.2.3.1 as shown using IRC sections identified in brackets:***

**R602.2 General requirements ~~Grade~~.** ~~Studs shall be a minimum No. 3, standard or stud grade lumber. Exception: Bearing studs not supporting floors and nonbearing studs may be utility grade lumber, provided the studs are spaced in accordance with Table R602.3(5).~~

**R602.2.1 [IRC602.8] Fireblocking required.** Fireblocking shall be provided in accordance with Section R302.11.

**R602.2.2 [IRC602.7.3] Nonbearing walls.** Load-bearing headers are not required in interior or exterior nonbearing walls. A single flat 2-inch-by-4-inch (51 mm by 102 mm) member may be used as a header in interior or exterior nonbearing walls for openings up to 8 feet (2438 mm) in width if the vertical distance to the parallel nailing surface above is not more than 24 inches (610 mm). For such nonbearing headers, no cripples or blocking are required above the header.

**R602.2.2.1 [IRC602.5] Interior nonbearing walls.** Interior nonbearing walls shall be permitted to be constructed with 2-inch-by-3-inch (51 mm by 76 mm) studs spaced 24 inches (610 mm) on center or, when not part of a *braced wall line*, 2-inch-by-4-inch (51 mm by 102 mm) flat studs spaced at 16 inches (406 mm) on center. Interior nonbearing walls shall be capped with at least a single top plate. Interior nonbearing walls shall be fireblocked in accordance with Section R302.11 ~~R602.8~~.

**R602.2.3 [IRC602.6] Drilling and notching--studs.**Drilling and notching of studs shall be in accordance with the following:

1. Notching. Any stud in an exterior wall or bearing partition may be cut or notched to a depth not exceeding 25 percent of its width. Studs in nonbearing partitions may be notched to a depth not to exceed 40 percent of a single stud width.

2. Drilling. Any stud may be bored or drilled, provided that the diameter of the resulting hole is no more than 60 percent of the stud width, the edge of the hole is no more than 5/8 inch (16 mm) to the edge of the stud, and the hole is not located in the same section as a cut or notch. Studs located in exterior walls or bearing partitions drilled over 40 percent and up to 60 percent shall also be doubled with no more than two successive doubled studs bored. See Figures R602.2.3(1) and R602.2.3(2) [IRC Figures R602.6(1) and R602.6(2)].

**Exception:** Use of approved stud shoes is permitted when they are installed in accordance with the manufacturer’s recommendations.

**R602.2.3.1 [IRC602.6.1] Drilling and notching of top plate.** When piping or ductwork is placed in or partly in an exterior wall or interior load-bearing wall, necessitating cutting, drilling or notching of the top plate by more than 50 percent of its width, a galvanized metal tie not less than 0.054 inch thick (1.37 mm) (16 ga) and 11/2 inches (38 mm) wide shall be fastened across and to the plate at each side of the opening with not less than eight 10d (0.148 inch diameter) having a minimum length of 1½ inches (38 mm) at each side or equivalent. The metal tie must extend a minimum of 6 inches past the opening. See Figure R602.2.3.1 [IRC Figure R602.6.1].

**Exception:** When the entire side of the wall with the notch or cut is covered by wood structural panel sheathing.

***Section R602.3 Design and construction. Revise to read as shown:***

**R602.3 Design and construction.** Exterior walls of light-frame wood construction shall be designed and constructed in accordance with the provisions of Section R301.2.1.1 or in accordance with the AF&PA NDS. Exterior walls of light-frame wood construction shall also comply with Section R602.1. and R602.2. ~~this chapter and Figures R602.3(1) and R602.3(2) or in accordance with AF&PA’s NDS. Components of exterior walls shall be fastened in accordance~~**~~……..~~**~~purposes shall comply with Section R703.~~

***The remainder of Section R602.3 and sections R602.4 through R602.12 are to be deleted and "Reserved."***

**[S5986 AS]**

***Section R603 STEEL WALL FRAMING. Delete text and reserve as shown:***

**SECTION R603**

**STEEL WALL FRAMING**

**RESERVED**

**[S6001 AS]**

**SECTION R606**

**GENERAL MASONRY CONSTRUCTION**

***Section R606.2 Thickness of masonry. Change to read as shown:***

**R606.2 Thickness of masonry.** The minimum nominal thickness of exterior concrete masonry walls shall be 8 inches or shall be designed in accordance with Section R606.1. ~~The nominal thickness of ma­sonry walls shall conform to the requirements of Sections R606.2.1 through R606.2.4.~~

**R606.2.1 Minimum thickness.** Reserved. ~~The minimum thickness of masonry bearing walls more than one story high shall be 8 inches (203 mm). Solid masonry walls of one-story dwell­ings and garages shall not be less than 6 inches (152 mm) in thickness when not greater than 9 feet (2743 mm) in height, provided that when gable construction is used, an additional 6 feet (1829mm) is permitted to the peak of the gable. Masonry walls shall be laterally supported in either the horizontal or vertical direction at intervals as required by Section R606.8.~~

**R606.2.2 Rubble stone masonry wall**. Reserved. ~~The minimum thickness of rough, random or coursed rubble stone ma­sonry walls shall be 16 inches (406 mm).~~

**R606.2.3 Change in thickness.** [No change to IRC text].

**R606.2.4 Parapet walls.** Unreinforced solid masonry para­pet walls shall not be less than 8 inches (203 mm) in thick­ness and their height shall not exceed four times their thickness. Unreinforced hollow unit masonry parapet walls shall be not less than 8 inches (203 mm) in thickness, and their height shall not exceed three times their thickness. Ma­sonry parapets in areas subject to wind loads ~~of 30 pounds per square foot (1.44 kN/m~~~~2~~~~)~~ shall be reinforced in accor­dance with ACI 530/ASCE 5/TMS 402. Masonry parapets, located in Seismic Design Category D0, D1 or D2, or on townhouses in Seismic Design Category C shall be reinforced in accordance with Section R606.12.

**(S6017 AS)**

***Section R606.5 Allowable stresses. Change to read as follows:***

**R606.5 Allowable stresses** ~~Allowable compressive stresses in masonry shall not exceed the values prescribed in Table R606.5.~~ Concrete masonry units shall be hollow or solid unit masonry in accordance with ASTM C 90 and shall have a minimum net area compressive strength of 1900 psi in compliance with ASTM C 90. Mortar shall comply with Section R607.1. In determining the stresses in masonry, the effects of all loads and conditions of loading and the influence of all forces affecting the design and strength of the several parts shall be taken into account.

**R606.5.1 Combined units.** [no change is proposed to IRC text]

**TABLE R606.5  
ALLOWABLE COMPRESSIVE STRESSES FOR  
EMPIRICAL DESIGN OF MASONRY**

Reserved

|  |  |  |
| --- | --- | --- |
|  | ~~ALLOWABLE~~  ~~COMPRESSIVE STRESSES~~~~a~~  ~~GROSS CROSS-SECTIONAL~~  ~~AREA~~~~b~~ | |
| ~~CONSTRUCTION; COMPRESSIVE~~  ~~STRENGTH OF UNIT, GROSS AREA~~ | ~~Type M or S~~  ~~Mortar~~ | ~~Type N mortar~~ |
| ~~Solid masonry of brick and other solid units~~  ~~of clay or shale; sand-lime or concrete brick:~~ |  |  |
| ~~8,000 + psi~~ | ~~350~~ | ~~300~~ |
| ~~4,500 psi~~ | ~~225~~ | ~~200~~ |
| ~~2,500 psi~~ | ~~160~~ | ~~140~~ |
| ~~1,500 psi~~ | ~~115~~ | ~~100~~ |
| ~~Grouted~~~~c~~ ~~masonry, of clay or shale;~~  ~~sand-lime or concrete:~~  ~~4,500+ psi~~ | ~~225~~ | ~~200~~ |
| ~~2,500 psi~~ | ~~160~~ | ~~140~~ |
| ~~1,500 psi~~ | ~~115~~ | ~~100~~ |
| ~~Solid masonry of solid concrete masonry~~  ~~units:~~  ~~3,000+ psi~~ | ~~225~~ | ~~200~~ |
| ~~2,000 psi~~ | ~~160~~ | ~~140~~ |
| ~~1,200 psi~~ | ~~115~~ | ~~100~~ |
| ~~Masonry of hollow load-bearing units:~~  ~~2,000+ psi~~ | ~~140~~ | ~~120~~ |
| ~~1,500 psi~~ | ~~115~~ | ~~100~~ |
| ~~1,000 psi~~ | ~~75~~ | ~~70~~ |
| ~~700 psi~~ | ~~60~~ | ~~55~~ |
| ~~Hollow walls (cavity or masonry bonded~~~~d~~~~)~~  ~~solid units:~~  ~~2,500+ psi~~ | ~~160~~ | ~~140~~ |
| ~~1,500 psi~~ | ~~115~~ | ~~100~~ |
| ~~Hollow units~~ | ~~75~~ | ~~70~~ |
| ~~Stone ashlar masonry:~~  ~~Granite~~ | ~~720~~ | ~~640~~ |
| ~~Limestone or marble~~ | ~~450~~ | ~~400~~ |
| ~~Sandstone or cast stone~~ | ~~360~~ | ~~320~~ |
| ~~Rubble stone masonry:~~  ~~Coarse, rough or random~~ | ~~120~~ | ~~100~~ |

~~For SI: 1 pound per square inch = 6.895 kPa.~~

1. ~~Linear interpolation shall be used for determining allowable stresses for ma­sonry units having compressive strengths that are intermediate between those given in the table.~~
2. ~~Gross cross-sectional area shall be calculated on the actual rather than nomi­nal dimensions.~~
3. ~~See Section R607.~~
4. ~~Where floor and roof loads are carried upon one wythe, the gross cross-sec­tional area is that of the wythe under load; if both wythes are loaded, the gross cross-sectional area is that of the wall minus the area of the cavity be­tween the wythes. Walls bonded with metal ties shall be considered as cavity walls unless the collar joints are filled with mortar or grout.~~

***Section R606.6 Piers. Change to read as follows:***

**R606.6 Piers.** Reserved. ~~The unsupported height of masonry piers shall not exceed ten times their least dimension. When structural clay tile or hollow concrete masonry units are used for isolated piers to support beams and girders, the cellular spaces shall be filled solidly with concrete or Type M or S mortar, except that unfilled hollow piers may be used if their unsupported height is not more than four times their least dimension. Where hollow masonry units are solidly filled with concrete or Type M, S or N mortar, the allowable compressive stress shall be permitted to be increased as provided in Table R606.4.~~

**R606.6.1 Pier cap.** Hollow piers shall be capped with 4 inches (102 mm) of solid masonry or concrete, a masonry cap block, or shall have cavities of the top course filled with concrete or grout or other approved methods.

***Section R606.8 Stack bond. Change to read as follows:***

**R606.8 ~~Stack~~ ~~b~~ Bond.** Masonry walls shall be running bond or stack bond construction. ~~In unreinforced masonry where masonry units are laid in stack bond, longitudinal reinforcement consist­ing of not less than two continuous wires each with a minimum aggregate cross-sectional area of 0.017 square inch (11 mm~~~~2~~~~) shall be provided in horizontal bed joints spaced not more than 16 inches (406 mm) on center vertically.~~

**R606.8.1 Joint reinforcement stack bond.** When masonry units are laid in stack bond, horizontal joint reinforcement shall be placed in bed joints at not more than 16 inches on center. Horizontal joint reinforcement shall be a minimum of 9-gage and shall be in addition to required vertical reinforcement, Joint reinforcement shall be embedded in accordance with R606.13.6.

***Section R606.9 Lateral support. Change to read as follows:***

**R606.9 Lateral support**. Reserved.  ~~Masonry walls shall be laterally supported in either the horizontal or the vertical direction. The maximum spacing between lateral supports shall not exceed the distances in Table R606.9. Lateral support shall be provided by cross walls, pilasters, buttresses or structural frame mem­bers when the limiting distance is taken horizontally, or by floors or roofs when the limiting distance is taken vertically.~~

**R606.9.1 Horizontal lateral support.** Reserved. ~~Lateral support in the horizontal direction provided by intersecting masonry walls shall be provided by one of the methods in Section R606.9.1.1 or Section R606.9.1.2.~~

**R606.9.1.1 Bonding pattern.** Reserved. ~~Fifty percent of the units at the intersection shall be laid in an overlapping ma­sonry bonding pattern, with alternate units having a bear­ing of not less than 3 inches (76mm) on the unit below.~~

**R606.9.1.2 Metal reinforcement.** Reserved. ~~Interior nonload-bearing walls shall be anchored at their intersec­tions, at vertical intervals of not more than 16 inches (406 mm) with joint reinforcement of at least 9 gage, or 1/4 inch (6.4 mm) galvanized mesh hardware cloth. Intersecting masonry walls, other than interior nonload-bearing walls, shall be anchored at vertical intervals of not more than 8 inches (203 mm) with joint reinforcement of at least 9 gage and shall extend at least 30 inches (762mm) in each direction at the intersection. Other metal ties, joint reinforcement or anchors, if used, shall be spaced to provide equivalent area of anchorage to that required by this section.~~

**R606.9.2 Vertical lateral support.** Reserved.  ~~Vertical lateral support of masonry walls shall be provided in accordance with one of the methods in Section R606.9.2. 1 or Section R606.9.2.2.~~

**R606.9.2.1 Roof structures.** Reserved. ~~Masonry walls shall be anchored to roof structures with metal strap anchors spaced in accordance with the manufacturer’s instructions, ½ inch (13 mm) bolts spaced not more than 6 feet (1829 mm) on center, or other approved anchors. Anchors shall be embedded at least 16 inches (406 mm) into the masonry, or b3e hooked or welded to bond beam reinforcement placed not less than 6 inches (152 mm) from the top of the wall..~~

**R606.9.2.2 Floor diaphragms.** Reserved. ~~Masonry walls shall be anchored to floor diaphragm framing by metal strap anchors spaced in accordance with the manufacturer’s instructions, ½ inch diameter (13 mm) bolts spaced at intervals not to exceed 6 feet (1829 mm) and installed as shown in Figure R606.11(1), or by other approved methods.~~

***Table R606.9 Spacing of Lateral Support for Masonry Walls. Delete table to read as follows:***

**TABLE R606.9**

**SPACING OF LATERAL SUPPORT FOR MASONRY WALLS**

Reserved

|  |  |
| --- | --- |
| **~~CONSTRUCTION~~** | **~~MAXIMUM WALL LENGTH TO THICKNESS~~**  **~~OR WALL HEIGHT TO THICKNESS~~~~a,b~~** |
| ~~Bearing walls:~~  ~~Solid or solid grouted~~  ~~All other~~ | ~~20~~  ~~18~~ |
| ~~Nonbearing walls:~~  ~~Exterior~~  ~~Interior~~ | ~~18~~  ~~36~~ |

~~For SI: 1 foot = 304.8 mm.~~

1. ~~Except for cavity walls and cantilevered walls, the thickness of a wall shall be its nominal thickness measured perpendicular to the face of the wall. For cavity walls, the thickness shall be determined as the sum of the nominal thicknesses of the individual wythes. For cantilever walls, except for para­pets, the ratio of height to nominal thickness shall not exceed 6 for solid ma­sonry, or 4 for hollow masonry. For parapets, see Section R606.2.4.~~
2. ~~An additional unsupported height of6feet is permitted for gable end walls.~~

***Section R606.10 Lintels. Change to read as follows:***

**R606.10 Lintels.** Reserved**.** ~~Masonry over openings shall be supported by steel lintels, reinforced concrete or masonry lintels or masonry arches, designed to support load imposed~~.

***Section R606.11 Anchorage. Change to read as follows:***

**R606.11 Anchorage.** Reserved. ~~Masonry walls shall be anchored to floor and roof systems in accordance with the details shown in Fig­ure R606. 11(1), R606.11(2), or R606.11(3). Footings may be considered as points of lateral support.~~

**Figure R606.11(1)**

**Anchorage Requirements for Masonry Walls Located in Seismic Design Category A, B or C and Where Wind Loads are Less than 30 psf.**

Reserved.

(S6017 AS)

***Section R606.12.1.1 Floor and roof diaphragm construction. Change to read as follows:***

**R606.12.1.1 Floor and roof diaphragm construction.** Floor and roof diaphragms shall be constructed of wood structural panels attached to wood framing in accordance with Section R602 ~~Table R602.3(1)~~ or to cold-formed steel floor framing in accordance with AISI S230 ~~Table R505.3(1)~~ or to cold-formed steel roof framing in accordance with AISI S230 ~~Table R804.3~~.

[No change to remainder of section]

**(S6128 Glitch Stafford)**

***Section R606.13 Protection for reinforcement. Completely change to read as follows:***

**R606.13 ~~Protection for r~~Reinforcement.** Reinforcing steel shall be a minimum of Grade 60 No. 5 or No. 4 bars and shall be identified in an approved manner. ~~Bars shall be com­pletely embedded in mortar or grout. Joint reinforcement embedded in horizontal mortar joints shall not have less than~~ ~~5~~~~/~~~~8~~~~-inch (15.9 mm) mortar coverage from the exposed face. All other reinforcement shall have a minimum coverage of one bar diameter over all bars, but not less than ¾ inch (19.1 mm), ex­cept where exposed to weather or soil, in which case the mini-mum coverage shall be 2 inches (51 mm).~~

**R606.13.1 Bundling.** Bundling shall be permitted when two bars are required at the same location in a wall or in a bond beam.

**R606.13.2 Splicing.** Splices shall be lap splices. Non-contact lap splices shall be permitted provided reinforcing bars are not spaced farther apart than 5 inches. Splice lengths shall be in accordance with Table R606.13.2. and shall be a minimum of 25 inches for No. 5 bars and 20 inches for No. 4 bars.

|  |  |
| --- | --- |
| **TABLE 606.13.2** | |
| **LAP SPLICE LENGTHS** | |
| **Bar Size (No.)** | **Lap Length (in.)** |
| 3 | 15 |
| 4 | 20 |
| 5 | 25 |
| 6 | 42 |
| 7 | 59 |

**R606.13.3 Bending.** Reinforcement shall be bent in the shop or in the field. All reinforcement shall be bent cold. The diameter of the bend, measured on the inside of the bar, shall not be less than six-bar diameters. Reinforcement partially embedded in concrete shall not be field bent.

**Exception:** Where bending is necessary to align dowel bars with a vertical cell, bars partially embedded in concrete shall be permitted to be bent at a slope of not more than 1 inch of horizontal displacement to 6 inches of vertical bar length.

**R606.13.4 Clearance from masonry.** Reinforcing bars embedded in grouted masonry cells shall have a minimum clear distance between reinforcing bars and any face of a cell of ¼-inch for fine grout or ½-inch for coarse grout.

**R606.13.5 Cover for reinforcing steel.** Reinforcing bars used in masonry walls shall have a masonry cover, including grout, of not less than 2 inches for masonry units with face exposed to earth or weather and 1½-inch for masonry units not exposed to earth or weather.

**R606.13.6 Joint reinforcement embedment.** Longitudinal wires of joint reinforcement shall be fully embedded in mortar or grout with a minimum cover of ⅝- inch when exposed to earth or weather and ½-inch when not exposed to earth or weather.

**R606.13.7 Cleanout openings.** Cleanout openings shall be provided for cells containing spliced reinforcement when the grout pour exceeds 5 feet in height. Where cleanout openings are required, an opening shall be provided in the bottom course of the masonry cell to be filled. Cleanout openings shall have a minimum opening dimension of 3 inches.

**R606.13.8 Termination.** All vertical wall reinforcement shall be terminated by hooking into a bond beam or footing with a standard hook. Standard hooks shall be formed by bending the vertical wall reinforcement in accordance with Section R606.13.3 or shall be a prefabricated standard hook. Splices to standard hooks shall be lap splices with the minimum extension length beyond the bend for standard hooks of 10 inches for No. 5 bars and 8 inches for No. 4 bars. Hooks at bond beams shall extend to the uppermost horizontal reinforcement of the bond beam and shall be embedded a minimum of 6 inches into the bond beam as detailed in Figure R606.13A and Figure R606.13B. Where multiple bars are required, a single standard hook shall terminate into the bond beam or footing. In narrow footings where the width is insufficient to accommodate a standard 90-degree hook and provide the concrete cover required by Table 1907.7.1 of the *Florida Building Code, Building*, the hook shall be rotated in the horizontal direction until the required concrete cover is achieved.

**R606.13.9 Continuity multi-story construction.** Vertical wall reinforcement in multi-story construction shall extend through bond beams and shall be continuous with the vertical wall reinforcement of the wall above or be offset in accordance with Section R606.13.1 and Figure R606.13.9B.

**Exception:** Where more than one bar in the same cell is required for vertical wall reinforcement, only one bar shall be required to be continuous between stories.

**R606.13.9.1 Offset reinforcement.** Vertical reinforcement shall be permitted to be offset between floor levels. Reinforcement for the lower story shall be anchored into the upper floor level bond beam and reinforcement for the upper story shall be anchored into the bond beams above and below in accordance with Section R606.13.8 and Figures R606.13.9A and R606.13.9B.

**(S6188GlitchAS)(S6129GlitchAS--Superseded by S6188)**

***Section R606.14.1 Joist bearing. Change to read as shown:***

**R606.14.1 Joist bearing.** Except where supported on a 1 inch by 4inch (25.4 mm by 102 mm) ribbon strip and nailed to the adjoining stud and as provided in Section R606.11, the ends of each joist shall not have less than 1 ½ inches (33 mm) of bearing on wood or metal, or less than 3 inches (76 mm) on masonry. ~~Joists shall have a bearing of not less than 1½ inches (38 mm),~~

~~except as provided in Section R606.14, and shall be supported in accordance with Figure R606.11(1).~~

Termination per 606.10.8



**FIGURE R606.13.9A**

**CONTINUITY OF REINFORCEMENT**

**ONE STORY MASONRY WALL**



**FIGURE R606.13.9B**

**CONTINUITY OF FIRST AND SECOND FLOOR**

**VERTICAL WALL REINFORCEMENT**

***(S6188GlitchBelcher)***

**[S6017 AS]**

***Section R606.16. Add a new section to read as shown:***

**R606.16 Masonry Opening Tolerances.**  Masonry rough openings may vary in the cross section dimension or elevation dimension specified on the approved plans from - 1/4 inches (6.4 mm) to + 1/2 inches (12.7 mm). For exterior window and door installation provisions see Sections R612.11 and R612.12.

**[S5950 AS]**

***Section R607, Unit Masonry. Change to read as shown:***

**SECTION R607**

**UNIT MASONRY**

**R607.1 Mortar.** Mortar for use in masonry construction shall be either Type M or S with a f’m of 1500- psi in accordance with ~~comply with~~ ASTM C 270~~. The type of mortar shall be in ac­cordance with Sections R607. 1.1, and R607. 1.2~~ and shall meet the proportion specifications of Table R607.1 or the property specifications of ASTM C 270.

**R607.1.1 – R607.1.3** [No change to text]

***Table R607.1 Mortar Proportions. Delete type N and O mortar in the table to read as shown:***

**TABLE R607.1  
MORTAR PROPORTIONSa, b**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
|  |  |  | **PROPORTIONS BY VOLUME (cementitious materials)** | | | | | | | |
| **Mortar** | **Type** | **Portland cement or blended cement** | **Mortar cement** | | | **Masonry cement** | | | **Hydrated limec or lime putty** | **Aggregate ratio**  **(measured in damp, loose conditions)** |
| **M** | **S** | **~~N~~** | **M** | **S** | **~~N~~** |
| Cement-lime | M  S  ~~N~~  ~~O~~ | 1  1  ~~1~~  ~~1~~ | --  --  ~~--~~  -- | --  --  --  -- | ~~--~~  ~~--~~  ~~--~~  ~~--~~ | --  --  --  -- | --  --  --  -- | ~~--~~  ~~--~~  ~~--~~  ~~--~~ | ¼  over ¼ to ½  over ½ to 1 ¼  over 1 ¼ to 2 ½ | Not less than 2 ¼ and not more than 3 times the sum of separate volumes of lime, if used, and cement |
| Mortar cement | M  M  S  S  ~~N~~  ~~O~~ | 1  --  ½  --  --  -- | --  1  --  --  --  -- | --  --  --  1  --  -- | ~~1~~  ~~--~~  ~~1~~  ~~--~~  ~~1~~  ~~1~~ | --  --  --  --  --  -- | --  --  --  --  --  -- | ~~--~~  ~~--~~  ~~--~~  ~~--~~  ~~--~~  ~~--~~ | --- |
| Masonry cement | M  M  S  S  ~~N~~  ~~O~~ | 1  --  ½  --  --  -- |  |  |  | --  1  --  --  --  -- | --  --  --  1  --  -- | ~~1~~  ~~--~~  ~~1~~  ~~--~~  ~~1~~  ~~1~~ | --- |

For SI: 1 cubic foot = 0.0283 m3, 1 pound = 0.454 kg.

1. For the purpose of these specifications, the weight of 1 cubic foot of the respective materials shall be considered to be as follows:

Portland Cement 94 pounds Masonry Cement Weight printed on bag

Mortar Cement Weight printed on bag Hydrated Lime 40 pounds

Lime Putty (Quicklime) 80 pounds Sand, damp and loose 80 pounds of dry sand

b. Two air-entraining materials shall not be combined in mortar.

c. Hydrated lime conforming to the requirements of ASTM C 207.

**(S6186GlitchAS)(S6017AS)**

**SECTION R609**

**GROUTED MASONRY**

***Section R609.1.2 Grouting requirements. Change to read as follows:***

**R609.1.2 Grout lift height**. ~~Grouting requirements. Maximum pour heights and the minimum dimensions of spaces provided for grout placement shall conform to Table R609. 1.2.~~ Where the following conditions are met, place grout in lifts not exceeding 12.67 ft (3.86 m).

1. The masonry has cured for at least 4 hours.

2. The grout slump is maintained between 10 and 11 in. (254 and 279 mm).

3. No intermediate reinforced bond beams are placed between the top and the bottom of the pour height.

Otherwise, place grout in lifts not exceeding 5 ft (1.52 m). If ~~the work~~ grouting is stopped for one hour or longer, the horizontal construction joints shall be formed by stopping all tiers at the same elevation and with the grout 1 inch (25.4 mm) below the top.

***Table R609.1.2 Grout Space Dimensions and Pour Heights. Delete table to read as shown:***

**TABLE R609.1.2**

**GROUT SPACE DIMENSIONS AND POUR HEIGHTS**

**Reserved**

**R609.1.4 Grout placement.** All cells containing reinforcement or anchor bolts shall be grouted solid. Grout shall be a plastic mix suitable for pumping without segregation of the constituents and shall be mixed thoroughly. Grout shall have a maximum coarse aggregate size of ⅜-inch, shall be placed at an 8 to 11-inch slump, and shall have a minimum specified compressive strength of 2000 psi at 28 days in accordance with ASTM C 476, or when tested in an approved manner. Grout shall be placed by pumping or by an approved alternate method and shall be placed before any initial set occurs and in no case more than 1½ hours after water has been added. Grouting shall be done in a continuous pour, in lifts ~~not exceeding 5 feet (1524 mm). It shall be consolidated by puddling or mechanical vibrating during placing and reconsolidated after excess moisture has been absorbed but before plasticity is lost.~~ in accordance with Section 609.1.2. Grout shall be consolidated at the time of placement in accordance with the following:

1. Consolidate grout pours 12 in. (305 mm) or less in height by mechanical vibration or by puddling.

2. Consolidate pours exceeding 12 in. (305 mm) in height by mechanical vibration, and reconsolidate by mechanical vibration after initial water loss and settlement has occurred.

**R609.1.5 Cleanouts.** ~~Where required by the building official, cleanouts shall be provided as specified in this section.~~ Cleanouts shall be provided at the bottom course at each pour of grout where such pour exceeds 5 feet (1524 mm) in height and where required by the building official. Cleanouts shall be provided with an opening of sufficient size to permit removal of debris. The minimum opening dimension shall be 3 in. (76.2 mm). The cleanouts shall be sealed before grouting and after inspection.

**R609.1.5.1 Grouted multiple-wythe masonry.** [No change proposed]

**R609.1.5.2 Grouted hollow unit masonry.** Reserved. ~~Cleanouts shall be provided at the bottom course of each cell to be grouted at each pour of grout, where such pour exceeds 4 5 feet (1219 mm) in height.~~

***DELETE SECTIONS R609.2, 609.3, AND R609.4 IN THEIR ENTIRETY AND ADD NEW SECTIONS R609.2 THROUGH R609.6 AS FOLLOWS:***

**R609.2 Bond beams.** A reinforced bond beam shall be provided in masonry walls at the top of the wall and at each floor level of each exterior wall. Masonry walls not extending to the roof line shall have a bond beam at the top of the wall.

**Exceptions:**

1. A bond beam is not required at the floor level for slab-on-ground floors.

2. Gable endwalls shall be in conformance with Section R609.4.

**R609.2.1 Bond beam types.** Bond beams shall be one of the following:

1. 8" thick x 8" high masonry.

8" thick x 12" high masonry.

8" thick x 16" high masonry.

8” thick x 24” high masonry.

8” thick x 32” high masonry.

2. Precast units certified by the manufacturer for the uplift loads as set forth in Table R802.11. Precast units shall be installed in accordance with the manufacturer’s specifications, and approved by the building official.

**R609.2.2 Bond beam reinforcement.** The minimum reinforcement for bond beam roof diaphragm chord tension reinforcement steel shall be as set forth in Table R609.2.2A1 through Table R609.2.2A-4 for the appropriate grade of steel and exposure category. The minimum reinforcement for bond beam uplift resisting reinforcement steel shall be as set forth in Tables R609.2.2B-1 through R609.2.2B-8 for the loads set forth in Table R802.11. The total minimum area of bond beam reinforcement shall be the sum of the required area of the diaphragm chord tension steel and the required area of bond beam uplift steel. Bond beam area shall be converted to bar size in accordance with Table R609.2.2C.

**R609.2.3 Location of reinforcement**. Reinforcement shall be located in the top of bond beams and in the top and bottom of bond beams also serving as lintels.

**R609.2.4 Corner continuity.** Corner continuity. Reinforcement in bond beams shall be continuous around corners as detailed in Figure R609.2.4.

**Exception:** In bond beams requiring two reinforcing bars, one bar shall be continuous around corners.

**R609.2.5 Change in height.** Changes in bond beam height shall be permitted as detailed in Figure R609.2.5.



**FIGURE R609.2.4**

**CORNER CONTINUITY OF BOND BEAM AND WALL REINFORCEMENT**



**FIGURE R609.2.5**

**CHANGES IN BOND BEAM HEIGHT**

**R609.2.6 Precast units reinforcement.** Precast bond beams shall properly receive and retain all vertical wall reinforcement. Precast bond beams shall contain the minimum amount of continuous reinforcement set forth in Sections R609.2.2 and R609.6 as applicable and shall be reinforced at joints o act as drag struts and diaphragm chords.

**TABLE R609.2.2A-1 GRADE 60 EXPOSURE B**

**ROOF DIAPHRAGM CHORD TENSION BOND BEAM STEEL AREA, IN2**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **BUILDING WIDTH** | **WALL HEIGHT** | **BUILDING LENGTH** | | | | |
| **40** | **50** | **60** | **70** | **80** |
| 100 | | 24 | 10 | 0.037 | 0.052 | 0.069 | 0.088 | 0.110 |
| 24 | 8 | 0.030 | 0.042 | 0.055 | 0.071 | 0.088 |
| 32 | 10 | 0.029 | 0.040 | 0.053 | 0.067 | 0.084 |
| 32 | 8 | 0.023 | 0.032 | 0.042 | 0.054 | 0.067 |
| 40 | 10 | 0.026 | 0.036 | 0.047 | 0.059 | 0.073 |
| 40 | 8 | 0.021 | 0.029 | 0.037 | 0.047 | 0.058 |
| 110 | | 24 | 10 | 0,045 | 0.063 | 0.084 | 0.107 | 0.133 |
| 24 | 8 | 0.036 | 0.050 | 0.067 | 0.086 | 0.107 |
| 32 | 10 | 0.035 | 0.048 | 0.064 | 0.082 | 0.101 |
| 32 | 8 | 0.028 | 0.039 | 0.051 | 0.065 | 0.081 |
| 40 | 10 | 0.032 | 0.043 | 0.057 | 0.072 | 0.088 |
| 40 | 8 | 0.025 | 0.035 | 0.045 | 0.057 | 0.070 |
| 120 | | 24 | 10 | 0.054 | 0.075 | 0.099 | 0.127 | 0.158 |
| 24 | 8 | 0.043 | 0.060 | 0.080 | 0.102 | 0.127 |
| 32 | 10 | 0.041 | 0.058 | 0.076 | 0.097 | 0.121 |
| 32 | 8 | 0.033 | 0.046 | 0.061 | 0.078 | 0.097 |
| 40 | 10 | 0.038 | 0.052 | 0.067 | 0.085 | 0.105 |
| 40 | 8 | 0.030 | 0.041 | 0.054 | 0.068 | 0.084 |
| 130 | | 24 | 10 | 0.063 | 0.088 | 0.117 | 0.149 | 0.186 |
| 24 | 8 | 0.050 | 0.070 | 0.093 | 0.120 | 0.149 |
| 32 | 10 | 0.049 | 0.068 | 0.089 | 0.114 | 0.142 |
| 32 | 8 | 0.039 | 0.054 | 0.071 | 0.091 | 0.113 |
| 40 | 10 | 0.044 | 0.061 | 0.079 | 0.100 | 0.123 |
| 40 | 8 | 0.035 | 0.048 | 0.063 | 0.080 | 0.098 |
| 140 | | 24 | 10 | 0.073 | 0.102 | 0.135 | 0.173 | 0.216 |
| 24 | 8 | 0.058 | 0.082 | 0.108 | 0.139 | 0.173 |
| 32 | 10 | 0.056 | 0.078 | 0.104 | 0.132 | 0.164 |
| 32 | 8 | 0.045 | 0.063 | 0.083 | 0.106 | 0.131 |
| 40 | 10 | 0.051 | 0.070 | 0.092 | 0.116 | 0.143 |
| 40 | 8 | 0.041 | 0.056 | 0.073 | 0.093 | 0.114 |
| 150 | | 24 | 10 | 0.084 | 0.117 | 0.155 | 0.199 | 0.248 |
| 24 | 8 | 0.067 | 0.094 | 0.124 | 0.159 | 0.198 |
| 32 | 10 | 0.065 | 0.090 | 0.119 | 0.152 | 0.189 |
| 32 | 8 | 0.052 | 0.072 | 0.095 | 0.121 | 0.151 |
| 40 | 10 | 0.059 | 0.081 | 0.105 | 0.133 | 0.164 |
| 40 | 8 | 0.047 | 0.064 | 0.084 | 0.106 | 0.131 |
| **Notes:** | | | | | | |
| 1. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C. | | | | | | |
| 2. The tabular value for diaphragm chord tension steel area shall be permitted to be multiplied by a factor of 0.65 for bond beam spans located in the end zone. | | | | | | |

**TABLE R609.2.2A-2 GRADE 60 ROOF EXPOSURE C**

**ROOF DIAPHRAGM CHORD TENSION BOND BEAM STEEL AREA, IN2**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **Vasd as determined in accordance with  Section R301.2.1.3** | | **BUILDING WIDTH** | **WALL HEIGHT** | **BUILDING LENGTH** | | | | |
| **40** | **50** | **60** | **70** | **80** |
| 100 | | 24 | 10 | 0.052 | 0.073 | 0.097 | 0.124 | 0.154 |
| 24 | 8 | 0.042 | 0.058 | 0.077 | 0.099 | 0.123 |
| 32 | 10 | 0.040 | 0.056 | 0.074 | 0.095 | 0.118 |
| 32 | 8 | 0.032 | 0.045 | 0.059 | 0.076 | 0.094 |
| 40 | 10 | 0.037 | 0.050 | 0.066 | 0.083 | 0.102 |
| 40 | 8 | 0.029 | 0.040 | 0.052 | 0.066 | 0.082 |
| 110 | | 24 | 10 | 0.063 | 0.088 | 0.117 | 0.150 | 0.187 |
| 24 | 8 | 0.051 | 0.071 | 0.094 | 0.120 | 0.149 |
| 32 | 10 | 0.049 | 0.068 | 0.090 | 0.114 | 0.142 |
| 32 | 8 | 0.039 | 0.054 | 0.072 | 0.092 | 0.114 |
| 40 | 10 | 0.044 | 0.061 | 0.079 | 0.100 | 0.124 |
| 40 | 8 | 0.035 | 0.049 | 0.063 | 0.080 | 0.099 |
| 120 | | 24 | 10 | 0.075 | 0.105 | 0.139 | 0.178 | 0.222 |
| 24 | 8 | 0.060 | 0.084 | 0.112 | 0.143 | 0.178 |
| 32 | 10 | 0.058 | 0.081 | 0.107 | 0.136 | 0.169 |
| 32 | 8 | 0.046 | 0.065 | 0.085 | 0.109 | 0.135 |
| 40 | 10 | 0.053 | 0.072 | 0.094 | 0.119 | 0.147 |
| 40 | 8 | 0.042 | 0.058 | 0.076 | 0.095 | 0.118 |
| 130 | | 24 | 10 | 0.088 | 0.123 | 0.164 | 0.209 | 0.261 |
| 24 | 8 | 0.071 | 0.099 | 0.131 | 0.168 | 0.209 |
| 32 | 10 | 0.068 | 0.095 | 0.125 | 0.160 | 0.199 |
| 32 | 8 | 0.055 | 0.076 | 0.100 | 0.128 | 0.159 |
| 40 | 10 | 0.062 | 0.085 | 0.111 | 0.140 | 0.173 |
| 40 | 8 | 0.050 | 0.068 | 0.089 | 0.112 | 0.138 |
| 140 | | 24 | 10 | 0.102 | 0.143 | 0.190 | 0.243 | 0.302 |
| 24 | 8 | 0.082 | 0.114 | 0.152 | 0.194 | 0.242 |
| 32 | 10 | 0.079 | 0.110 | 0.145 | 0.185 | 0.230 |
| 32 | 8 | 0.063 | 0.088 | 0.116 | 0.148 | 0.184 |
| 40 | 10 | 0.072 | 0.098 | 0.129 | 0.162 | 0.200 |
| 40 | 8 | 0.057 | 0.079 | 0.103 | 0.130 | 0.160 |
| 150 | | 24 | 10 | 0.118 | 0.164 | 0.218 | 0.279 | 0.347 |
| 24 | 8 | 0.094 | 0.131 | 0.174 | 0.223 | 0.278 |
| 32 | 10 | 0.091 | 0.126 | 0.167 | 0.213 | 0.264 |
| 32 | 8 | 0.073 | 0.101 | 0.133 | 0.170 | 0.212 |
| 40 | 10 | 0.082 | 0.113 | 0.148 | 0.187 | 0.230 |
| 40 | 8 | 0.066 | 0.090 | 0.118 | 0.149 | 0.184 |
| **Notes:** | | | | | | |
| 1. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C. | | | | | | |
| 2. The tabular value for diaphragm chord tension steel area shall be permitted to be multiplied by a factor of 0.65 for bond beam spans located in the end zone. | | | | | | |

**TABLE R609.2.2A-3 GRADE 40 EXPOSURE B**

**ROOF DIAPHRAGM CHORD TENSION BOND BEAM STEEL AREA, IN2**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **BUILDING WIDTH** | **WALL HEIGHT** | **BUILDING LENGTH** | | | | |
| **40** | **50** | **60** | **70** | **80** |
| 100 | | 24 | 10 | 0.056 | 0.078 | 0.104 | 0.133 | 0.165 |
| 24 | 8 | 0.045 | 0.062 | 0.083 | 0.106 | 0.132 |
| 32 | 10 | 0.043 | 0.060 | 0.079 | 0.101 | 0.126 |
| 32 | 8 | 0.035 | 0.048 | 0.063 | 0.081 | 0.101 |
| 40 | 10 | 0.039 | 0.054 | 0.070 | 0.089 | 0.109 |
| 40 | 8 | 0.031 | 0.043 | 0.056 | 0.071 | 0.087 |
| 110 | | 24 | 10 | 0.068 | 0.094 | 0.125 | 0.160 | 0.200 |
| 24 | 8 | 0.054 | 0.076 | 0.100 | 0.128 | 0.160 |
| 32 | 10 | 0.052 | 0.073 | 0.096 | 0.122 | 0.152 |
| 32 | 8 | 0.042 | 0.058 | 0.077 | 0.098 | 0.122 |
| 40 | 10 | 0.047 | 0.065 | 0.085 | 0.107 | 0.132 |
| 40 | 8 | 0.038 | 0.052 | 0.068 | 0.086 | 0.106 |
| 120 | | 24 | 10 | 0.081 | 0.112 | 0.149 | 0.191 | 0.238 |
| 24 | 8 | 0.064 | 0.090 | 0.119 | 0.153 | 0.190 |
| 32 | 10 | 0.062 | 0.086 | 0.114 | 0.146 | 0.181 |
| 32 | 8 | 0.050 | 0.069 | 0.091 | 0.117 | 0.145 |
| 40 | 10 | 0.056 | 0.077 | 0.101 | 0.128 | 0.157 |
| 40 | 8 | 0.045 | 0.062 | 0.081 | 0.102 | 0.126 |
| 130 | | 24 | 10 | 0.095 | 0.132 | 0.175 | 0.224 | 0.279 |
| 24 | 8 | 0.076 | 0.106 | 0.140 | 0.179 | 0.223 |
| 32 | 10 | 0.073 | 0.101 | 0.134 | 0.171 | 0.212 |
| 32 | 8 | 0.058 | 0.081 | 0.107 | 0.137 | 0.170 |
| 40 | 10 | 0.066 | 0.091 | 0.119 | 0.150 | 0.185 |
| 40 | 8 | 0.053 | 0.073 | 0.095 | 0.120 | 0.148 |
| 140 | | 24 | 10 | 0.110 | 0.153 | 0.203 | 0.260 | 0.324 |
| 24 | 8 | 0.088 | 0.122 | 0.162 | 0.208 | 0.259 |
| 32 | 10 | 0.085 | 0.117 | 0.155 | 0.198 | 0.246 |
| 32 | 8 | 0.068 | 0.094 | 0.124 | 0.159 | 0.197 |
| 40 | 10 | 0.077 | 0.105 | 0.138 | 0.174 | 0.214 |
| 40 | 8 | 0.062 | 0.084 | 0.110 | 0.139 | 0.171 |
| 150 | | 24 | 10 | 0.126 | 0.176 | 0.233 | 0.298 | 0.371 |
| 24 | 8 | 0.101 | 0.140 | 0.186 | 0.239 | 0.297 |
| 32 | 10 | 0.097 | 0.135 | 0.178 | 0.228 | 0.283 |
| 32 | 8 | 0.078 | 0.108 | 0.143 | 0.182 | 0.226 |
| 40 | 10 | 0.088 | 0.121 | 0.158 | 0.200 | 0.246 |
| 40 | 8 | 0.071 | 0.097 | 0.126 | 0.160 | 0.197 |
| **Notes:** | | | | | | | |
| 1. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C | | | | | | | |
| 2. The tabular value for diaphragm chord tension steel area shall be permitted to be multiplied by a factor of 0.65 for bond beam spans located in the end zone. | | | | | | | |

**TABLE R609.2.2A-4 GRADE 40 EXPOSURE C**

**ROOF DIAPHRAGM CHORD TENSION BOND BEAM STEEL AREA, IN2**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **Vasd as determined in  accordance with Section R301.2.1.3** | | **BUILDING WIDTH** | **WALL HEIGHT** | **BUILDING LENGTH** | | | | |
| **40** | **50** | 60 | 70 | **80** |
| 100 | | 24 | 10 | 0.078 | 0.109 | 0.145 | 0.186 | 0.231 |
| 24 | 8 | 0.063 | 0.088 | 0.116 | 0.149 | 0.185 |
| 32 | 10 | 0.060 | 0.084 | 0.111 | 0.142 | 0.176 |
| 32 | 8 | 0.048 | 0.067 | 0.089 | 0.114 | 0.141 |
| 40 | 10 | 0.055 | 0.075 | 0.098 | 0.124 | 0.153 |
| 40 | 8 | 0.044 | 0.060 | 0.079 | 0.099 | 0.123 |
| 110 | | 24 | 10 | 0.095 | 0.132 | 0.176 | 0.225 | 0.280 |
| 24 | 8 | 0.076 | 0.106 | 0.141 | 0.180 | 0.224 |
| 32 | 10 | 0.073 | 0.102 | 0.135 | 0.172 | 0.213 |
| 32 | 8 | 0.059 | 0.081 | 0.108 | 0.137 | 0.171 |
| 40 | 10 | 0.067 | 0.091 | 0.119 | 0.150 | 0.185 |
| 40 | 8 | 0.053 | 0.073 | 0.095 | 0.120 | 0.148 |
| 120 | | 24 | 10 | 0.113 | 0.158 | 0.209 | 0.268 | 0.333 |
| 24 | 8 | 0.090 | 0.126 | 0.167 | 0.214 | 0.267 |
| 32 | 10 | 0.087 | 0.121 | 0.160 | 0.204 | 0.254 |
| 32 | 8 | 0.070 | 0.097 | 0.128 | 0.163 | 0.203 |
| 40 | 10 | 0.079 | 0.108 | 0.142 | 0.179 | 0.221 |
| 40 | 8 | 0.063 | 0.087 | 0.113 | 0.143 | 0.176 |
| 130 | | 24 | 10 | 0.133 | 0.185 | 0.245 | 0.314 | 0.391 |
| 24 | 8 | 0.1066 | 0.148 | 0.196 | 0.251 | 0.313 |
| 32 | 10 | 0.102 | 0.142 | 0.188 | 0.240 | 0.298 |
| 32 | 8 | 0.082 | 0.114 | 0.150 | 0.192 | 0.238 |
| 40 | 10 | 0.093 | 0.127 | 0.166 | 0.210 | 0.259 |
| 40 | 8 | 0.074 | 0.102 | 0.133 | 0.168 | 0.207 |
| 140 | | 24 | 10 | 0.154 | 0.214 | 0.285 | 0.364 | 0.454 |
| 24 | 8 | 0.123 | 0.172 | 0.288 | 0.291 | 0.363 |
| 32 | 10 | 0.119 | 0.165 | 0.218 | 0.278 | 0.345 |
| 32 | 8 | 0.095 | 0.132 | 0.174 | 0.223 | 0.276 |
| 40 | 10 | 0.108 | 0.148 | 0.193 | 0.244 | 0.300 |
| 40 | 8 | 0.086 | 0.118 | 0.154 | 0.195 | 0.240 |
| 150 | | 24 | 10 | 0.176 | 0.246 | 0.327 | 0.418 | 0.521 |
| 24 | 8 | 0.141 | 0.197 | 0.261 | 0.335 | 0.417 |
| 32 | 10 | 0.136 | 0.189 | 0.250 | 0.319 | 0.397 |
| 32 | 8 | 0.109 | 0.151 | 0.200 | 0.255 | 0.317 |
| 40 | 10 | 0.124 | 0.169 | 0.221 | 0.280 | 0.345 |
| 40 | 8 | 0.099 | 0.136 | 0.177 | 0.224 | 0.276 |
| **Notes:** | | | | | | |
| 1. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C. | | | | | | |
| 2. The tabular value for diaphragm chord tension steel area shall be permitted to be multiplied by a factor of 0.65 for bond beam spans located in the end zone. | | | | | | |

**(S6133 GlitchAS)**

**TABLE R609.2.2B-1 GRADE 60**

**AREA OF STEEL REQUIRED IN BOND BEAM FOR UPLIFT BENDING, IN2**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **UPLIFT, plf  (ALLOWABLE  STRESS  DESIGN)** | | **8 IN. BOND BEAM/LINTEL SPAN, FT.** | | | | | | | |
| **4** | **6** | **8** | **10** | **12** | **14** | **16** | **18** |
| 50 | | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 100 | | 0.009 | 0.021 | 0.038 | 0.060 | 0.088 | 0.123 | NP | NP |
| 150 | | 0.016 | 0.037 | 0.067 | 0.107 | 0.159 | NP | NP | NP |
| 200 | | 0.023 | 0.053 | 0.096 | 0.157 | NP | NP | NP | NP |
| 250 | | 0.030 | 0.069 | 0.127 | 0.211 | NP | NP | NP | NP |
| 300 | | 0.037 | 0.086 | 0.160 | 0.270 | NP | NP | NP | NP |
| 350 | | 0.044 | 0.103 | 0.194 | NP | NP | NP | NP | NP |
| 400 | | 0.051 | 0.120 | 0.230 | NP | NP | NP | NP | NP |
| 450 | | 0.058 | 0.138 | 0.269 | NP | NP | NP | NP | NP |
| 500 | | 0.065 | 0.156 | NP | NP | NP | NP | NP | NP |
| 550 | | 0.073 | 0.175 | NP | NP | NP | NP | NP | NP |
| 600 | | 0.080 | 0.195 | NP | NP | NP | NP | NP | NP |
| 650 | | 0.088 | 0.215 | NP | NP | NP | NP | NP | NP |
| 700 | | 0.095 | 0.235 | NP | NP | NP | NP | NP | NP |
| 750 | | 0.103 | 0.257 | NP | NP | NP | NP | NP | NP |
| 800 | | 0.110 | 0.280 | NP | NP | NP | NP | NP | NP |
| 850 | | 0.118 | NP | NP | NP | NP | NP | NP | NP |
| 900 | | 0.126 | NP | NP | NP | NP | NP | NP | NP |
| 950 | | 0.134 | NP | NP | NP | NP | NP | NP | NP |
| 1000 | | 0.142 | NP | NP | NP | NP | NP | NP | NP |
| 1050 | | 0.150 | NP | NP | NP | NP | NP | NP | NP |
| 1100 | | 0.158 | NP | NP | NP | NP | NP | NP | NP |
| **Notes:** | | | | | | | |
| 1. When reinforcement required is 0.00, only diaphragm chord tension reinforcement is required. | | | | | | | |
| 2. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C. | | | | | | | |

**TABLE R609.2.2B-2 GRADE 60**

**AREA OF STEEL REQUIRED IN BOND BEAM FOR UPLIFT BENDING, IN2**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **UPLIFT, plf  (ALLOWABLE  STRESS DESIGN)** | | **16 IN. BOND BEAM/LINTEL SPAN, FT.** | | | | | | | |
| **4** | **6** | **8** | **10** | **12** | **14** | **16** | **18** |
| 50 | | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 100 | | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 150 | | 0.005 | 0.010 | 0.019 | 0.029 | 0.042 | 0.58 | 0.076 | 0.097 |
| 200 | | 0.007 | 0.017 | 0.030 | 0.046 | 0.067 | 0.092 | 0.121 | 0.154 |
| 250 | | 0.010 | 0.023 | 0.040 | 0.063 | 0.092 | 0.126 | 0.167 | 0.214 |
| 300 | | 0.013 | 0.029 | 0.051 | 0.081 | 0.117 | 0.162 | 0.214 | 0.275 |
| 350 | | 0.015 | 0.035 | 0.062 | 0.098 | 0.143 | 0.197 | 0.262 | NP |
| 400 | | 0.018 | 0.041 | 0.073 | 0.116 | 0.169 | 0.234 | 0.312 | NP |
| 450 | | 0.021 | 0.047 | 0.084 | 0.134 | 0.195 | 0.271 | NP | NP |
| 500 | | 0.023 | 0.053 | 0.096 | 0.152 | 0.222 | 0.309 | NP | NP |
| 550 | | 0.026 | 0.059 | 0.107 | 0.170 | 0.249 | 0.348 | NP | NP |
| 600 | | 0.029 | 0.066 | 0.118 | 0.188 | 0.277 | 0.388 | NP | NP |
| 650 | | 0.032 | 0.072 | 0.130 | 0.206 | 0.305 | 0.429 | NP | NP |
| 700 | | 0.034 | 0.078 | 0.141 | 0.225 | 0.334 | NP | NP | NP |
| 750 | | 0.037 | 0.084 | 0.152 | 0.244 | 0.363 | NP | NP | NP |
| 800 | | 0.040 | 0.091 | 0.164 | 0.263 | 0.392 | NP | NP | NP |
| 850 | | 0.042 | 0.097 | 0.176 | 0.282 | 0.422 | NP | NP | NP |
| 900 | | 0.045 | 0.103 | 0.187 | 0.302 | 0.453 | NP | NP | NP |
| 950 | | 0.048 | 0.110 | 0.199 | 0.321 | NP | NP | NP | NP |
| 1000 | | 0.051 | 0.116 | 0.211 | 0.341 | NP | NP | NP | NP |
| 1050 | | 0.053 | 0.122 | 0.223 | 0.362 | NP | NP | NP | NP |
| 1100 | | 0.056 | 0.129 | 0.235 | 0.382 | NP | NP | NP | NP |
| **Notes:** | | | | | | | |
| 1. When reinforcement required is 0.00, only diaphragm chord tension reinforcement is required. | | | | | | | |
| 2. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C. | | | | | | | |

**(S6133 GlitchStafford)**

**TABLE R609.2.2B-3 GRADE 60**

**AREA OF STEEL REQUIRED IN BOND BEAM FOR UPLIFT BENDING, IN2**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **UPLIFT, plf (ALLOWABLE STRESS DESIGN)** | **24 IN. BOND BEAM/LINTEL SPAN, FT.** | | | | | | | | |
| **4** | **6** | **8** | **10** | **12** | **14** | **16** | **18** | |
| 50 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | |
| 100 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | |
| 150 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | |
| 200 | 0.004 | 0.008 | 0.014 | 0.022 | 0.032 | 0.043 | 0.057 | 0.072 | |
| 250 | 0.005 | 0.012 | 0.021 | 0.032 | 0.047 | 0.064 | 0.084 | 0.106 | |
| 300 | 0.007 | 0.015 | 0.027 | 0.043 | 0.062 | 0.085 | 0.1111 | 0.142 | |
| 350 | 0.009 | 0.019 | 0.034 | 0.054 | 0.077 | 0.106 | 0.139 | 0.177 | |
| 400 | 0.010 | 0.023 | 0.041 | 0.064 | 0.093 | 0.127 | 0.167 | 0.213 | |
| 450 | 0.012 | 0.027 | 0.048 | 0.075 | 0.108 | 0.148 | 0.195 | 0.249 | |
| 500 | 0.014 | 0.031 | 0.054 | 0.086 | 0.124 | 0.170 | 0.224 | 0.286 | |
| 550 | 0.015 | 0.034 | 0.061 | 0.096 | 0.140 | 0.192 | 0.253 | 0.323 | |
| 600 | 0.017 | 0.038 | 0.068 | 0.107 | 0.155 | 0.213 | 0.282 | 0.361 | |
| 650 | 0.019 | 0.042 | 0.075 | 0.118 | 0.171 | 0.235 | 0.311 | 0.399 | |
| 700 | 0.020 | 0.046 | 0.082 | 0.129 | 0.187 | 0.257 | 0.341 | 0.438 | |
| 750 | 0.022 | 0.050 | 0.089 | 0.140 | 0.203 | 0.280 | 0.371 | 0.477 | |
| 800 | 0.024 | 0.053 | 0.095 | 0.150 | 0.219 | 0.302 | 0.401 | 0.517 | |
| 850 | 0.025 | 0.057 | 0.102 | 0.161 | 0.235 | 0.325 | 0.423 | 0.558 | |
| 900 | 0.027 | 0.061 | 0.109 | 0.172 | 0.251 | 0.347 | 0.462 | NP | |
| 950 | 0.029 | 0.065 | 0.116 | 0.183 | 0.268 | 0.370 | 0.494 | NP | |
| 1000 | 0.030 | 0.069 | 0.123 | 0.194 | 0.284 | 0.394 | 0.525 | NP | |
| 1050 | 0.032 | 0.072 | 0.130 | 0.206 | 0.301 | 0.417 | 0.557 | NP | |
| 1100 | 0.034 | 0.076 | 0.137 | 0.217 | 0.317 | 0.440 | NP | NP | |
| **Notes:** | | | | | | | | | |
| 1. When reinforcement required is 0.00, only diaphragm chord tension reinforcement is required. | | | | | | | | | |
| 2. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C.  **(S6133 GlitchStafford)** | | | | | | | | | |

**TABLE R609.2.2B-4 GRADE 60**

**AREA OF STEEL REQUIRED IN BOND BEAM FOR UPLIFT BENDING, IN2**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **UPLIFT, plf (ALLOWABLE STRESS  DESIGN)** | **32 IN. BOND BEAM/LINTEL SPAN, FT.** | | | | | | | |
| **4** | **6** | **8** | **10** | **12** | **14** | **16** | **18** |
| 50 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 100 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 150 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 200 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 250 | 0.003 | 0.007 | 0.012 | 0.019 | 0.027 | 0.037 | 0.048 | 0.061 |
| 300 | 0.004 | 0.009 | 0.017 | 0.026 | 0.038 | 0.052 | 0.068 | 0.086 |
| 350 | 0.005 | 0.012 | 0.022 | 0.034 | 0.049 | 0.067 | 0.087 | 0.111 |
| 400 | 0.007 | 0.015 | 0.027 | 0.042 | 0.060 | 0.082 | 0.107 | 0.136 |
| 450 | 0.008 | 0.018 | 0.031 | 0.049 | 0.071 | 0.097 | 0.127 | 0.161 |
| 500 | 0.009 | 0.020 | 0.036 | 0.057 | 0.082 | 0.112 | 0.147 | 0.187 |
| 550 | 0.010 | 0.023 | 0.041 | 0.065 | 0.093 | 0.127 | 0.167 | 0.213 |
| 600 | 0.011 | 0.026 | 0.046 | 0.072 | 0.104 | 0.143 | 0.187 | 0.239 |
| 650 | 0.013 | 0.029 | 0.051 | 0.080 | 0.116 | 0.158 | 0.208 | 0.265 |
| 700 | 0.014 | 0.031 | 0.056 | 0.088 | 0.127 | 0.174 | 0.288 | 0.291 |
| 750 | 0.015 | 0.034 | 0.061 | 0.095 | 0.138 | 0.189 | 0.249 | 0.317 |
| 800 | 0.016 | 0.037 | 0.066 | 0.103 | 0.149 | 0.205 | 0.269 | 0.344 |
| 850 | 0.018 | 0.040 | 0.071 | 0.111 | 0.161 | 0.220 | 0.290 | 0.370 |
| 900 | 0.019 | 0.042 | 0.076 | 0.119 | 0.172 | 0.236 | 0.311 | 0.397 |
| 950 | 0.020 | 0.045 | 0.081 | 0.127 | 0.183 | 0.252 | 0.332 | 0.424 |
| 1000 | 0.021 | 0.048 | 0.085 | 0.134 | 0.195 | 0.267 | 0.353 | 0.451 |
| 1050 | 0.022 | 0.051 | 0.090 | 0.142 | 0.206 | 0.283 | 0.374 | 0.479 |
| 1100 | 0.024 | 0.053 | 0.095 | 0.150 | 0.218 | 0.299 | 0.395 | 0.506 |
| **Notes:** | | | | | | | |
| 1. When reinforcement required is 0.00, only diaphragm chord tension reinforcement is required. | | | | | | | |
| 2. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C.  **(S6133 GlitchStafford)** | | | | | | | |

**TABLE R609.2.2B-5 GRADE 40**

**AREA BOND BEAM/LINTEL UPLIFT STEEL DESIGN**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **UPLIFT, plf  (ALLOWABLE  STRESS DESIGN)** | **8 IN. BOND BEAM/LINTEL SPAN, FT.** | | | | | | | |
| **4** | **6** | **8** | **10** | **12** | **14** | **16** | **18** |
| 50 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 100 | 0.014 | 0.032 | 0.057 | 0.090 | 0.132 | 0.184 | NP | NP |
| 150 | 0.024 | 0.055 | 0.100 | 0.160 | 0.239 | NP | NP | NP |
| 200 | 0.034 | 0.079 | 0.144 | 0.235 | NP | NP | NP | NP |
| 250 | 0.045 | 0.103 | 0.191 | NP | NP | NP | NP | NP |
| 300 | 0.055 | 0.128 | 0.240 | NP | NP | NP | NP | NP |
| 350 | 0.066 | 0.154 | NP | NP | NP | NP | NP | NP |
| 400 | 0.076 | 0.180 | NP | NP | NP | NP | NP | NP |
| 450 | 0.087 | 0.207 | NP | NP | NP | NP | NP | NP |
| 500 | 0.098 | 0.234 | NP | NP | NP | NP | NP | NP |
| 550 | 0.109 | 0.263 | NP | NP | NP | NP | NP | NP |
| 600 | 0.120 | NP | NP | NP | NP | NP | NP | NP |
| 650 | 0.131 | NP | NP | NP | NP | NP | NP | NP |
| 700 | 0.143 | NP | NP | NP | NP | NP | NP | NP |
| 750 | 0.154 | NP | NP | NP | NP | NP | NP | NP |
| 800 | 0.166 | NP | NP | NP | NP | NP | NP | NP |
| 850 | 0.177 | NP | NP | NP | NP | NP | NP | NP |
| 900 | 0.189 | NP | NP | NP | NP | NP | NP | NP |
| 950 | 0.201 | NP | NP | NP | NP | NP | NP | NP |
| 1000 | 0.213 | NP | NP | NP | NP | NP | NP | NP |
| 1050 | 0.225 | NP | NP | NP | NP | NP | NP | NP |
| 1100 | 0.238 | NP | NP | NP | NP | NP | NP | NP |
| **Notes:** | | | | | | | |
| 1. When reinforcement required is 0.00, only diaphragm chord tension reinforcement is required. | | | | | | | |
| 2. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C. | | | | | | | |

**TABLE R609.2.2B-6 GRADE 40**

**AREA BOND BEAM/LINTEL UPLIFT STEEL DESIGN**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **UPLIFT, plf (ALLOWABLE STRESS DESIGN)** | **16 IN. BOND BEAM/LINTEL SPAN, FT.** | | | | | | | |
| **4** | **6** | **8** | **10** | **12** | **14** | **16** | **18** |
| 50 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 100 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 150 | 0.007 | 0.016 | 0.028 | 0.044 | 0.063 | 0.087 | 0.114 | 0.145 |
| 200 | 0.011 | 0.025 | 0.044 | 0.069 | 0.101 | 0.138 | 0.181 | 0.231 |
| 250 | 0.015 | 0.034 | 0.061 | 0.095 | 0.138 | 0.190 | 0.250 | 0.320 |
| 300 | 0.019 | 0.043 | 0.077 | 0.121 | 0.176 | 0.242 | 0.321 | 0.413 |
| 350 | 0.023 | 0.052 | 0.093 | 0.147 | 0.215 | 0.296 | 0.393 | NP |
| 400 | 0.027 | 0.061 | 0.110 | 0.174 | 0.254 | 0.351 | 0.468 | NP |
| 450 | 0.031 | 0.071 | 0.127 | 0.200 | 0.293 | 0.407 | NP | NP |
| 500 | 0.035 | 0.080 | 0.143 | 0.227 | 0.333 | 0.464 | NP | NP |
| 550 | 0.039 | 0.089 | 0.160 | 0.254 | 0.374 | 0.523 | NP | NP |
| 600 | 0.043 | 0.098 | 0.177 | 0.282 | 0.415 | 0.583 | NP | NP |
| 650 | 0.047 | 0.108 | 0.194 | 0.310 | 0.458 | 0.644 | NP | NP |
| 700 | 0.051 | 0.117 | 0.211 | 0.338 | 0.500 | NP | NP | NP |
| 750 | 0.056 | 0.126 | 0.229 | 0.366 | 0.544 | NP | NP | NP |
| 800 | 0.060 | 0.136 | 0.246 | 0.394 | 0.588 | NP | NP | NP |
| 850 | 0.064 | 0.145 | 0.264 | 0.423 | 0.633 | NP | NP | NP |
| 900 | 0.068 | 0.155 | 0.281 | 0.453 | 0.679 | NP | NP | NP |
| 950 | 0.072 | 0.164 | 0.299 | 0.482 | NP | NP | NP | NP |
| 1000 | 0.076 | 0.174 | 0.317 | 0.512 | NP | NP | NP | NP |
| 1050 | 0.080 | 0.183 | 0.335 | 0.542 | NP | NP | NP | NP |
| 1100 | 0.084 | 0.193 | 0.353 | 0.573 | NP | NP | NP | NP |
| **Notes:** | | | | | | | |
| 1. When reinforcement required is 0.00, only diaphragm chord tension reinforcement is required. | | | | | | | |
| 2. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C. | | | | | | | |

**TABLE R609.2.2B-7 GRADE 40**

**AREA BOND BEAM/LINTEL UPLIFT STEEL DESIGN**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **UPLIFT, plf (ALLOWABLE  STRESS  DESIGN)** | **24 IN. BOND BEAM/LINTEL SPAN, FT.** | | | | | | | |
| **4** | **6** | **8** | **10** | **12** | **14** | **16** | **18** |
| 50 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 100 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 150 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 200 | 0.005 | 0.012 | 0.021 | 0.033 | 0.048 | 0.065 | 0.085 | 0.108 |
| 250 | 0.008 | 0.017 | 0.031 | 0.049 | 0.070 | 0.096 | 0.126 | 0.160 |
| 300 | 0.010 | 0.023 | 0.041 | 0.065 | 0.093 | 0.127 | 0.167 | 0.212 |
| 350 | 0.013 | 0.029 | 0.051 | 0.080 | 0.116 | 0.159 | 0.209 | 0.266 |
| 400 | 0.015 | 0.034 | 0.061 | 0.096 | 0.139 | 0.191 | 0.251 | 0.319 |
| 450 | 0.018 | 0.040 | 0.072 | 0.112 | 0.163 | 0.223 | 0.293 | 0.374 |
| 500 | 0.020 | 0.046 | 0.082 | 0.128 | 0.186 | 0.255 | 0.336 | 0.429 |
| 550 | 0.023 | 0.051 | 0.092 | 0.144 | 0.209 | 0.287 | 0.379 | 0.485 |
| 600 | 0.025 | 0.057 | 0.102 | 0.161 | 0.233 | 0.320 | 0.423 | 0.542 |
| 650 | 0.028 | 0.063 | 0.112 | 0.177 | 0.257 | 0.353 | 0.467 | 0.599 |
| 700 | 0.030 | 0.069 | 0.123 | 0.193 | 0.280 | 0.386 | 0.511 | 0.657 |
| 750 | 0.033 | 0.074 | 0.133 | 0.209 | 0.304 | 0.419 | 0.556 | 0.716 |
| 800 | 0.035 | 0.080 | 0.143 | 0.226 | 0.329 | 0.453 | 0.601 | 0.776 |
| 850 | 0.038 | 0.086 | 0.154 | 0.242 | 0.353 | 0.487 | 0.647 | 0.837 |
| 900 | 0.040 | 0.091 | 0.164 | 0.259 | 0.377 | 0.521 | 0.694 | NP |
| 950 | 0.043 | 0.097 | 0.174 | 0.275 | 0.402 | 0.556 | 0.741 | NP |
| 1000 | 0.045 | 0.103 | 0.185 | 0.292 | 0.426 | 0.590 | 0.788 | NP |
| 1050 | 0.048 | 0.109 | 0.195 | 0.308 | 0.451 | 0.625 | 0.836 | NP |
| 1100 | 0.051 | 0.114 | 0.205 | 0.325 | 0.476 | 0.661 | NP | NP |
| **Notes:** | | | | | | | |
| 1. When reinforcement required is 0.00, only diaphragm chord tension reinforcement is required. | | | | | | | |
| 2. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C. | | | | | | | |

**TABLE R609.2.2B-8 GRADE 40**

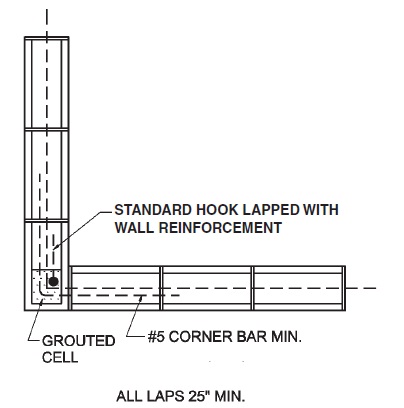
**AREA BOND BEAM/LINTEL UPLIFT STEEL DESIGN**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **UPLIFT, plf (ALLOWABLE  STRESS  DESIGN)** | **32 IN. BOND BEAM/LINTEL SPAN, FT.** | | | | | | | |
| **4** | **6** | **8** | **10** | **12** | **14** | **16** | **18** |
| 50 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 100 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 150 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 200 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 250 | 0.004 | 0.010 | 0.018 | 0.028 | 0.040 | 0.055 | 0.072 | 0.091 |
| 300 | 0.006 | 0.014 | 0.025 | 0.039 | 0.057 | 0.077 | 0.101 | 0.129 |
| 350 | 0.008 | 0.018 | 0.033 | 0.051 | 0.073 | 0.100 | 0.131 | 0.166 |
| 400 | 0.010 | 0.022 | 0.040 | 0.062 | 0.090 | 0.123 | 0.161 | 0.204 |
| 450 | 0.012 | 0.026 | 0.047 | 0.074 | 0.107 | 0.145 | 0.191 | 0.242 |
| 500 | 0.014 | 0.031 | 0.054 | 0.085 | 0.123 | 0.168 | 0.221 | 0.281 |
| 550 | 0.015 | 0.035 | 0.062 | 0.097 | 0.140 | 0.191 | 0.251 | 0.319 |
| 600 | 0.017 | 0.039 | 0.069 | 0.108 | 0.157 | 0.214 | 0.281 | 0.358 |
| 650 | 0.019 | 0.043 | 0.077 | 0.120 | 0.173 | 0.237 | 0.312 | 0.397 |
| 700 | 0.021 | 0.047 | 0.084 | 0.132 | 0.190 | 0.260 | 0.342 | 0.436 |
| 750 | 0.023 | 0.051 | 0.091 | 0.143 | 0.207 | 0.284 | 0.373 | 0.476 |
| 800 | 0.025 | 0.055 | 0.099 | 0.155 | 0.224 | 0.307 | 0.404 | 0.515 |
| 850 | 0.026 | 0.059 | 0.106 | 0.166 | 0.241 | 0.330 | 0.435 | 0.556 |
| 900 | 0.028 | 0.064 | 0.113 | 0.178 | 0.258 | 0.354 | 0.466 | 0.596 |
| 950 | 0.030 | 0.068 | 0.121 | 0.190 | 0.275 | 0.377 | 0.497 | 0.636 |
| 1000 | 0.032 | 0.072 | 0.128 | 0.201 | 0.292 | 0.401 | 0.529 | 0.677 |
| 1050 | 0.034 | 0.076 | 0.136 | 0.213 | 0.309 | 0.425 | 0.561 | 0.718 |
| 1100 | 0.035 | 0.080 | 0.143 | 0.225 | 0.326 | 0.449 | 0.592 | 0.760 |
| **Notes:** | | | | | | | |
| 1. When reinforcement required is 0.00, only diaphragm chord tension reinforcement is required. | | | | | | | |
| 2. Diaphragm chord tension steel area shall be added to bond beam uplift steel area for total required bond beam area of steel. Select appropriate bar size and number of bars from Table R609.2.2C. | | | | | | | |

**TABLE R609.2.2C**

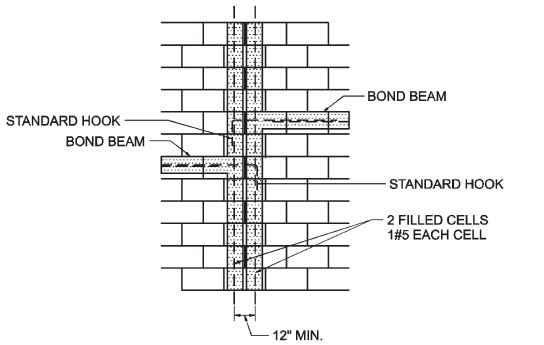
**BOND BEAM AREA OF STEEL PROVIDED IN2/FT**

|  |  |  |  |
| --- | --- | --- | --- |
| **NUMBER OF BARS** | **BAR SIZE** | | |
|  | No. 4 | No. 5 | No. 6 |
| 1 | 0.20 | 0.31 | 0.44 |
| 2 | 0.40 | 0.62 | 0.88 |
|  |  |  |  |
|  |  |  |  |



**Figure R609.2.4**

**Corner Continuity of Bond Beam and Wall Reinforcement**



**Figure R609.2.5**

**Changes in Bond Beam Height**

***(S6017 AS)***

**R609.3 Vertical Reinforcement**. Vertical reinforcement shall be provided in conformance with Sections R609.3.1 through R609.3.6.

**R609.3.1** One reinforcement bar shall be provided in each corner, including interior corners and corners created by changes in wall direction or offsetting of walls.

**R609.3.2 Openings.** A minimum of one bar of the size used for vertical wall reinforcement shall be provided on each side of openings wider than 6 feet. If more vertical reinforcement is interrupted by an opening than is provided beside the opening (total in the first and second cells adjacent to the opening), one-half of the equivalent area of reinforcement interrupted by the opening shall be placed on each side of the opening. This reinforcement shall be placed within the first and/or second cells beside the opening.

**R609.3.2.1 Girders.** At least one reinforcement bar shall be provided where girders or girder trusses bear on masonry walls. **(S6132GlitchAS)**

**R609.3.3 Spacing.** Vertical reinforcement shall be provided as set forth in Tables R609.3.3.A-1 through Table R609.3.3A-4 and R609.3.3.B-1, through R609.3.3B-4 as applicable.

**R609.3.4 Precast bond beams.** Vertical reinforcement used in conjunction with precast bond beams shall be spaced the same as for masonry bond beams. Reinforcement shall terminate in the precast beam as set forth in Section R606.9.8.

**R609.3.5 Duplication.** Reinforcing steel requirements shall not be additive. A single bar shall be permitted to satisfy multiple requirements.

**R609.3.6 Termination.** Vertical reinforcement shall terminate in footings and bond beams as set forth in Section R606.9.8.

**TABLE R609.3.3A-1 GRADE 60**

**SINGLE STORY AND TOP STORY WALLS PARALLEL TO RIDGE VERTICAL REINFORCEMENT SPACING NO. 5 BARS (5/8 IN.)**

|  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- |
| **EXPOSURE** | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **100** | 8.00 | 9.87 | 9.87 | 9.87 | 8.34 | 8.34 | 8.34 |
| 8.67 | 9.97 | 9.97 | 9.97 | 8.42 | 8.42 | 8.42 |
| 9.33 | 10.06 | 10.06 | 10.06 | 8.49 | 8.49 | 8.49 |
| 10.00 | 10.14 | 10.14 | 10.14 | 8.57 | 8.57 | 8.54 |
| **EXPOSURE** | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **110** | 8.00 | 8.97 | 8.7 | 8.97 | 7.58 | 7.58 | 7.58 |
| 8.67 | 9.06 | 9.06 | 9.06 | 7.65 | 7.56 | 7.65 |
| 9.33 | 9.14 | 9.14 | 9.14 | 7.72 | 7.72 | 7.72 |
| 10.00 | 9.22 | 9.22 | 9.22 | 7.79 | 7.79 | 7.79 |
| **EXPOSURE** | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **120** | 8.00 | 8.23 | 8.23 | 8.23 | 6.95 | 6.95 | 6.95 |
| 8.67 | 8.30 | 8.30 | 8.30 | 7.01 | 7.01 | 7.01 |
| 9.33 | 8.38 | 8.38 | 8.38 | 7.08 | 7.08 | 7.08 |
| 10.00 | 8.45 | 8.45 | 8.45 | 6.87 | 6.87 | 6.87 |
| **EXPOSURE** | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in  accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **130** | 8.00 | 7.59 | 7.59 | 7.59 | 6.41 | 6.41 | 6.21 |
| 8.67 | 7.67 | 7.67 | 7.67 | 6.47 | 6.14 | 5.67 |
| 9.33 | 7.73 | 7.73 | 7.57 | 6.03 | 5.58 | 5.19 |
| 10.00 | 7.80 | 7.36 | 6.90 | 5.44 | 5.07 | 4.74 |
| **EXPOSURE** | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **140** | 8.00 | 7.05 | 7.05 | 7.05 | 5.95 | 5.76 | 5.27 |
| 8.67 | 7.12 | 7.12 | 7.01 | 5.70 | 5.22 | 4.82 |
| 9.33 | 7.18 | 6.88 | 6.40 | 5.14 | 4.75 | 4.41 |
| 10.00 | 6.70 | 6.24 | 5.85 | 4.64 | 4.32 | 4.04 |
| **EXPOSURE** | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **150** | 8.00 | 6.58 | 6.58 | 6.58 | 5.46 | 4.96 | 4.54 |
| 8.67 | 6.64 | 6.50 | 6.01 | 4.91 | 4.50 | 4.15 |
| 9.33 | 6.39 | 5.91 | 5.49 | 4.43 | 4.09 | 3.80 |
| 10.00 | 5.76 | 5.37 | 5.02 | 4.00 | 3.72 | 3.48 |

**TABLE R609.3.3A-2 GRADE 60**

**SINGLE STORY AND TOP STORY WALLS PARALLEL TO RIDGE VERTICAL REINFORCEMENT SPACING NO. 4 BARS (1/2 IN.)**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXPOSURE** | | **B** | **B** | **B** | | **C** | | **C** | **C** |
| **Vasd as determined in  accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | **32** | | **40** | | **24** | **32** | **40** |
| **100** | 8.00 | 9.87 | 9.87 | | 9.87 | | 8.34 | 7.97 | 7.32 |
| 8.67 | 9.97 | 9.97 | | 9.97 | | 7.79 | 7.17 | 6.65 |
| 9.33 | 10.06 | 9.69 | | 9.08 | | 6.98 | 6.48 | 6.05 |
| 10.00 | 9.06 | 8.72 | | 8.22 | | 6.27 | 5.87 | 5.51 |
| **EXPOSURE** | | **B** | **B** | | **B** | | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | **32** | **40** | | **24** | | **32** | **40** |
| **110** | 8.00 | 8.97 | 8.97 | 8.76 | | 7.01 | | 6.38 | 5.85 |
| 8.67 | 9.06 | 9.06 | 9.06 | | 6.83 | | 6.83 | 6.83 |
| 9.33 | 8.45 | 8.45 | 8.45 | | 5.99 | | 5.99 | 5.99 |
| 10.00 | 7.47 | 7.47 | 7.47 | | 5.30 | | 5.30 | 5.30 |
| **EXPOSURE** | | **B** | **B** | **B** | | **C** | | **C** | **C** |
| **Vasd as determined in  accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | **32** | **40** | | **24** | | **32** | **40** |
| **120** | 8.00 | 8.23 | 7.72 | 7.09 | | 5.75 | | 5.23 | 4.80 |
| 8.67 | 8.07 | 8.07 | 8.07 | | 5.72 | | 5.72 | 5.72 |
| 9.33 | 7.08 | 7.08 | 7.08 | | 5.02 | | 5.02 | 5.02 |
| 10.00 | 6.26 | 6.26 | 6.26 | | 4.43 | | 4.43 | 4.43 |
| **EXPOSURE** | | **B** | **B** | **B** | | **C** | | **C** | **C** |
| **Vasd as determined in  accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | **32** | **40** | | **24** | | **32** | **40** |
| **130** | 8.00 | 7.04 | 6.41 | 5.88 | | 4.82 | | 4.38 | 4.01 |
| 8.67 | 6.29 | 5.78 | 5.35 | | 4.32 | | 3.96 | 3.66 |
| 9.33 | 5.65 | 5.24 | 4.88 | | 3.89 | | 3.60 | 3.35 |
| 10.00 | 5.09 | 4.75 | 4.45 | | 3.51 | | 3.27 | 3.06 |
| **EXPOSURE** | | **B** | **B** | **B** | | **C** | | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | **32** | **40** | | **24** | | **32** | **40** |
| **140** | 8.00 | 5.95 | 5.41 | 4.08 | | 4.09 | | 3.72 | 3.40 |
| 8.67 | 5.33 | 4.89 | 4.52 | | 3.68 | | 3.37 | 3.11 |
| 9.33 | 4.79 | 4.44 | 4.13 | | 3.31 | | 3.08 | 2.85 |
| 10.00 | 4.32 | 4.00 | 3.77 | | 2.99 | | 2.78 | 2.60 |
| **EXPOSURE** | | **B** | **B** | **B** | | **C** | | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | **32** | **40** | | **24** | | **32** | **40** |
| **150** | 8.00 | 5.10 | 4.63 | 4.25 | | 3.53 | | 3.20 | 2.93 |
| 8.67 | 4.57 | 4.20 | 3.87 | | 3.17 | | 2.90 | 2.68 |
| 9.33 | 4.12 | 3.81 | 3.54 | | 2.86 | | 2.64 | 2.45 |
| 10.00 | 3.71 | 3.46 | 3.24 | | 2.58 | | 2.40 | 2.24 |

**TABLE R609.3.3A-3 GRADE 40**

**SINGLE STORY AND TOP STORY WALLS PARALLEL TO RIDGE VERTICAL REINFORCEMENT SPACING NO. 5 BARS (5/8 IN.)**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXPOSURE** | | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as  determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **100** | 8.00 | | 9.87 | 9.87 | 9.87 | 8.34 | 8.23 | 7.57 |
| 8.67 | | 9.97 | 9.97 | 9.97 | 8.05 | 7.41 | 6.87 |
| 9.33 | | 10.06 | 10.01 | 9.38 | 7.22 | 6.70 | 6.25 |
| 10.00 | | 9.36 | 9.01 | 8.49 | 6.48 | 6.06 | 5.69 |
| **EXPOSURE** | | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in  accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **110** | | 8.00 | 8.97 | 8.97 | 8.97 | 7.24 | 6.59 | 6.05 |
| 8.67 | 9.06 | 9.06 | 9.06 | 7.06 | 7.06 | 7.06 |
| 9.33 | 8.73 | 8.73 | 8.73 | 6.19 | 6.19 | 6.19 |
| 10.00 | 7.72 | 7.72 | 7.72 | 5.47 | 5.47 | 5.47 |
| **EXPOSURE** | | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **120** | | 8.00 | 8.23 | 7.98 | 7.33 | 5.95 | 5.41 | 4.96 |
| 8.67 | 8.30 | 8.30 | 8.30 | 5.92 | 5.92 | 5.92 |
| 9.33 | 7.32 | 7.32 | 7.32 | 5.19 | 5.19 | 5.19 |
| 10.00 | 6.47 | 6.47 | 6.47 | 4.58 | 4.58 | 4.58 |
| **EXPOSURE** | | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **130** | | 8.00 | 7.27 | 6.62 | 6.07 | 4.98 | 4.52 | 4.14 |
| 8.67 | 6.50 | 5.98 | 5.53 | 4.47 | 4.10 | 3.78 |
| 9.33 | 5.84 | 5.41 | 5.04 | 4.02 | 3.72 | 3.46 |
| 10.00 | 5.26 | 4.91 | 4.60 | 3.63 | 3.38 | 3.16 |
| **EXPOSURE** | | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **140** | | 8.00 | 6.15 | 5.59 | 5.13 | 4.23 | 3.84 | 3.52 |
| 8.67 | 5.51 | 5.06 | 4.67 | 3.80 | 3.48 | 3.21 |
| 9.33 | 4.95 | 4.59 | 4.27 | 3.43 | 3.17 | 2.94 |
| 10.00 | 4.46 | 4.16 | 3.90 | 3.09 | 2.88 | 2.69 |
| **EXPOSURE** | | | **B** | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **150** | | 8.00 | 5.27 | 4.79 | 4.39 | 3.64 | 3.31 | 3.02 |
| 8.67 | 4.73 | 4.34 | 4.00 | 3.27 | 3.00 | 2.76 |
| 9.33 | 4.26 | 3.94 | 3.66 | 2.95 | 2.73 | 2.53 |
| 10.00 | 3.84 | 3.58 | 3.35 | 2.66 | 2.48 | 2.32 |

**TABLE R609.3.3A-4 GRADE 40**

**SINGLE STORY AND TOP STORY WALLS PARALLEL TO RIDGE VERTICAL REINFORCEMENT SPACING NO. 4 BARS (1/2 IN.)**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXPOSURE** | | | **B** | | **B** | **B** | **C** | **C** | **C** |
| **Vasd as determined in accordance with Section R301.2.1.3** | **WALL HEIGHT** | | **BUILDING WIDTH** | | | | | | |
| **24** | **32** | | **40** | **24** | **32** | **40** | |
| **100** | 8.00 | | 8.78 | 8.04 | | 7.42 | 5.82 | 5.31 | 4.88 | |
| 8.67 | | 7.77 | 7.19 | | 6.69 | 5.19 | 4.78 | 4.43 | |
| 9.33 | | 6.83 | 6.46 | | 6.05 | 4.66 | 4.32 | 4.03 | |
| 10.00 | | 6.04 | 5.81 | | 5.48 | 4.18 | 3.91 | 3.67 | |
| **EXPOSURE** | | | **B** | **B** | | **B** | **C** | **C** | **C** | |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | | **32** | **40** | **24** | **32** | **40** | |
| **110** | | 8.00 | 6.94 | | 6.34 | 5.84 | 4.67 | 4.25 | 3.90 | |
| 8.67 | 6.42 | | 6.42 | 6.42 | 4.56 | 4.56 | 4.56 | |
| 9.33 | 5.63 | | 5.63 | 5.63 | 4.00 | 4.00 | 4.00 | |
| 10.00 | 4.98 | | 4.98 | 4.98 | 3.53 | 3.53 | 3.53 | |
| **EXPOSURE** | | | **B** | | **B** | **B** | **C** | **C** | **C** | |
| **Vasd as determined in  accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | | **32** | **40** | **24** | **32** | **40** | |
| **120** | | 8.00 | 5.64 | | 5.15 | 4.73 | 3.84 | 3.49 | 3.20 | |
| 8.67 | 5.38 | | 5.38 | 5.38 | 3.82 | 3.82 | 3.82 | |
| 9.33 | 4.72 | | 4.72 | 4.72 | 3.35 | 3.35 | 3.35 | |
| 10.00 | 4.17 | | 4.17 | 4.17 | 2.95 | 2.95 | 2.95 | |
| **EXPOSURE** | | | **B** | | **B** | **B** | **C** | **C** | **C** | |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | | **32** | **40** | **24** | **32** | **40** | |
| **130** | | 8.00 | 4.69 | | 4.27 | 3.92 | 3.21 | 2.92 | 2.67 | |
| 8.67 | 4.19 | | 3.86 | 3.57 | 2.88 | 2.64 | 2.44 | |
| 9.33 | 3.77 | | 3.49 | 3.25 | 2.60 | 2.40 | 2.23 | |
| 10.00 | 3.39 | | 3.17 | 2.97 | 2.34 | 2.18 | 2.04 | |
| **EXPOSURE** | | | **B** | | **B** | **B** | **C** | **C** | **C** | |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | | **32** | **40** | **24** | **32** | **40** | |
| **140** | | 8.00 | 3.97 | | 3.61 | 3.31 | 2.73 | 2.46 | 2.27 | |
| 8.67 | 3.55 | | 3.26 | 3.01 | 2.45 | 2.25 | 2.07 | |
| 9.33 | 3.20 | | 2.96 | 2.75 | 2.21 | 2.04 | 1.90 | |
| 10.00 | 2.88 | | 2.69 | 2.52 | 1.99 | 1.86 | 1.74 | |
| **EXPOSURE** | | | **B** | | **B** | **B** | **C** | **C** | **C** | |
| **Vasd as determined in  accordance with Section R301.2.1.3** | | **WALL HEIGHT** | **BUILDING WIDTH** | | | | | | | |
| **24** | | **32** | **40** | **24** | **32** | **40** | |
| **150** | | 8.00 | 3.40 | | 3.09 | 2.83 | 2.35 | 2.13 | 1.95 | |
| 8.67 | 3.05 | | 2.80 | 2.58 | 2.11 | 1.93 | 1.78 | |
| 9.33 | 2.75 | | 2.54 | 2.36 | 1.90 | 1.76 | 1.63 | |
| 10.00 | 2.48 | | 2.31 | 2.16 | 1.72 | 1.60 | 1.50 | |

**(S6134GlitchAS)**

**TABLE R609.3.3B-1 GRADE 60**

**MAXIMUM SPACING OF NO. 5 (5/8 IN.) VERTICAL REINFORCEMENT AT CONTINUOUS CONCRETE MASONRY LOWER STORIES OF MULTISTORY AND GABLE ENDS SINGLE STORY OR TOP STORY OF MULTISTORY, FEET**

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **100** | | **110** | | **120** | | **130** | | **140** | | **150** | |
| **EXP** | **WALL HT** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** |
| **B** | 8 | 9.87 | 10.53 | 8.97 | 9.57 | 8.23 | 8.78 | 7.59 | 8.10 | 7.05 | 7.52 | 6.58 | 7.02 |
| 8.67 | 9.97 | 10.59 | 9.06 | 9.63 | 8.30 | 8.82 | 7.67 | 8.15 | 7.12 | 7.56 | 6.64 | 7.06 |
| 9.33 | 10.06 | 10.64 | 9.14 | 9.68 | 8.38 | 8.87 | 7.73 | 8.19 | 7.18 | 7.60 | 6.70 | 7.10 |
| 10 | 10.14 | 10.69 | 9.22 | 9.72 | 8.45 | 8.91 | 7.80 | 8.23 | 7.08 | 7.64 | 6.14 | 6.85 |
| 12 | 10.17 | 10.83 | 8.38 | 9.14 | 7.01 | 7.65 | 5.95 | 6.49 | 5.10 | 5.57 | 4.42 | 4.83 |
| 14 | 7.75 | 8.29 | 6.37 | 6.82 | 5.33 | 5.70 | 4.51 | 4.83 | 3.86 | 4.14 | 3.34 | 3.58 |
| 16 | 6.12 | 6.43 | 5.03 | 5.29 | 4.19 | 4.41 | 3.55 | 3.73 | 3.03 | 3.19 | 2.62 | 2.76 |
| 18 | 4.97 | 5.14 | 4.07 | 4.21 | 3.39 | 3.51 | 2.86 | 2.96 | 2.44 | 2.53 | 2.10 | 2.18 |
| 20 | 4.12 | 4.20 | 3.38 | 3.44 | 2.81 | 2.86 | 2.36 | 2.41 | 2.01 | 2.05 | 1.72 | 1.76 |
| 22 | 3.48 | 3.49 | 2.84 | 2.85 | 2.36 | 2.36 | 1.98 | 1.99 | 1.68 | 1.68 | 1.43 | 1.44 |
| **C** | 8 | 8.34 | 8.89 | 7.58 | 8.09 | 6.95 | 7.41 | 6.41 | 6.84 | 5.95 | 6.35 | 5.56 | 5.93 |
| 8.67 | 8.42 | 8.94 | 7.65 | 8.13 | 7.01 | 7.45 | 6.47 | 6.88 | 6.01 | 6.39 | 5.24 | 5.96 |
| 9.33 | 8.49 | 8.99 | 7.72 | 8.17 | 7.08 | 7.49 | 6.52 | 6.91 | 5.53 | 6.37 | 4.75 | 5.53 |
| 10 | 8.57 | 9.03 | 7.79 | 8.21 | 6.87 | 7.53 | 5.83 | 6.50 | 5.00 | 5.58 | 4.31 | 4.84 |
| 12 | 7.21 | 7.86 | 5.92 | 6.47 | 4.95 | 5.40 | 4.19 | 4.58 | 3.59 | 3.92 | 3.10 | 3.39 |
| 14 | 5.47 | 5.86 | 4.49 | 4.81 | 3.75 | 4.01 | 3.16 | 3.39 | 2.70 | 2.90 | 2.33 | 2.50 |
| 16 | 4.31 | 4.54 | 3.53 | 3.72 | 2.94 | 3.09 | 2.47 | 2.61 | 2.11 | 2.22 | 1.81 | 1.91 |
| 18 | 3.49 | 3.61 | 2.85 | 2.95 | 2.37 | 2.45 | 1.99 | 2.06 | 1.68 | 1.75 | 1.44 | 1.49 |
| 20 | 2.89 | 2.94 | 2.35 | 2.40 | 1.94 | 1.98 | 1.63 | 1.66 | 1.37 | 1.40 | 1.16 | 1.19 |
| 22 | 2.43 | 2.43 | 1.97 | 1.98 | 1.62 | 1.63 | 1.35 | 1.35 | 1.13 | 1.14 | 1.03 | 1.26 |

**TABLE R609.3.3B-2 GRADE**

**60 MAXIMUM SPACING OF NO. 4 (1/2 IN.) VERTICAL REINFORCEMENT AT CONTINUOUS MASONRY LOWER STORIES OF MULTISTORY AND GABLE ENDS SINGLE STORY OR TOP STORY OF MULTISTORY, FEET**

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **Vasd as determined in accordance with Section R301.2.1.3** | | **100** | | **110** | | **120** | | **130** | | **140** | | **150** | |
| **EXP** | **WALL HT** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** |
| **B** | 8 | 9.87 | 10.53 | 8.97 | 9.57 | 8.23 | 8.78 | 7.59 | 8.10 | 6.42 | 7.52 | 5.47 | 6.68 |
| 8.67 | 9.97 | 10.59 | 9.06 | 9.63 | 8.07 | 8.82 | 6.86 | 7.76 | 5.77 | 6.68 | 4.93 | 5.80 |
| 9.33 | 10.06 | 10.64 | 8.45 | 9.48 | 7.08 | 7.95 | 6.02 | 6.76 | 5.17 | 5.81 | 4.46 | 5.05 |
| 10 | 9.06 | 10.09 | 7.47 | 8.32 | 6.26 | 6.97 | 5.32 | 5.92 | 4.57 | 5.09 | 3.96 | 4.42 |
| 12 | 6.56 | 7.16 | 5.40 | 5.89 | 4.52 | 4.93 | 3.84 | 4.19 | 3.29 | 3.59 | 2.85 | 3.12 |
| 14 | 5.00 | 5.35 | 4.11 | 4.40 | 3.44 | 3.68 | 2.91 | 3.12 | 2.49 | 2.67 | 2.16 | 2.31 |
| 16 | 3.95 | 4.15 | 3.24 | 3.41 | 2.71 | 2.85 | 2.29 | 2.41 | 1.96 | 2.06 | 1.69 | 1.78 |
| 18 | 3.21 | 3.31 | 2.63 | 2.72 | 2.19 | 2.27 | 1.85 | 1.91 | 1.58 | 1.63 | 1.36 | 1.41 |
| 20 | 2.66 | 2.71 | 2.18 | 2.22 | 1.81 | 1.84 | 1.52 | 1.55 | 1.30 | 1.32 | 1.11 | 1.13 |
| 22 | 2.25 | 2.25 | 1.83 | 1.84 | 1.52 | 1.53 | 1.28 | 1.28 | 1.08 | 1.09 | 0.93 | 0.93 |
| **C** | 8 | 8.34 | 8.89 | 7.58 | 8.09 | 6.20 | 7.41 | 5.15 | 6.29 | 4.36 | 5.30 | 3.73 | 4.53 |
| 8.67 | 8.29 | 8.94 | 6.83 | 7.73 | 5.58 | 6.48 | 4.65 | 5.50 | 3.94 | 4.73 | 3.38 | 4.09 |
| 9.33 | 7.28 | 8.17 | 5.99 | 6.73 | 5.02 | 5.64 | 4.21 | 4.79 | 3.57 | 4.11 | 3.06 | 3.57 |
| 10 | 6.43 | 7.16 | 5.30 | 5.90 | 4.43 | 4.94 | 3.76 | 4.19 | 3.22 | 3.60 | 2.78 | 3.12 |
| 12 | 4.65 | 5.07 | 3.82 | 4.17 | 3.19 | 3.49 | 2.70 | 2.95 | 2.31 | 2.53 | 2.00 | 2.19 |
| 14 | 3.53 | 3.78 | 2.90 | 3.10 | 2.42 | 2.59 | 2.04 | 2.19 | 1.74 | 1.87 | 1.50 | 1.61 |
| 16 | 2.78 | 2.93 | 2.28 | 2.40 | 1.90 | 2.00 | 1.60 | 1.68 | 1.36 | 1.43 | 1.17 | 1.23 |
| 18 | 2.25 | 2.33 | 1.84 | 1.90 | 1.53 | 1.58 | 1.28 | 1.33 | 1.09 | 1.13 | 0.93 | 0.96 |
| 20 | 1.86 | 1.90 | 1.52 | 1.55 | 1.25 | 1.28 | 1.05 | 1.07 | 0.88 | 0.90 | 0.75 | 0.77 |
| 22 | 1.57 | 1.57 | 1.27 | 1.28 | 1.05 | 1.05 | 0.87 | 0.87 | 0.73 | 0.73 | 0.67 | 0.81 |

**TABLE R609.3.3B-3 GRADE 40**

**MAXIMUM SPACING OF NO. 5 (5/8 IN.) VERTICAL REINFORCEMENT AT CONTINUOUS CONCRETE MASONRY LOWER STORIES OF MULTISTORY AND GABLE ENDS SINGLE STORY OR TOP STORY OF MULTISTORY, FEET**

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **Vasd as determined in accordance with  Section R301.2.1.3** | | **100** | | **110** | | **120** | | **130** | | **140** | | **150** | |
| **EXP** | **WALL HT** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** |
| **B** | 8 | 9.87 | 10.53 | 8.97 | 9.57 | 8.23 | 8.78 | 7.59 | 8.10 | 6.63 | 7.52 | 5.65 | 6.91 |
| 8.67 | 9.97 | 10.59 | 9.06 | 9.63 | 8.30 | 8.82 | 7.09 | 8.02 | 5.97 | 6.90 | 5.09 | 5.99 |
| 9.33 | 10.06 | 10.64 | 8.73 | 9.68 | 7.32 | 8.21 | 6.22 | 6.98 | 5.35 | 6.00 | 4.61 | 5.21 |
| 10 | 9.36 | 10.42 | 7.72 | 8.60 | 6.47 | 7.20 | 5.49 | 6.12 | 4.72 | 5.26 | 4.10 | 4.57 |
| 12 | 6.78 | 7.39 | 5.58 | 6.09 | 4.67 | 5.10 | 3.96 | 4.33 | 3.40 | 3.71 | 2.95 | 3.22 |
| 14 | 5.16 | 5.53 | 4.25 | 4.55 | 3.55 | 3.80 | 3.01 | 3.22 | 2.58 | 2.76 | 2.23 | 2.39 |
| 16 | 4.08 | 4.29 | 3.35 | 3.52 | 2.80 | 2.94 | 2.36 | 2.49 | 2.02 | 2.13 | 1.74 | 1.84 |
| 18 | 3.31 | 3.43 | 2.72 | 2.81 | 2.26 | 2.34 | 1.91 | 1.98 | 1.63 | 1.69 | 1.40 | 1.45 |
| 20 | 2.75 | 2.80 | 2.25 | 2.29 | 1.87 | 1.90 | 1.57 | 1.60 | 1.34 | 1.36 | 1.15 | 1.17 |
| 22 | 2.32 | 2.33 | 1.90 | 1.90 | 1.57 | 1.58 | 1.32 | 1.32 | 1.12 | 1.12 | 0.00 | 0.00 |
| **C** | 8 | 8.34 | 8.89 | 7.58 | 8.09 | 6.40 | 7.41 | 5.32 | 6.50 | 4.50 | 5.47 | 3.86 | 4.68 |
| 8.67 | 8.42 | 8.94 | 7.06 | 7.99 | 5.77 | 6.69 | 4.80 | 5.69 | 4.07 | 4.89 | 3.49 | 4.22 |
| 9.33 | 7.52 | 8.44 | 6.19 | 6.95 | 5.19 | 5.82 | 4.35 | 4.95 | 3.69 | 4.25 | 3.17 | 3.68 |
| 10 | 6.65 | 7.40 | 5.47 | 6.10 | 4.58 | 5.10 | 3.88 | 4.33 | 3.33 | 3.72 | 2.87 | 3.22 |
| 12 | 4.80 | 5.24 | 3.95 | 4.31 | 3.30 | 3.60 | 2.79 | 3.05 | 2.39 | 2.61 | 2.07 | 2.26 |
| 14 | 3.65 | 3.91 | 3.00 | 3.21 | 2.50 | 2.68 | 2.11 | 2.26 | 1.80 | 1.93 | 1.55 | 1.67 |
| 16 | 2.88 | 3.02 | 2.35 | 2.48 | 1.96 | 2.06 | 1.65 | 1.74 | 1.40 | 1.48 | 1.21 | 1.27 |
| 18 | 2.33 | 2.41 | 1.90 | 1.97 | 1.58 | 1.63 | 1.32 | 1.37 | 1.12 | 1.16 | 0.00 | 0.00 |
| 20 | 1.92 | 1.96 | 1.57 | 1.60 | 1.30 | 1.32 | 1.08 | 1.10 | 0.00 | 0.00 | 0.00 | 0.00 |
| 22 | 1.62 | 1.62 | 1.31 | 1.32 | 1.08 | 1.09 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |

**TABLE R609.3.3B-4 GRADE 40**

**MAXIMUM SPACING OF NO. 4 (1/2 IN.) VERTICAL REINFORCEMENT AT CONTINUOUS CONCRETE OR MASONRY LOWER STORIES OF MULTISTORY AND GABLE ENDS SINGLE STORY OR TOP STORY OF MULTISTORY, FEET**

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **Vasd as determined in accordance with  Section R301.2.1.3** | | **100** | | **110** | | **120** | | **130** | | **140** | | **150** | |
| **EXP** | **WALL HT** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** | **END ZONE** | **INT ZONE** |
| **B** | 8 | 8.89 | 10.23 | 7.41 | 8.44 | 6.21 | 7.08 | 5.10 | 6.02 | 4.28 | 5.18 | 3.64 | 4.46 |
| 8.67 | 7.78 | 8.80 | 6.42 | 7.26 | 5.38 | 6.09 | 4.57 | 5.17 | 3.85 | 4.45 | 3.29 | 3.87 |
| 9.33 | 6.83 | 7.66 | 5.63 | 6.32 | 4.72 | 5.30 | 4.01 | 4.50 | 3.45 | 3.87 | 2.97 | 3.36 |
| 10 | 6.04 | 6.73 | 4.98 | 5.55 | 4.17 | 4.65 | 3.54 | 3.95 | 3.05 | 3.39 | 2.64 | 2.95 |
| 12 | 4.38 | 4.77 | 3.60 | 3.93 | 3.02 | 3.29 | 2.56 | 2.79 | 2.19 | 2.40 | 1.90 | 2.08 |
| 14 | 3.33 | 3.56 | 2.74 | 2.93 | 2.29 | 2.45 | 1.94 | 2.08 | 1.66 | 1.78 | 1.44 | 1.54 |
| 16 | 2.63 | 2.77 | 2.16 | 2.27 | 1.80 | 1.90 | 1.53 | 1.61 | 1.30 | 1.37 | 1.13 | 1.19 |
| 18 | 2.14 | 2.21 | 1.75 | 1.81 | 1.46 | 1.51 | 1.23 | 1.27 | 1.05 | 1.09 | 0.90 | 0.94 |
| 20 | 1.77 | 1.81 | 1.45 | 1.48 | 1.21 | 1.23 | 1.02 | 1.03 | 0.86 | 0.88 | 0.74 | 0.76 |
| 22 | 1.50 | 1.50 | 1.22 | 1.23 | 1.01 | 1.02 | 0.85 | 0.85 | 0.72 | 0.72 | 0.00 | 0.00 |
| **C** | 8 | 6.38 | 7.28 | 5.08 | 6.00 | 4.13 | 5.03 | 3.43 | 4.19 | 2.90 | 3.53 | 2.49 | 3.02 |
| 8.67 | 5.53 | 6.25 | 4.56 | 5.15 | 3.72 | 4.32 | 3.10 | 3.67 | 2.62 | 3.15 | 2.25 | 2.72 |
| 9.33 | 4.85 | 5.44 | 4.00 | 4.49 | 3.35 | 3.76 | 2.80 | 3.19 | 2.38 | 2.74 | 2.04 | 2.38 |
| 10 | 4.29 | 4.78 | 3.53 | 3.93 | 2.95 | 3.29 | 2.51 | 2.79 | 2.15 | 2.40 | 1.85 | 2.08 |
| 12 | 3.10 | 3.38 | 2.55 | 2.78 | 2.13 | 2.32 | 1.80 | 1.97 | 1.54 | 1.69 | 1.33 | 1.46 |
| 14 | 2.35 | 2.52 | 1.93 | 2.07 | 1.61 | 1.73 | 1.36 | 1.46 | 1.16 | 1.25 | 1.00 | 1.08 |
| 16 | 1.85 | 1.95 | 1.52 | 1.60 | 1.26 | 1.33 | 1.06 | 1.12 | 0.91 | 0.96 | 0.78 | 0.82 |
| 18 | 1.50 | 1.55 | 1.23 | 1.27 | 1.02 | 1.05 | 0.85 | 0.89 | 0.72 | 0.75 | 0.00 | 0.00 |
| 20 | 1.24 | 1.26 | 1.01 | 1.03 | 0.84 | 0.85 | 0.70 | 0.71 | 0.00 | 0.00 | 0.00 | 0.00 |
| 22 | 1.04 | 1.05 | 0.85 | 0.85 | 0.70 | 0.70 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |

**(S6135GlitchAS)**

**R609.3.4 Precast bond beams.** Vertical reinforcement used in conjunction with precast bond beams shall be spaced the same as for masonry bond beams. Reinforcement shall terminate in the precast beam as set forth in Section R606.9.8.

**R609.3.5 Duplication.** Reinforcing steel requirements shall not be additive. A single bar shall be permitted to satisfy multiple requirements.

**R609.3.6 Termination.** Vertical reinforcement shall terminate in footings and bond beams as set forth in Section R606.9.8.

**R609.4 Masonry gables**. Gable end walls of concrete or masonry shall be constructed full height to the roof line.

**Exception:** Gable end trusses or wood framed gable end walls in conformance with Tables R609.4A and R609.4B and Figure R609.4. Wood gable stud wall connectors shall be capable of resisting the vertical and horizontal loads of Table R609.4B as well as the uplift load stipulated at Figure R609.4. Where masonry gable end walls do not go to the roof, a bond beam complying with Section R609.2 shall be provided at the top of the masonry.

**TABLE R609.4A**

**WOOD GABLE BRACE NAILING**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
|  | **Vasd as determined in accordance  with Section R301.2.1.3** | **NAIL SIZE** | **RAKE HEIGHT** | | | | | |
| **12** | **14** | **16** | **18** | **20** | **22** |
| **EXPOSURE B** | 100 | 10d | 4 | 4 | 5 | 5 | 6 | 6 |
| 8d | 6 | 7 | 8 | 8 | 9 | 10 |
| 110 | 10d | 4 | 5 | 6 | 6 | 7 | 7 |
| 8d | 7 | 8 | 9 | 10 | 11 | 11 |
| 120 | 10d | 5 | 6 | 7 | 7 | 8 | 8 |
| 8d | 9 | 10 | 11 | 12 | 13 | 13 |
| 130 | 10d | 6 | 7 | 8 | 9 | 10 | 10 |
| 8d | 10 | 11 | 13 | 14 | 15 | 15 |
| 140 | 10d | 7 | 8 | 9 | 10 | 11 | 11 |
| 8d | 12 | 13 | 15 | 16 | 18 | 18 |
| 150 | 10d | 8 | 9 | 11 | 12 | 13 | 13 |
| 8d | 13 | 15 | 17 | 19 | 20 | 20 |
| **EXPOSURE C** | 100 | 10d | 5 | 6 | 7 | 7 | 8 | 8 |
| 8d | 8 | 9 | 11 | 12 | 13 | 13 |
| 110 | 10d | 6 | 7 | 8 | 9 | 10 | 10 |
| 8d | 10 | 11 | 13 | 14 | 15 | 15 |
| 120 | 10d | 7 | 8 | 9 | 10 | 11 | 11 |
| 8d | 12 | 14 | 15 | 17 | 18 | 18 |
| 130 | 10d | 9 | 10 | 11 | 12 | 13 | 13 |
| 8d | 14 | 16 | 18 | 20 | 22 | 22 |
| 140 | 10d | 10 | 12 | 13 | 14 | 15 | 15 |
| 8d | 16 | 19 | 21 | 23 | 25 | 25 |
| 150 | 10d | 12 | 13 | 15 | 16 | 18 | 18 |
| 8d | 19 | 21 | 24 | 26 | 29 | 29 |

**(S6136GlitchAS)**

**TABLE R609.4B**

**WOOD GABLE STUD CONNECTOR LOADS**

|  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- |
|  | **Vasd as  determined in  accordance  with Section R301.2.1.3** | **ROOF  ZONE** | **CONNECTOR LOADS (Allowable Stress Design)** | | **WALL ZONE** |
| **VERT** | **HORIZ1** |
| **EXPOSURE B** | 100 | 2E | 43 | 16 | 1E |
| 2E | 43 | 11 | 1 |
| 110 | 2E | 53 | 20 | 1E |
| 2E | 53 | 13 | 1 |
| 120 | 2E | 62 | 23 | 1E |
| 2E | 62 | 15 | 1 |
| 130 | 2E | 73 | 27 | 1E |
| 2E | 73 | 18 | 1 |
| 140 | 2E | 85 | 32 | 1E |
| 2E | 85 | 21 | 1 |
| 150 | 2E | 98 | 36 | 1E |
| 2E | 98 | 24 | 1 |
| **EXPOSURE C** | 100 | 2E | 61 | 23 | 1E |
| 2E | 61 | 15 | 1 |
| 110 | 2E | 74 | 28 | 1E |
| 2E | 74 | 18 | 1 |
| 120 | 2E | 88 | 33 | 1E |
| 2E | 88 | 22 | 1 |
| 130 | 2E | 103 | 38 | 1E |
| 2E | 103 | 25 | 1 |
| 140 | 2E | 119 | 45 | 1E |
| 2E | 119 | 30 | 1 |
| 150 | 2E | 137 | 51 | 1E |
| 2E | 137 | 34 | 1 |
| 1. Unit load on stud. Multiply by 1/2 stud length plus 1/2 wall height for total connector load. | | | | |

**(S6137GlitchAS)**

|  |
| --- |
|  |

|  |
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|  |

**FIGURE R609.4**

**GABLE END BRACING FOR MASONRY WALLS NOT CONTINUOUS TO THE ROOF DIAPHRAGM**

**(S6139GlitchAS)**

**R609.4.1 Rake beam.** Where concrete or masonry is carried full height to the roof line, a cast-in-place rake beam as detailed in Figure R609.4.1 shall be provided. The minimum thickness of the rake beam from top of masonry shall be 4 inches (102 mm). One No. 5 continuous reinforcing bar shall be placed in the rake beam along the roof line.

|  |
| --- |
|  |

**FIGURE R609.4.1**

**CONTINUOUS GABLE ENDWALL REINFORCEMENT ONE AND MULTISTORY**

**R609.4.2 Vertical reinforcement.** Vertical reinforcement shall be provided at the maximum spacing as set forth in Tables R609.3.3B-1 through R609.3.3.B-4 as applicable.

**R609.4.3 Termination.** Required vertical reinforcement shall terminate at the rake beam in accordance with Section R606.9.8.

**R609.4.4 Nailer**. A minimum 2 feet 4 inch nailer for connecting roof sheathing shall be bolted to the top of the wall with a minimum of 1/2 inch (12.7 mm) anchor bolts spaced as set forth in Table R609.4.4. The nailer shall be permitted to be bolted to the inside or outside of the wall.

**R609.4 .5 Gable overhang.** Gable overhangs up to 2 feet (610 mm) in width complying with Figure R609.4.5 shall be permitted.

**TABLE R609.4.4**

**ANCHOR BOLT SPACING FOR ATTACHING 2 × 4 MINIMUM WOOD NAILER TO RAKE BEAM**

|  |  |
| --- | --- |
| **REQUIRED ROOF DIAPHRAGM CAPACITY (ALLOWABLE STRESS DESIGN)** | **1/2-INCH ANCHOR BOLT  MAXIMUM SPACING** |
| ≤105 | 6′ - 0″ |
| 145 | 5′ - 0″ |
| 195 | 4′ - 0″ |
| 230 | 3′ - 6″ |
| 270 | 3′ - 0″ |
| 325 | 2′ - 6″ |
| 415 | 2′ - 0″ |
| 565 | 1′ - 6″ |
| 700 | 1′ - 2″ |
| 845 | 1′ - 0″ |
| **R609.4.5 Ga** | |

**FIGURE R609.4.5 GABLE OVERHANG**

**R609.5 Exterior shearwalls.** Each exterior wall shall have the required length of effective shearwall to resist horizontal movement or forces at the ends of diaphragms in conformance with this section.

**R609.5.1 Shearwall lengths.** The required shearwall segment length shall be as set forth in Tables R609.5.1A through R609.5.1R as applicable. **(S6140GlitchAS)**

**R609.5.2 Multistory shearwalls.** Shearwall segments in an upper story shall be located directly over and within the length of shearwall segments in the story below. Reinforcement at the ends of shearwall segments shall be continuous from the bond beam of the upper story through the story below.

**Exception:** Offsetting of vertical reinforcement as set forth in Section R606.13.9.1 shall be permitted.

**R609.5.3** The connector load for total shear at the top story wall shall be determined in accordance with Table R609.5.3A and Figure R609.5.3. Transverse connector loads shall be in accordance with Table R609.5.3B and Figure R609.5.3.

**R609.5.4** Endwall roof shear loads shall be in accordance with Table R609.5.4.

**TABLE R609.5.1A GRADE 60**

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO. 4 REINFORCEMENT1,2,3,5** **ROOF ANGLE ≤ 23°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance  with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 1.62 | 2.31 | 3.14 | 3.68 | 4.98 | 6.49 | 5.41 | 7.28 | 9.43 |
| 110 | 1.96 | 2.79 | 3.80 | 4.45 | 6.03 | 7.85 | 6.55 | 8.81 | 11.42 |
| 120 | 2.33 | 3.32 | 4.52 | 5.30 | 7.17 | 9.34 | 7.79 | 10.48 | 13.58 |
| 130 | 2.73 | 3.90 | 5.31 | 6.22 | 8.42 | 10.96 | 9.14 | 12.30 | 15.94 |
| 140 | 3.17 | 4.52 | 6.16 | 7.22 | 9.376 | 12.71 | 10.60 | 14.27 | 18.49 |
| 150 | 3.64 | 5.19 | 7.07 | 8.28 | 11.21 | 14.59 | 12.17 | 16.38 | 21.23 |
| **C** | 100 | 1.92 | 2.77 | 3.82 | 4.86 | 6.63 | 8.69 | 7.56 | 10.15 | 13.13 |
| 110 | 2.32 | 3.35 | 4.62 | 5.88 | 8.02 | 10.52 | 9.14 | 12.28 | 15.88 |
| 120 | 2.76 | 3.99 | 5.50 | 7.00 | 9.54 | 12.52 | 10.88 | 14.61 | 18.90 |
| 130 | 3.24 | 4.68 | 6.46 | 8.21 | 11.20 | 14.69 | 12.77 | 17.15 | 22.18 |
| 140 | 3.76 | 5.43 | 7.49 | 9.53 | 12.99 | 17.04 | 14.81 | 19.89 | 25.73 |
| 150 | 4.32 | 6.23 | 8.59 | 10.94 | 14.91 | 19.56 | 17.00 | 22.83 | 29.53 |

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO. 5 REINFORCEMENT1,2,3,6** **ROOF ANGLE ≤ 23°**

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | | **Vasd as determined in  accordance  with Section  R301.2.1.3** | **TOP STORY** | | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | | |
| **BUILDING WIDTH** | | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | |
| **24** | **32** | **40** | | **24** | **32** | **40** | **24** | | **32** | **40** |
| **B** | | 100 | 1.09 | 1.55 | 2.12 | | 2.48 | 3.35 | 4.37 | 3.64 | | 4.90 | 6.35 |
| 110 | 1.32 | 1.88 | 2.56 | | 3.00 | 4.06 | 5.28 | 4.41 | | 5.93 | 7.68 |
| 120 | 1.57 | 2.23 | 3.05 | | 3.57 | 4.83 | 6.29 | 5.24 | | 7.06 | 9.14 |
| 130 | 1.84 | 2.62 | 3.57 | | 4.19 | 5.67 | 7.38 | 6.16 | | 8.28 | 10.73 |
| 140 | 2.13 | 3.04 | 4.15 | | 4.86 | 6.57 | 8.56 | 7.14 | | 9.60 | 12.45 |
| 150 | 2.45 | 3.49 | 4.76 | | 5.58 | 7.54 | 9.82 | 8.19 | | 11.02 | 14.29 |
| **C** | | 100 | 1.29 | 1.86 | 2.57 | | 3.27 | 4.46 | 5.85 | 5.09 | | 6.83 | 8.84 |
| 110 | 1.56 | 2.26 | 3.11 | | 3.96 | 5.40 | 7.08 | 6.16 | | 8.26 | 10.69 |
| 120 | 1.86 | 2.68 | 3.70 | | 4.71 | 6.42 | 8.43 | 7.33 | | 9.84 | 12.72 |
| 130 | 2.18 | 3.15 | 4.35 | | 5.53 | 7.54 | 9.89 | 8.60 | | 11.54 | 14.93 |
| 140 | 2.53 | 3.65 | 5.04 | | 6.41 | 8.74 | 11.47 | 9.97 | | 13.39 | 17.32 |
| 150 | 2.91 | 4.19 | 5.79 | | 7.36 | 10.04 | 13.17 | 11.45 | | 15.37 | 19.88 |
| **Notes:** | | | | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not one continuous shear wall segment, the total shear wall length required shall be increased by 0.67 feet (8 inches) for each additional shear wall segment making up the total shear wall length on a side. | | | | | | | | | | | | | |
| 2. The minimum shear wall segment length shall be 2 feet-0 inches. Values less than 2 feet-0 inches are shown only for summation of shear wall segments and for interpolation purposes. A grouted cell with vertical reinforcement of the size indicated is required at each end of every shear wall segment. | | | | | | | | | | | | | |
| 3. Other than incidental utility penetrations, shearwall piers and shearwall segments shall not contain openings with a maximum horizontal or vertical dimension of 5 inches for piers and 12 inches for portions of shearwall segments above and below piers. The total area of openings in any one segment of shearwall shall not exceed 144 square inches. | | | | | | | | | | | | | |
| 4. Required shearwall lengths normal (perpendicular) to the ridge are per lineal foot of building length. Multiply tabular values by building length distance between adjacent shear walls perpendicular to the ridge if interior shear walls are used for total shear wall length required. | | | | | | | | | | | | | |
| 5. Shear wall lengths are based on shear wall segment heights of 80 inches (height from the floor to the top of the highest opening adjacent to the shear segment—corners and openings as permitted by Note 3 of this table are not considered for the purpose of this measurement). For shear segment heights other than 80 inches, multiply tabular length values as follows: | | | | | | | | | | | | | |
| **Segment Height (inches)** | | | | | **Length Multiplier** | | | | |
| 88 | | | | | 1.09 | | | | |
| 96 | | | | | 1.19 | | | | |
| 104 | | | | | 1.28 | | | | |
| 112 | | | | | 1.37 | | | | |

**(S6183GlitchAM , S6229GlitchAS)**

**TABLE R609.5.1B GRADE 60**

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO. 4 REINFORCEMENT1,2,3,5** **ROOF ANGLE 30°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in  accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 1.66 | 2.45 | 3.40 | 3.55 | 4.92 | 6.49 | 5.18 | 7.13 | 9.36 |
| 110 | 2.01 | 2.96 | 4.11 | 4.30 | 5.96 | 7.86 | 6.27 | 8.63 | 11.32 |
| 120 | 2.39 | 3.52 | 4.89 | 5.12 | 7.09 | 9.35 | 7.46 | 10.27 | 13.47 |
| 130 | 2.81 | 4.13 | 5.74 | 6.00 | 8.32 | 10.97 | 8.76 | 12.05 | 15.81 |
| 140 | 3.26 | 4.79 | 6.66 | 6.96 | 9.65 | 12.73 | 10.15 | 13.98 | 18.34 |
| 150 | 3.74 | 5.50 | 7.64 | 7.99 | 11.08 | 14.61 | 11.66 | 16.05 | 21.05 |
| **C** | 100 | 2.00 | 2.99 | 4.21 | 4.73 | 6.62 | 8.82 | 7.22 | 9.92 | 12.97 |
| 110 | 2.42 | 3.62 | 5.10 | 5.72 | 8.01 | 10.67 | 8.74 | 12.00 | 15.70 |
| 120 | 2.88 | 4.30 | 6.07 | 6.81 | 9.53 | 12.70 | 10.40 | 14.28 | 18.68 |
| 130 | 3.37 | 5.05 | 7.12 | 7.99 | 11.19 | 14.90 | 12.20 | 16.76 | 21.92 |
| 140 | 3.91 | 5.86 | 8.26 | 9.27 | 12.98 | 17.28 | 14.15 | 19.43 | 25.43 |
| 150 | 4.49 | 6.73 | 9.48 | 10.64 | 14.90 | 19.84 | 16.25 | 22.31 | 29.19 |

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO. 5 REINFORCEMENT1,2,3,6 ROOF ANGLE 30°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined  in accordance with Section  R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 1.12 | 1.65 | 2.29 | 2.39 | 3.31 | 4.37 | 3.49 | 4.80 | 6.30 |
| 110 | 1.35 | 1.99 | 2.77 | 2.89 | 4.01 | 5.29 | 4.22 | 5.81 | 7.62 |
| 120 | 1.61 | 2.37 | 3.29 | 3.44 | 4.77 | 6.29 | 5.02 | 6.91 | 9.07 |
| 130 | 1.89 | 2.78 | 3.86 | 4.04 | 5.60 | 7.39 | 5.89 | 8.11 | 10.64 |
| 140 | 2.19 | 3.23 | 4.48 | 4.69 | 6.50 | 8.57 | 6.84 | 9.41 | 12.34 |
| 150 | 2.52 | 3.71 | 5.14 | 5.38 | 7.46 | 9.83 | 7.85 | 10.80 | 14.17 |
| **C** | 100 | 1.34 | 2.01 | 2.84 | 3.18 | 4.46 | 5.93 | 4.86 | 6.67 | 8.73 |
| 110 | 1.63 | 2.44 | 3.43 | 3.85 | 5.39 | 7.18 | 5.88 | 8.08 | 10.57 |
| 120 | 1.94 | 2.90 | 4.08 | 4.58 | 6.42 | 8.55 | 7.00 | 9.61 | 12.57 |
| 130 | 2.27 | 3.40 | 4.79 | 5.38 | 7.53 | 10.03 | 8.21 | 11.28 | 14.76 |
| 140 | 2.63 | 3.94 | 5.56 | 6.24 | 8.74 | 11.63 | 9.53 | 13.08 | 17.12 |
| 150 | 3.02 | 4.53 | 6.38 | 7.16 | 10.03 | 13.35 | 10.94 | 15.02 | 19.65 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not one continuous shear wall segment, the total shear wall length required shall be increased by 0.67 feet (8 inches) for each additional shear wall segment making up the total shear wall length on a side. | | | | | | | | | | |
| 2. The minimum shear wall segment length shall be 2 feet-0 inches. Values less than 2 feet-0 inches are shown only for summation of shear wall segments and for interpolation purposes. A grouted cell with vertical reinforcement of the size indicated is required at each end of every shear wall segment. | | | | | | | | | | |
| 3. Other than incidental utility penetrations, shearwall piers and shearwall segments shall not contain openings with a maximum horizontal or vertical dimension of 5 inches for piers and 12 inches for portions of shearwall segments above and below piers. The total area of openings in any one segment of shearwall shall not exceed 144 square inches. | | | | | | | | | | |
| 4. Required shearwall lengths normal (perpendicular) to the ridge are per lineal foot of building length. Multiply tabular values by building length distance between adjacent shear walls perpendicular to the ridge if interior shear walls are used for total shear wall length required. | | | | | | | | | | |
| 5. Shear wall lengths are based on shear wall segment heights of 80 inches (height from the floor to the top of the highest opening adjacent to the shear segmentâ€"corners and openings as permitted by Note 3 of this table are not considered for the purpose of this measurement). For shear segment heights other than 80 inches, multiply tabular length values as follows:   |  |  | | --- | --- | | **Segment Height (inches)** | **Length Multiplier** | | 88 | 1.0 | | 96 | 1.19 | | 104 | 1.2 | | 112 | 1.37 | | | | | | | | | | | |

**(S6183GlitchAM, S6229GlitchAS)**

**TABLE R609.5.1C GRADE 60**

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO. 4 REINFORCEMENT1,2,3,5** **ROOF ANGLE 45°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in accordance  with Section  R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 2.11 | 3.24 | 4.64 | 2.11 | 5.72 | 7.73 | 5.77 | 8.17 | 11.00 |
| 110 | 2.56 | 3.92 | 5.61 | 4.85 | 6.92 | 9.36 | 6.98 | 9.89 | 13.31 |
| 120 | 3.05 | 4.67 | 6.68 | 5.77 | 8.23 | 11.13 | 8.31 | 11.77 | 15.85 |
| 130 | 3.57 | 5.48 | 7.84 | 6.77 | 9.66 | 13.07 | 9.75 | 13.81 | 18.60 |
| 140 | 4.15 | 6.35 | 9.09 | 7.85 | 11.20 | 15.16 | 11.31 | 16.02 | 21.57 |
| 150 | 4.76 | 7.29 | 10.43 | 9.01 | 12.86 | 17.40 | 12.98 | 18.39 | 24.76 |
| **C** | 100 | 2.63 | 4.13 | 6.03 | 5.45 | 7.90 | 10.84 | 7.22 | 9.92 | 12.97 |
| 110 | 3.18 | 4.99 | 7.30 | 6.60 | 9.56 | 13.12 | 8.74 | 12.00 | 15.70 |
| 120 | 3.79 | 5.94 | 8.68 | 7.85 | 11.38 | 15.61 | 10.40 | 14.28 | 18.68 |
| 130 | 4.45 | 6.97 | 10.19 | 9.21 | 13.35 | 18.32 | 12.20 | 16.76 | 21.92 |
| 140 | 5.16 | 8.09 | 11.82 | 10.68 | 15.48 | 21.25 | 14.15 | 19.43 | 25.43 |
| 150 | 5.92 | 9.28 | 13.57 | 12.26 | 17.77 | 24.39 | 16.25 | 22.31 | 29.19 |

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO. 5 REINFORCEMENT1,2,3,6 ROOF ANGLE 45°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined  in accordance  with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 1.42 | 2.18 | 3.12 | 2.70 | 3.85 | 5.21 | 3.88 | 5.50 | 7.41 |
| 110 | 1.72 | 2.64 | 3.78 | 3.26 | 4.66 | 6.30 | 4.70 | 6.66 | 8.96 |
| 120 | 2.05 | 3.14 | 4.49 | 3.88 | 5.54 | 7.50 | 5.59 | 7.92 | 10.67 |
| 130 | 2.41 | 3.69 | 5.27 | 4.56 | 6.50 | 8.80 | 6.56 | 9.30 | 12.52 |
| 140 | 2.79 | 4.27 | 6.12 | 5.29 | 7.54 | 10.20 | 7.61 | 10.78 | 14.52 |
| 150 | 3.20 | 4.91 | 7.02 | 6.07 | 8.66 | 11.71 | 8.74 | 12.38 | 16.67 |
| **C** | 100 | 1.77 | 2.78 | 4.06 | 3.67 | 5.32 | 7.30 | 5.38 | 7.59 | 10.18 |
| 110 | 2.14 | 3.36 | 4.91 | 4.44 | 6.43 | 8.83 | 6.51 | 9.19 | 12.32 |
| 120 | 2.55 | 4.00 | 5.85 | 5.28 | 7.66 | 10.51 | 7.75 | 10.93 | 14.66 |
| 130 | 2.99 | 4.69 | 6.86 | 6.20 | 8.99 | 12.33 | 9.09 | 12.83 | 17.21 |
| 140 | 3.47 | 5.44 | 7.96 | 7.19 | 10.42 | 14.30 | 10.55 | 14.88 | 19.96 |
| 150 | 3.98 | 6.25 | 9.13 | 8.26 | 11.96 | 16.42 | 12.11 | 17.09 | 22.91 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not one continuous shear wall segment, the total shear wall length required shall be increased by 0.67 feet (8 inches) for each additional shear wall segment making up the total shear wall length on a side. | | | | | | | | | | |
| 2. The minimum shear wall segment length shall be 2 feet-0 inches. Values less than 2 feet-0 inches are shown only for summation of shear wall segments and for interpolation purposes. A grouted cell with vertical reinforcement of the size indicated is required at each end of every shear wall segment. | | | | | | | | | | |
| 3. Other than incidental utility penetrations, shearwall piers and shearwall segments shall not contain openings with a maximum horizontal or vertical dimension of 5 inches for piers and 12 inches for portions of shearwall segments above and below piers. The total area of openings in any one segment of shearwall shall not exceed 144 square inches. | | | | | | | | | | |
| 4. Required shearwall lengths normal (perpendicular) to the ridge are per lineal foot of building length. Multiply tabular values by building length distance between adjacent shear walls perpendicular to the ridge if interior shear walls are used for total shear wall length required. | | | | | | | | | | |
| 5. Shear wall lengths are based on shear wall segment heights of 80 inches (height from the floor to the top of the highest opening adjacent to the shear segmentâ€"corners and openings as permitted by Note 3 of this table are not considered for the purpose of this measurement). For shear segment heights other than 80 inches, multiply tabular length values as follows:   |  |  | | --- | --- | | **Segment Height (inches)** | **Length Multiplier** | | 88 | 1.09 | | 96 | 1.19 | | 104 | 1.28 | | 112 | 1.37 | | | | | | | | | | | |

**(S6183GlitchhAM, S6229GlitchAS)**

**TABLE R609.5.1D GRADE 60**

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO. 4 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,5** **ROOF ANGLE 23°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined  in  accordance  with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.054 | 0.053 | 0.053 | 0.142 | 0.142 | 0.141 | 0.231 | 0.231 | 0.230 |
| 110 | 0.065 | 0.065 | 0.064 | 0.172 | 0.172 | 0.171 | 0.279 | 0.279 | 0.278 |
| 120 | 0.077 | 0.077 | 0.076 | 0.205 | 0.204 | 0.204 | 0.333 | 0.332 | 0.331 |
| 130 | 0.091 | 0.090 | 0.089 | 0.240 | 0.240 | 0.239 | 0.390 | 0.390 | 0.389 |
| 140 | 0.105 | 0.104 | 0.103 | 0.279 | 0.278 | 0.277 | 0.453 | 0.452 | 0.451 |
| 150 | 0.121 | 0.120 | 0.119 | 0.320 | 0.319 | 0.318 | 0.520 | 0.519 | 0.518 |
| **C** | 100 | 0.075 | 0.075 | 0.074 | 0.199 | 0.199 | 0.198 | 0.324 | 0.323 | 0.322 |
| 110 | 0.091 | 0.090 | 0.089 | 0.241 | 0.241 | 0.240 | 0.392 | 0.391 | 0.390 |
| 120 | 0.108 | 0.108 | 0.106 | 0.287 | 0.287 | 0.285 | 0.466 | 0.466 | 0.464 |
| 130 | 0.127 | 0.126 | 0.125 | 0.337 | 0.336 | 0.335 | 0.547 | 0.546 | 0.545 |
| 140 | 0.147 | 0.147 | 0.145 | 0.391 | 0.390 | 0.388 | 0.635 | 0.634 | 0.632 |
| 150 | 0.169 | 0.168 | 0.166 | 0.449 | 0.448 | 0.446 | 0.729 | 0.728 | 0.726 |

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO. 5 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,6 ROOF ANGLE 23°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance  with Section  R301.2.1.3** | **TOP STORY** | | | **1ST STORY PF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.036 | 0.036 | 0.035 | 0.096 | 0.096 | 0.095 | 0.155 | 0.155 | 0.155 |
| 110 | 0.044 | 0.043 | 0.043 | 0.116 | 0.116 | 0.115 | 0.188 | 0.188 | 0.187 |
| 120 | 0.052 | 0.052 | 0.051 | 0.138 | 0.138 | 0.137 | 0.224 | 0.224 | 0.223 |
| 130 | 0.061 | 0.061 | 0.060 | 0.162 | 0.162 | 0.161 | 0.263 | 0.262 | 0.262 |
| 140 | 0.071 | 0.070 | 0.070 | 0.188 | 0.187 | 0.186 | 0.305 | 0.304 | 0.303 |
| 150 | 0.081 | 0.081 | 0.080 | 0.215 | 0.215 | 0.214 | 0.350 | 0.349 | 0.348 |
| **C** | 100 | 0.051 | 0.050 | 0.050 | 0.134 | 0.134 | 0.133 | 0.218 | 0.218 | 0.217 |
| 110 | 0.061 | 0.061 | 0.060 | 0.162 | 0.162 | 0.161 | 0.264 | 0.263 | 0.263 |
| 120 | 0.073 | 0.072 | 0.072 | 0.193 | 0.193 | 0.192 | 0.314 | 0.313 | 0.313 |
| 130 | 0.086 | 0.085 | 0.084 | 0.227 | 0.226 | 0.225 | 0.368 | 0.368 | 0.367 |
| 140 | 0.099 | 0.099 | 0.097 | 0.263 | 0.263 | 0.261 | 0.427 | 0.427 | 0.425 |
| 150 | 0.114 | 0.113 | 0.112 | 0.302 | 0.301 | 0.300 | 0.490 | 0.490 | 0.488 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not one continuous shear wall segment, the total shear wall length required shall be increased by 0.67 feet (8 inches) for each additional shear wall segment making up the total shear wall length on a side. | | | | | | | | | | |
| 2. The minimum shear wall segment length shall be 2 feet-0 inches. Values less than 2 feet-0 inches are shown only for summation of shear wall segments and for interpolation purposes. A grouted cell with vertical reinforcement of the size indicated is required at each end of every shear wall segment. | | | | | | | | | | |
| 3. Other than incidental utility penetrations, shearwall piers and shearwall segments shall not contain openings with a maximum horizontal or vertical dimension of 5 inches for piers and 12 inches for portions of shearwall segments above and below piers. The total area of openings in any one segment of shearwall shall not exceed 144 square inches. | | | | | | | | | | |
| 4. Required shearwall lengths normal (perpendicular) to the ridge are per lineal foot of building length. Multiply tabular values by building length distance between adjacent shear walls perpendicular to the ridge if interior shear walls are used for total shear wall length required. | | | | | | | | | | |
| 5. Shear wall lengths are based on shear wall segment heights of 80 inches (height from the floor to the top of the highest opening adjacent to the shear segmentâ€"corners and openings as permitted by Note 3 of this table are not considered for the purpose of this measurement). For shear segment heights other than 80 inches, multiply tabular length values as follows:   |  |  | | --- | --- | | **Segment Height (inches)** | **Length Multiplier** | | 88 | 1.09 | | 96 | 1.19 | | 104 | 1.28 | | 112 | 1.37 | | | | | | | | | | | |

**(S6183GlitchAM, S6229GlitchAS)**

**TABLE R609.5.1E GRADE 60**

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO. 4 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,5 ROOF ANGLE 30°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance  with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.088 | 0.102 | 0.117 | 0.166 | 0.180 | 0.195 | 0.243 | 0.257 | 0.272 |
| 110 | 0.107 | 0.124 | 0.142 | 0.201 | 0.217 | 0.236 | 0.294 | 0.311 | 0.329 |
| 120 | 0.127 | 0.147 | 0.169 | 0.239 | 0.259 | 0.280 | 0.350 | 0.371 | 0.392 |
| 130 | 0.149 | 0.173 | 0.198 | 0.280 | 0.304 | 0.329 | 0.411 | 0.435 | 0.460 |
| 140 | 0.173 | 0.200 | 0.230 | 0.325 | 0.352 | 0.382 | 0.477 | 0.504 | 0.534 |
| 150 | 0.199 | 0.230 | 0.264 | 0.373 | 0.404 | 0.438 | 0.548 | 0.579 | 0.613 |
| **C** | 100 | 0.124 | 0.143 | 0.164 | 0.232 | 0.252 | 0.273 | 0.341 | 0.361 | 0.382 |
| 110 | 0.150 | 0.173 | 0.199 | 0.281 | 0.305 | 0.330 | 0.413 | 0.437 | 0.462 |
| 120 | 0.178 | 0.206 | 0.237 | 0.335 | 0.363 | 0.393 | 0.491 | 0.519 | 0.550 |
| 130 | 0.209 | 0.242 | 0.278 | 0.393 | 0.426 | 0.461 | 0.577 | 0.610 | 0.645 |
| 140 | 0.242 | 0.281 | 0.322 | 0.456 | 0.494 | 0.535 | 0.669 | 0.707 | 0.748 |
| 150 | 0.278 | 0.322 | 0.370 | 0.523 | 0.567 | 0.614 | 0.768 | 0.812 | 0.859 |

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO. 5 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,6 ROOF ANGLE 30°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.059 | 0.069 | 0.079 | 0.112 | 0.121 | 0.131 | 0.164 | 0.173 | 0.183 |
| 110 | 0.072 | 0.083 | 0.095 | 0.135 | 0.146 | 0.159 | 0.198 | 0.210 | 0.222 |
| 120 | 0.086 | 0.099 | 0.114 | 0.161 | 0.174 | 0.189 | 0.236 | 0.249 | 0.264 |
| 130 | 0.100 | 0.116 | 0.133 | 0.189 | 0.204 | 0.222 | 0.277 | 0.293 | 0.310 |
| 140 | 0.116 | 0.135 | 0.155 | 0.219 | 0.237 | 0.257 | 0.321 | 0.339 | 0.359 |
| 150 | 0.134 | 0.155 | 0.177 | 0.251 | 0.272 | 0.295 | 0.369 | 0.390 | 0.412 |
| **C** | 100 | 0.083 | 0.096 | 0.111 | 0.156 | 0.170 | 0.184 | 0.230 | 0.243 | 0.257 |
| 110 | 0.101 | 0.117 | 0.134 | 0.189 | 0.205 | 0.222 | 0.278 | 0.294 | 0.311 |
| 120 | 0.120 | 0.139 | 0.159 | 0.225 | 0.244 | 0.265 | 0.331 | 0.350 | 0.370 |
| 130 | 0.141 | 0.163 | 0.187 | 0.264 | 0.287 | 0.311 | 0.388 | 0.410 | 0.434 |
| 140 | 0.163 | 0.189 | 0.217 | 0.307 | 0.333 | 0.360 | 0.450 | 0.476 | 0.504 |
| 150 | 0.187 | 0.217 | 0.249 | 0.352 | 0.382 | 0.413 | 0.517 | 0.546 | 0.578 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not one continuous shear wall segment, the total shear wall length required shall be increased by 0.67 feet (8 inches) for each additional shear wall segment making up the total shear wall length on a side. | | | | | | | | | | |
| 2. The minimum shear wall segment length shall be 2 feet-0 inches. Values less than 2 feet-0 inches are shown only for summation of shear wall segments and for interpolation purposes. A grouted cell with vertical reinforcement of the size indicated is required at each end of every shear wall segment. | | | | | | | | | | |
| 3. Other than incidental utility penetrations, shearwall piers and shearwall segments shall not contain openings with a maximum horizontal or vertical dimension of 5 inches for piers and 12 inches for portions of shearwall segments above and below piers. The total area of openings in any one segment of shearwall shall not exceed 144 square inches. | | | | | | | | | | |
| 4. Required shearwall lengths normal (perpendicular) to the ridge are per lineal foot of building length. Multiply tabular values by building length distance between adjacent shear walls perpendicular to the ridge if interior shear walls are used for total shear wall length required. | | | | | | | | | | |
| 5. Shear wall lengths are based on shear wall segment heights of 80 inches (height from the floor to the top of the highest opening adjacent to the shear segmentâ€"corners and openings as permitted by Note 3 of this table are not considered for the purpose of this measurement). For shear segment heights other than 80 inches, multiply tabular length values as follows:   |  |  | | --- | --- | | **Segment Height (inches)** | **Length Multiplier** | | 88 | 1.09 | | 96 | 1.19 | | 104 | 1.28 | | 112 | 1.37 | | | | | | | | | | | |

**(S6183GlitchAM, S6229GlitchAS)**

**TABLE R609.5.1F GRADE 60**

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO.4 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,5** **ROOF ANGLE 45°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in accordance  with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.118 | 0.142 | 0.168 | 0.196 | 0.220 | 0.246 | 0.273 | 0.297 | 0.323 |
| 110 | 0.143 | 0.172 | 0.204 | 0.237 | 0.266 | 0.297 | 0.331 | 0.360 | 0.391 |
| 120 | 0.170 | 0.205 | 0.242 | 0.282 | 0.317 | 0.354 | 0.393 | 0.428 | 0.466 |
| 130 | 0.200 | 0.241 | 0.284 | 0.331 | 0.372 | 0.415 | 0.462 | 0.503 | 0.546 |
| 140 | 0.232 | 0.279 | 0.330 | 0.384 | 0.431 | 0.482 | 0.536 | 0.583 | 0.634 |
| 150 | 0.266 | 0.320 | 0.378 | 0.440 | 0.495 | 0.553 | 0.615 | 0.669 | 0.727 |
| **C** | 100 | 0.166 | 0.200 | 0.236 | 0.274 | 0.308 | 0.345 | 0.383 | 0.417 | 0.453 |
| 110 | 0.200 | 0.241 | 0.285 | 0.332 | 0.373 | 0.417 | 0.464 | 0.505 | 0.549 |
| 120 | 0.239 | 0.287 | 0.340 | 0.395 | 0.444 | 0.496 | 0.552 | 0.600 | 0.653 |
| 130 | 0.280 | 0.337 | 0.399 | 0.464 | 0.521 | 0.582 | 0.647 | 0.705 | 0.766 |
| 140 | 0.325 | 0.391 | 0.462 | 0.538 | 0.604 | 0.675 | 0.751 | 0.817 | 0.888 |
| 150 | 0.373 | 0.449 | 0.531 | 0.617 | 0.694 | 0.775 | 0.862 | 0.938 | 1.020 |

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO.5 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,6 ROOF ANGLE 45°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance  with Section  R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.080 | 0.096 | 0.113 | 0.132 | 0.148 | 0.165 | 0.184 | 0.200 | 0.218 |
| 110 | 0.096 | 0.116 | 0.137 | 0.159 | 0.179 | 0.200 | 0.223 | 0.242 | 0.263 |
| 120 | 0.115 | 0.138 | 0.163 | 0.190 | 0.213 | 0.238 | 0.265 | 0.288 | 0.313 |
| 130 | 0.134 | 0.162 | 0.191 | 0.223 | 0.250 | 0.280 | 0.311 | 0.338 | 0.368 |
| 140 | 0.156 | 0.188 | 0.222 | 0.258 | 0.290 | 0.324 | 0.361 | 0.392 | 0.427 |
| 150 | 0.179 | 0.216 | 0.255 | 0.296 | 0.333 | 0.372 | 0.414 | 0.450 | 0.490 |
| **C** | 100 | 0.111 | 0.134 | 0.159 | 0.185 | 0.208 | 0.232 | 0.258 | 0.281 | 0.305 |
| 110 | 0.135 | 0.163 | 0.192 | 0.223 | 0.251 | 0.281 | 0.312 | 0.340 | 0.369 |
| 120 | 0.161 | 0.193 | 0.229 | 0.266 | 0.299 | 0.334 | 0.371 | 0.404 | 0.439 |
| 130 | 0.188 | 0.227 | 0.268 | 0.312 | 0.351 | 0.392 | 0.436 | 0.474 | 0.516 |
| 140 | 0.219 | 0.263 | 0.311 | 0.362 | 0.407 | 0.455 | 0.505 | 0.550 | 0.598 |
| 150 | 0.251 | 0.302 | 0.357 | 0.416 | 0.467 | 0.522 | 0.580 | 0.632 | 0.687 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not one continuous shear wall segment, the total shear wall length required shall be increased by 0.67 feet (8 inches) for each additional shear wall segment making up the total shear wall length on a side. | | | | | | | | | | |
| 2. The minimum shear wall segment length shall be 2 feet-0 inches. Values less than 2 feet-0 inches are shown only for summation of shear wall segments and for interpolation purposes. A grouted cell with vertical reinforcement of the size indicated is required at each end of every shear wall segment. | | | | | | | | | | |
| 3. Other than incidental utility penetrations, shear wall piers and shear wall segments shall not contain openings with a maximum horizontal or vertical dimension of 5 inches for piers and 12 inches for portions of shear wall segments above and below piers. The total area of openings in any one segment of shear wall shall not exceed 144 square inches. | | | | | | | | | | |
| 4. Required shear wall lengths normal (perpendicular) to the ridge are per lineal foot of building length. Multiply tabular values by building length distance between adjacent shear walls perpendicular to the ridge if interior shear walls are used for total shear wall length required. | | | | | | | | | | |
| 5. Shear wall lengths are based on shear wall segment heights of 80 inches (height from the floor to the top of the highest opening adjacent to the shear segment—corners and openings as permitted by Note 3 of this table are not considered for the purpose of this measurement). For shear segment heights other than 80 inches, multiply tabular length values as follows:   |  |  | | --- | --- | | **Segment Height (inches)** | **Length Multiplier** | | 88 | 1.09 | | 96 | 1.19 | | 104 | 1.28 | | 112 | 1.37 | | | | | | | | | | | |

**(S6183GlitchAM)(S6229GlitchAS)**

**TABLE R609.5.1G GRADE 40**

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO.4 REINFORCEMENT1,2,3,5** **ROOF ANGLE ≤ 23°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 2.32 | 3.31 | 4.51 | 5.29 | 7.15 | 9.31 | 7.77 | 10.45 | 13.55 |
| 110 | 2.81 | 4.01 | 5.46 | 6.40 | 8.65 | 11.27 | 9.40 | 12.65 | 16.39 |
| 120 | 3.35 | 4.77 | 6.50 | 7.61 | 10.30 | 13.41 | 11.19 | 15.05 | 19.51 |
| 130 | 3.93 | 5.59 | 7.63 | 8.94 | 12.09 | 15.74 | 13.13 | 17.67 | 22.90 |
| 140 | 4.55 | 6.49 | 8.84 | 10.36 | 14.02 | 18.26 | 15.23 | 20.49 | 26.56 |
| 150 | 5.23 | 7.45 | 10.15 | 11.90 | 16.09 | 20.96 | 17.48 | 23.52 | 30.48 |
| **C** | 100 | 2.75 | 3.98 | 5.49 | 6.98 | 9.52 | 12.49 | 10.85 | 14.57 | 18.85 |
| 110 | 3.33 | 4.81 | 6.64 | 8.45 | 11.52 | 15.11 | 13.13 | 17.63 | 22.81 |
| 120 | 3.97 | 5.73 | 7.90 | 10.05 | 13.70 | 17.98 | 15.63 | 20.98 | 27.14 |
| 130 | 4.66 | 6.72 | 9.27 | 11.80 | 16.08 | 21.10 | 18.34 | 24.63 | 31.86 |
| 140 | 5.40 | 7.79 | 10.75 | 13.68 | 18.65 | 24.47 | 21.27 | 28.56 | 36.95 |
| 150 | 6.20 | 8.95 | 12.34 | 15.71 | 21.41 | 28.10 | 24.42 | 32.79 | 42.41 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length required. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches and shearwall segment length of 24 inches. | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches and shearwall segment length of 24 inches. | | | | | | | | | | |

**TABLE R609.5.1H GRADE 40**

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO.5 REINFORCEMENT1,2,3,6 ROOF ANGLE ≤ 23°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 1.59 | 2.26 | 3.08 | 3.61 | 4.88 | 6.36 | 5.30 | 7.14 | 9.25 |
| 110 | 1.92 | 2.73 | 3.73 | 4.37 | 5.91 | 7.69 | 6.42 | 8.63 | 11.19 |
| 120 | 2.28 | 3.25 | 4.44 | 5.20 | 7.03 | 9.16 | 7.64 | 10.28 | 13.32 |
| 130 | 2.68 | 3.82 | 5.21 | 6.10 | 8.25 | 10.75 | 8.96 | 12.06 | 15.63 |
| 140 | 3.11 | 4.43 | 6.04 | 7.07 | 9.57 | 12.46 | 10.40 | 13.99 | 18.13 |
| 150 | 3.57 | 5.08 | 6.93 | 8.12 | 10.99 | 14.31 | 11.93 | 16.06 | 20.81 |
| **C** | 100 | 1.88 | 2.71 | 3.75 | 4.77 | 6.50 | 8.52 | 7.41 | 9.95 | 12.87 |
| 110 | 2.28 | 3.28 | 4.53 | 5.77 | 7.86 | 10.31 | 8.96 | 12.04 | 15.57 |
| 120 | 2.71 | 3.91 | 5.39 | 6.86 | 9.36 | 12.28 | 10.67 | 14.32 | 18.53 |
| 130 | 3.18 | 4.59 | 6.33 | 8.05 | 10.98 | 14.41 | 12.52 | 16.81 | 21.75 |
| 140 | 3.69 | 5.32 | 7.34 | 9.34 | 12.73 | 16.71 | 14.52 | 19.50 | 25.22 |
| 150 | 4.23 | 6.11 | 8.43 | 10.72 | 14.62 | 19.18 | 16.67 | 22.38 | 28.95 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length required. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches and shearwall segment length of 24 inches. | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches and shearwall segment length of 24 inches. | | | | | | | | | | |

**TABLE R609.5.1I GRADE 40**

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO.4 REINFORCEMENT1,2,3,5** **ROOF ANGLE 30°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in accordance with Section  R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 2.39 | 3.51 | 4.88 | 5.10 | 7.07 | 9.32 | 7.44 | 10.24 | 13.44 |
| 110 | 2.89 | 4.25 | 5.90 | 6.17 | 8.55 | 11.28 | 9.00 | 12.39 | 16.26 |
| 120 | 3.43 | 5.06 | 7.02 | 7.35 | 10.18 | 13.43 | 10.71 | 14.75 | 19.35 |
| 130 | 4.03 | 5.94 | 8.24 | 8.62 | 11.95 | 15.76 | 12.57 | 17.31 | 22.71 |
| 140 | 4.67 | 6.89 | 9.56 | 10.00 | 13.86 | 18.28 | 14.58 | 20.08 | 26.34 |
| 150 | 5.37 | 7.90 | 10.98 | 11.48 | 15.91 | 20.98 | 16.74 | 23.05 | 30.23 |
| **C** | 100 | 2.87 | 4.29 | 6.05 | 6.79 | 9.51 | 12.66 | 10.37 | 14.24 | 18.63 |
| 110 | 3.47 | 5.19 | 7.32 | 8.22 | 11.51 | 15.32 | 12.55 | 17.23 | 22.54 |
| 120 | 4.13 | 6.18 | 8.71 | 9.78 | 13.69 | 18.23 | 14.93 | 20.51 | 26.83 |
| 130 | 4.85 | 7.26 | 10.23 | 11.48 | 16.07 | 21.40 | 17.53 | 24.06 | 31.49 |
| 140 | 5.62 | 8.41 | 11.86 | 13.31 | 18.64 | 24.82 | 20.33 | 27.91 | 36.52 |
| 150 | 6.45 | 9.66 | 13.62 | 15.28 | 21.39 | 28.49 | 23.33 | 32.04 | 41.92 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length required. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**TABLE R609.5.1J GRADE 40**

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO.5 REINFORCEMENT1,2,3,6** **ROOF ANGLE 30°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 1.63 | 2.40 | 3.33 | 3.48 | 4.83 | 6.37 | 5.08 | 6.99 | 9.17 |
| 110 | 1.97 | 2.90 | 4.03 | 4.21 | 5.84 | 7.70 | 6.15 | 8.46 | 11.10 |
| 120 | 2.34 | 3.45 | 4.80 | 5.02 | 6.95 | 9.17 | 7.31 | 10.07 | 13.21 |
| 130 | 2.75 | 4.05 | 5.63 | 5.89 | 8.16 | 10.76 | 8.58 | 11.82 | 15.50 |
| 140 | 3.19 | 4.70 | 6.53 | 6.83 | 9.46 | 12.48 | 9.96 | 13.71 | 17.98 |
| 150 | 3.66 | 5.40 | 7.49 | 7.84 | 10.86 | 14.32 | 11.43 | 15.73 | 20.64 |
| **C** | 100 | 1.96 | 2.93 | 4.13 | 4.64 | 6.49 | 8.64 | 7.08 | 9.72 | 12.72 |
| 110 | 2.37 | 3.55 | 5.00 | 5.61 | 7.85 | 10.46 | 8.57 | 11.76 | 15.39 |
| 120 | 2.82 | 4.22 | 5.95 | 6.68 | 9.35 | 12.45 | 10.19 | 14.00 | 18.31 |
| 130 | 3.31 | 4.95 | 6.98 | 7.84 | 10.97 | 14.61 | 11.96 | 16.43 | 21.49 |
| 140 | 3.84 | 5.74 | 8.10 | 9.09 | 12.72 | 16.94 | 13.88 | 19.05 | 24.93 |
| 150 | 4.41 | 6.59 | 9.29 | 10.43 | 14.61 | 19.45 | 15.93 | 21.87 | 28.62 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length required. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**TABLE R609.5.1K GRADE 40**

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO.4 REINFORCEMENT1,2,3,5** **ROOF ANGLE 45°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 3.04 | 4.65 | 6.66 | 3.04 | 8.21 | 11.11 | 8.28 | 11.74 | 15.80 |
| 110 | 3.68 | 5.63 | 8.06 | 6.96 | 9.93 | 13.44 | 10.02 | 14.20 | 19.12 |
| 120 | 4.37 | 6.70 | 9.59 | 8.29 | 11.82 | 15.99 | 11.93 | 16.90 | 22.76 |
| 130 | 5.13 | 7.86 | 11.25 | 9.72 | 13.87 | 18.77 | 14.00 | 19.84 | 26.71 |
| 140 | 5.95 | 9.12 | 13.05 | 11.28 | 16.09 | 21.77 | 16.24 | 23.01 | 30.97 |
| 150 | 6.83 | 10.47 | 14.98 | 12.95 | 18.47 | 24.99 | 18.64 | 26.41 | 35.56 |
| **C** | 100 | 3.78 | 5.93 | 8.66 | 7.83 | 11.34 | 15.57 | 10.37 | 14.24 | 18.63 |
| 110 | 4.57 | 7.17 | 10.48 | 9.47 | 13.73 | 18.84 | 12.55 | 17.23 | 22.54 |
| 120 | 5.44 | 8.53 | 12.47 | 11.27 | 16.34 | 22.42 | 14.93 | 20.51 | 26.83 |
| 130 | 6.39 | 10.01 | 14.64 | 13.23 | 19.17 | 26.31 | 17.53 | 24.06 | 31.49 |
| 140 | 7.41 | 11.61 | 16.98 | 15.34 | 22.24 | 30.52 | 20.33 | 27.91 | 36.52 |
| 150 | 8.50 | 13.33 | 19.49 | 17.61 | 25.53 | 35.03 | 23.33 | 32.04 | 41.92 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall required. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**TABLE R609.5.1L GRADE 40**

**REQUIRED SHEARWALL LENGTH PARALLEL TO RIDGE NO.5 REINFORCEMENT1,2,3,6 ROOF ANGLE 45°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance with Section  R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 2.07 | 3.18 | 4.55 | 3.93 | 5.60 | 7.58 | 5.66 | 8.01 | 10.79 |
| 110 | 2.51 | 3.84 | 5.50 | 4.75 | 6.78 | 9.17 | 6.84 | 9.70 | 13.05 |
| 120 | 2.99 | 4.57 | 6.55 | 5.66 | 8.07 | 10.92 | 8.14 | 11.54 | 15.54 |
| 130 | 3.50 | 5.37 | 7.68 | 6.64 | 9.47 | 12.81 | 9.56 | 13.54 | 18.23 |
| 140 | 4.06 | 6.23 | 8.91 | 7.70 | 10.99 | 14.86 | 11.08 | 15.71 | 21.15 |
| 150 | 4.67 | 7.15 | 10.23 | 8.84 | 12.61 | 17.06 | 12.72 | 18.03 | 24.27 |
| **C** | 100 | 2.58 | 4.05 | 5.91 | 5.34 | 7.74 | 10.63 | 7.84 | 11.06 | 14.83 |
| 110 | 3.12 | 4.89 | 7.15 | 6.47 | 9.37 | 12.86 | 9.48 | 13.38 | 17.94 |
| 120 | 3.71 | 5.83 | 8.51 | 7.69 | 11.15 | 15.31 | 11.29 | 15.93 | 21.36 |
| 130 | 4.36 | 6.84 | 9.99 | 9.03 | 13.09 | 17.96 | 13.25 | 18.69 | 25.06 |
| 140 | 5.06 | 7.93 | 11.59 | 10.47 | 15.18 | 20.83 | 15.36 | 21.68 | 29.07 |
| 150 | 5.80 | 9.10 | 13.30 | 12.02 | 17.43 | 23.92 | 17.63 | 24.88 | 33.37 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**TABLE R609.5.1M GRADE 40**

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO.4 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,5** **ROOF ANGLE ≤ 23°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.077 | 0.077 | 0.076 | 0.204 | 0.204 | 0.203 | 0.332 | 0.331 | 0.330 |
| 110 | 0.093 | 0.093 | 0.092 | 0.247 | 0.247 | 0.246 | 0.401 | 0.401 | 0.400 |
| 120 | 0.111 | 0.110 | 0.109 | 0.294 | 0.294 | 0.292 | 0.478 | 0.477 | 0.476 |
| 130 | 0.130 | 0.129 | 0.128 | 0.345 | 0.345 | 0.343 | 0.561 | 0.560 | 0.558 |
| 140 | 0.151 | 0.150 | 0.148 | 0.400 | 0.400 | 0.398 | 0.650 | 0.649 | 0.647 |
| 150 | 0.173 | 0.172 | 0.170 | 0.460 | 0.459 | 0.457 | 0.746 | 0.745 | 0.743 |
| **C** | 100 | 0.108 | 0.107 | 0.106 | 0.286 | 0.286 | 0.285 | 0.465 | 0.464 | 0.463 |
| 110 | 0.131 | 0.130 | 0.128 | 0.347 | 0.346 | 0.344 | 0.563 | 0.562 | 0.560 |
| 120 | 0.155 | 0.155 | 0.153 | 0.413 | 0.412 | 0.410 | 0.670 | 0.669 | 0.667 |
| 130 | 0.182 | 0.181 | 0.179 | 0.484 | 0.483 | 0.481 | 0.786 | 0.785 | 0.783 |
| 140 | 0.212 | 0.210 | 0.208 | 0.562 | 0.560 | 0.558 | 0.911 | 0.910 | 0.908 |
| 150 | 0.243 | 0.242 | 0.239 | 0.645 | 0.643 | 0.640 | 1.046 | 1.045 | 1.042 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**TABLE R609.5.1N GRADE 40**

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO.5 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,6 ROOF ANGLE ≤ 23°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.053 | 0.052 | 0.052 | 0.139 | 0.139 | 0.139 | 0.226 | 0.226 | 0.226 |
| 110 | 0.064 | 0.063 | 0.063 | 0.169 | 0.168 | 0.168 | 0.274 | 0.274 | 0.273 |
| 120 | 0.076 | 0.075 | 0.074 | 0.201 | 0.200 | 0.200 | 0.326 | 0.326 | 0.325 |
| 130 | 0.089 | 0.088 | 0.087 | 0.236 | 0.235 | 0.234 | 0.383 | 0.382 | 0.381 |
| 140 | 0.103 | 0.102 | 0.101 | 0.273 | 0.273 | 0.272 | 0.444 | 0.443 | 0.442 |
| 150 | 0.118 | 0.118 | 0.116 | 0.314 | 0.313 | 0.312 | 0.509 | 0.509 | 0.507 |
| **C** | 100 | 0.074 | 0.073 | 0.072 | 0.196 | 0.195 | 0.194 | 0.317 | 0.317 | 0.316 |
| 110 | 0.089 | 0.089 | 0.088 | 0.237 | 0.236 | 0.235 | 0.384 | 0.384 | 0.383 |
| 120 | 0.106 | 0.106 | 0.104 | 0.282 | 0.281 | 0.280 | 0.457 | 0.457 | 0.455 |
| 130 | 0.125 | 0.124 | 0.122 | 0.331 | 0.330 | 0.328 | 0.536 | 0.536 | 0.534 |
| 140 | 0.144 | 0.144 | 0.142 | 0.383 | 0.383 | 0.381 | 0.622 | 0.621 | 0.620 |
| 150 | 0.166 | 0.165 | 0.163 | 0.440 | 0.439 | 0.437 | 0.714 | 0.713 | 0.711 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**TABLE R609.5.1O GRADE 40**

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO.4 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,5** **ROOF ANGLE 30°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance  with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.127 | 0.147 | 0.168 | 0.238 | 0.258 | 0.280 | 0.349 | 0.370 | 0.391 |
| 110 | 0.153 | 0.178 | 0.204 | 0.288 | 0.312 | 0.338 | 0.423 | 0.447 | 0.473 |
| 120 | 0.182 | 0.211 | 0.242 | 0.343 | 0.372 | 0.403 | 0.503 | 0.532 | 0.563 |
| 130 | 0.214 | 0.248 | 0.284 | 0.402 | 0.436 | 0.473 | 0.591 | 0.624 | 0.661 |
| 140 | 0.248 | 0.288 | 0.330 | 0.467 | 0.506 | 0.548 | 0.685 | 0.724 | 0.766 |
| 150 | 0.285 | 0.330 | 0.379 | 0.536 | 0.581 | 0.629 | 0.786 | 0.831 | 0.880 |
| **C** | 100 | 0.178 | 0.206 | 0.236 | 0.334 | 0.362 | 0.392 | 0.490 | 0.518 | 0.548 |
| 110 | 0.215 | 0.249 | 0.285 | 0.404 | 0.438 | 0.474 | 0.593 | 0.627 | 0.663 |
| 120 | 0.256 | 0.296 | 0.340 | 0.481 | 0.521 | 0.565 | 0.706 | 0.746 | 0.789 |
| 130 | 0.300 | 0.348 | 0.399 | 0.564 | 0.612 | 0.663 | 0.828 | 0.876 | 0.926 |
| 140 | 0.348 | 0.403 | 0.462 | 0.654 | 0.709 | 0.768 | 0.960 | 1.015 | 1.074 |
| 150 | 0.400 | 0.463 | 0.531 | 0.751 | 0.814 | 0.882 | 1.102 | 1.166 | 1.233 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**TABLE R609.5.1P GRADE 40**

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO.5 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,6** **ROOF ANGLE 30°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.087 | 0.100 | 0.115 | 0.163 | 0.176 | 0.191 | 0.239 | 0.252 | 0.267 |
| 110 | 0.105 | 0.121 | 0.139 | 0.197 | 0.213 | 0.231 | 0.289 | 0.305 | 0.323 |
| 120 | 0.125 | 0.144 | 0.165 | 0.234 | 0.254 | 0.275 | 0.344 | 0.363 | 0.384 |
| 130 | 0.146 | 0.169 | 0.194 | 0.275 | 0.298 | 0.323 | 0.403 | 0.426 | 0.451 |
| 140 | 0.170 | 0.196 | 0.225 | 0.319 | 0.345 | 0.374 | 0.468 | 0.494 | 0.523 |
| 150 | 0.195 | 0.225 | 0.258 | 0.366 | 0.397 | 0.430 | 0.537 | 0.568 | 0.601 |
| **C** | 100 | 0.121 | 0.140 | 0.161 | 0.228 | 0.247 | 0.268 | 0.334 | 0.354 | 0.374 |
| 110 | 0.147 | 0.170 | 0.195 | 0.276 | 0.299 | 0.324 | 0.405 | 0.428 | 0.453 |
| 120 | 0.175 | 0.202 | 0.232 | 0.328 | 0.356 | 0.385 | 0.482 | 0.509 | 0.539 |
| 130 | 0.205 | 0.237 | 0.272 | 0.385 | 0.418 | 0.452 | 0.565 | 0.598 | 0.632 |
| 140 | 0.238 | 0.275 | 0.316 | 0.447 | 0.484 | 0.525 | 0.656 | 0.693 | 0.734 |
| 150 | 0.273 | 0.316 | 0.362 | 0.513 | 0.556 | 0.602 | 0.753 | 0.796 | 0.842 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**TABLE R609.5.1Q GRADE 40**

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO.4 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,5** **ROOF ANGLE 45°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance  with Section  R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.170 | 0.204 | 0.242 | 0.281 | 0.316 | 0.353 | 0.392 | 0.427 | 0.464 |
| 110 | 0.205 | 0.247 | 0.292 | 0.340 | 0.382 | 0.427 | 0.475 | 0.517 | 0.562 |
| 120 | 0.244 | 0.294 | 0.348 | 0.405 | 0.455 | 0.508 | 0.565 | 0.615 | 0.669 |
| 130 | 0.287 | 0.345 | 0.408 | 0.475 | 0.534 | 0.597 | 0.663 | 0.722 | 0.785 |
| 140 | 0.333 | 0.401 | 0.474 | 0.551 | 0.619 | 0.692 | 0.769 | 0.837 | 0.910 |
| 150 | 0.382 | 0.460 | 0.544 | 0.632 | 0.710 | 0.794 | 0.883 | 0.961 | 1.045 |
| **C** | 100 | 0.238 | 0.287 | 0.339 | 0.394 | 0.443 | 0.495 | 0.550 | 0.599 | 0.651 |
| 110 | 0.288 | 0.347 | 0.410 | 0.477 | 0.536 | 0.599 | 0.666 | 0.725 | 0.788 |
| 120 | 0.343 | 0.413 | 0.488 | 0.567 | 0.637 | 0.713 | 0.792 | 0.862 | 0.937 |
| 130 | 0.402 | 0.484 | 0.572 | 0.666 | 0.748 | 0.836 | 0.930 | 1.012 | 1.100 |
| 140 | 0.466 | 0.562 | 0.664 | 0.772 | 0.868 | 0.970 | 1.078 | 1.174 | 1.276 |
| 150 | 0.535 | 0.645 | 0.762 | 0.887 | 0.996 | 1.113 | 1.238 | 1.347 | 1.465 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**TABLE R609.5.1R GRADE 40**

**REQUIRED SHEARWALL LENGTH PERPENDICULAR TO RIDGE NO.5 REINFORCEMENT PER FOOT OF BUILDING LENGTH1,2,3,4,6 ROOF ANGLE 45°**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in accordance with Section R301.2.1.3** | **TOP STORY** | | | **1ST STORY OF 2 STORY OR 2ND STORY OF 3 STORY** | | | **1ST STORY OF 3 STORY** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 0.116 | 0.140 | 0.165 | 0.192 | 0.216 | 0.241 | 0.268 | 0.292 | 0.317 |
| 110 | 0.140 | 0.169 | 0.200 | 0.232 | 0.261 | 0.292 | 0.324 | 0.353 | 0.384 |
| 120 | 0.167 | 0.201 | 0.237 | 0.276 | 0.310 | 0.347 | 0.386 | 0.420 | 0.456 |
| 130 | 0.196 | 0.236 | 0.279 | 0.324 | 0.364 | 0.407 | 0.453 | 0.493 | 0.536 |
| 140 | 0.227 | 0.273 | 0.323 | 0.376 | 0.422 | 0.472 | 0.525 | 0.572 | 0.621 |
| 150 | 0.261 | 0.314 | 0.371 | 0.432 | 0.485 | 0.542 | 0.603 | 0.656 | 0.713 |
| **C** | 100 | 0.162 | 0.196 | 0.231 | 0.269 | 0.302 | 0.338 | 0.376 | 0.409 | 0.444 |
| 110 | 0.196 | 0.237 | 0.280 | 0.325 | 0.366 | 0.409 | 0.454 | 0.495 | 0.538 |
| 120 | 0.234 | 0.282 | 0.333 | 0.387 | 0.435 | 0.486 | 0.541 | 0.589 | 0.640 |
| 130 | 0.274 | 0.331 | 0.391 | 0.455 | 0.511 | 0.571 | 0.635 | 0.691 | 0.751 |
| 140 | 0.318 | 0.383 | 0.453 | 0.527 | 0.592 | 0.662 | 0.736 | 0.801 | 0.871 |
| 150 | 0.365 | 0.440 | 0.520 | 0.605 | 0.680 | 0.760 | 0.845 | 0.920 | 1.000 |
| **Notes:** | | | | | | | | | | |
| 1. The cumulative shear wall segment length for each side of the building shall be equal to or greater than the tabular shear wall length required. If the required shear wall segment length provided is not continuous the total shear wall length required shall be increased by 8" for each additional shear wall segment on a side. | | | | | | | | | | |
| 2. Minimum shear wall segment length shall be 2′-0". A grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. A minimum length of 24 inches (610 mm) of solid wall segment, extending the full height of each wall, shall be provided at exterior corners of exterior walls. A fully grouted cell with reinforcing steel shall be provided at each end of every shear wall segment. | | | | | | | | | | |
| 3. Portions of walls with openings shall not be considered part of the shear wall length. | | | | | | | | | | |
| 4. Required shearwall lengths perpendicular to the ridge are per lineal foot of building length. Multiply tabular values by building length for total shear wall length. | | | | | | | | | | |
| 5. Shearwall lengths for no. 4 reinforcement are based on shearwall segment height of 80 inches (2032 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |
| 6. Shearwall lengths for no. 5 reinforcement are based on shearwall segment height of 96 inches (2438 mm) and shearwall segment length of 24 inches (610 mm). | | | | | | | | | | |

**(S6141GlitchAS)(S6183GlitchAM)**

**TABLE R609.5.3A**

**TOTAL SHEAR AT TOP OF TOP STORY WALL1,2**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as determined in accordance with Section R301.2.1.3** | **VEL PRESSURE** | **ROOF ANGLE UP TO 45°** | | | **ROOF ANGLE UP TO 30°** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 15.2 | 2022 | 3113 | 4456 | 1568 | 2319 | 3215 |
| 110 | 18.4 | 2447 | 3767 | 5392 | 1897 | 2806 | 3890 |
| 120 | 22.0 | 2912 | 4483 | 6417 | 2258 | 3339 | 4630 |
| 130 | 25.8 | 3418 | 5262 | 7531 | 2649 | 3919 | 5433 |
| 140 | 29.9 | 3964 | 6102 | 8734 | 3073 | 4545 | 6301 |
| 150 | 34.3 | 4550 | 7005 | 10,027 | 3527 | 5218 | 7234 |
| **C** | 100 | 21.4 | 2835 | 4365 | 6248 | 2198 | 3251 | 4507 |
| 110 | 25.9 | 3431 | 5282 | 7560 | 2660 | 3934 | 5454 |
| 120 | 30.8 | 4083 | 6286 | 8997 | 3165 | 4682 | 6491 |
| 130 | 36.1 | 4792 | 7377 | 10,559 | 3715 | 5495 | 7618 |
| 140 | 41.9 | 5557 | 8555 | 12,246 | 4308 | 6372 | 8835 |
| 150 | 48.1 | 6380 | 9821 | 14,058 | 4946 | 7315 | 10,142 |

|  |
| --- |
| 1. Loads are based on 10' wall height. Multiply by 0.9 for 8-foot (2438 mm) wall heights. |
| 2. To determine individual connector load parallel to the wall, divide shear value by the number of connectors (Load F1 from Figure R609.5.3). |

**(S6142GlitchAM)**

**TABLE R609.5.3B**

**TRANSVERSE CONNECTOR LOAD (F2)1,2**

|  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in accordance with Section R301.2.1.3** | **VEL PRESSURE** | **ROOF ANGLE <23°** | | **ROOF ANGLE >23°** | |
| **EDGE ZONE** | **INT ZONE** |
| **B** | 100 | 15.2 | 394 | 319 | 289 | |
| 110 | 18.4 | 477 | 386 | 349 | |
| 120 | 22.0 | 568 | 460 | 416 | |
| 130 | 25.8 | 667 | 539 | 488 | |
| 140 | 29.9 | 773 | 626 | 566 | |
| 150 | 34.3 | 887 | 718 | 650 | |
| **C** | 100 | 21.4 | 553 | 448 | 405 | |
| 110 | 25.9 | 669 | 541 | 490 | |
| 120 | 30.8 | 796 | 644 | 583 | |
| 130 | 36.1 | 935 | 756 | 684 | |
| 140 | 41.9 | 1084 | 877 | 793 | |
| 150 | 48.1 | 1244 | 1007 | 911 | |
| 1. Loads are based on 10-foot wall height. Multiply by 0.8 for 8-foot (2438 mm)  wall height. | | | | | |
| 2. F2 load in accordance with Figure R609.5.3. **(S6183GlitchAM)(S6143GlitchAM)**  **Fig. R609.5.3**  **Typical Roof to Wall Connections** | | | | | |

**TABLE R609.5.4**

**END WALL ROOF SHEAR PER FOOT OF BUILDING LENGTH**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **EXP** | **Vasd as  determined in accordance  with Section R301.2.1.3** | **POUNDS PER FT OFBLDGLENGTH FOR 23° ROOF SLOPE** | | | **POUNDS PER FT OFBLDGLENGTH FOR 30° ROOF SLOPE** | | | **POUNDS PER FT OFBLDGLENGTH FOR 45° ROOF SLOPE** | | |
| **BUILDING WIDTH** | | | **BUILDING WIDTH** | | | **BUILDING WIDTH** | | |
| **24** | **32** | **40** | **24** | **32** | **40** | **24** | **32** | **40** |
| **B** | 100 | 43.6 | 42.8 | 43.1 | 76.9 | 87.8 | 100.1 | 104.4 | 123.8 | 145.1 |
| 110 | 52.7 | 51.8 | 52.2 | 93.1 | 106.3 | 121.1 | 126.3 | 149.8 | 175.5 |
| 120 | 62.8 | 61.7 | 62.1 | 110.8 | 126.5 | 144.2 | 150.3 | 178.3 | 208.9 |
| 130 | 73.7 | 72.4 | 72.8 | 130.0 | 148.5 | 169.2 | 176.4 | 209.2 | 245.1 |
| 140 | 85.4 | 84.0 | 84.5 | 150.8 | 172.2 | 196.2 | 204.6 | 242.6 | 284.3 |
| 150 | 98.1 | 96.4 | 97.0 | 173.1 | 197.7 | 225.3 | 234.8 | 278.5 | 326.4 |
| **C** | 100 | 61.1 | 60.1 | 60.4 | 107.9 | 123.2 | 140.4 | 146.3 | 173.6 | 203.4 |
| 110 | 73.9 | 72.7 | 73.1 | 130.5 | 149.0 | 169.9 | 177.1 | 210.0 | 246.1 |
| 120 | 88.0 | 86.5 | 87.0 | 155.3 | 177.4 | 202.1 | 210.7 | 249.9 | 292.9 |
| 130 | 103.3 | 101.5 | 102.1 | 182.3 | 208.1 | 237.2 | 247.3 | 293.3 | 343.7 |
| 140 | 119.8 | 117.7 | 118.4 | 211.4 | 241.4 | 275.1 | 286.8 | 340.2 | 398.6 |
| 150 | 137.5 | 135.2 | 136.0 | 242.7 | 277.1 | 315.8 | 329.2 | 390.5 | 457.6 |
| **Notes:** | | | | | | | | | | |
| 1. Tabular values between 23° and 30° and between 30° and 45° are permitted to be interpolated. | | | | | | | | | | |
| 2. Multiply by total building length for total end wall shear. Divide total shear by building width for required shear capacity of roof diaphragm and connections. | | | | | | | | | | |

**(S6144GlitchAM)**

**R609.6 Assemblies and beams spanning openings.**

**R609.6.1 Preengineered assemblies for masonry walls.**

**R609.6.1.1** Unreinforced masonry units above an opening and 8-inch (203 mm) high bond beams above an opening shall be supported by an assembly.

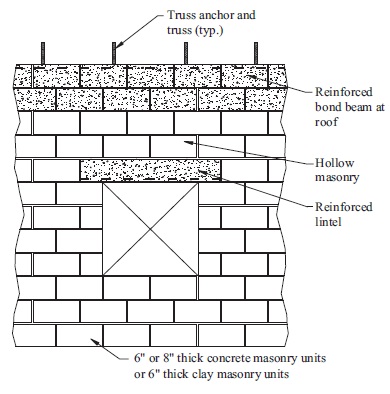
**R609.6.1.2** Preengineered assemblies shall be selected from a manufacturer’s approved schedule or other approved tables for the load capacities based on the appropriate minimum gravity load carrying capacities established in Tables R609.6.1.2(1), R609.6.1.2(2), and R609.6.1.2(3).

**TABLE R609.6.1.2(1)**

**SUPERIMPOSED LOADS MINIMUM RATED LOAD CAPACITY OF 6-INCHOR 8-INCH-THICK PRE-ENGINEERED ASSEMBLIES SPANNING OPENINGS OF ONE STORY AND TOP STORY OF MULTI-STORY BUILDINGS1,3,4**

|  |  |  |  |  |
| --- | --- | --- | --- | --- |
| **ROOF SPAN (FT)** | **UPLIFT (plf)** | | | |
| **GRAVITY (plf)** | **Vasd as determined in accordance with Section R301.2.1.3** | | |
| **100 mph** | **120 mph** | **140 mph** |
| **42** | 150 | 85 | 112 | 165 |
| **12** | 330 | 152 | 204 | 305 |
| **24** | 600 | 262 | 351 | 525 |
| **36** | 870 | 374 | 502 | 745 |
| **44** | 1,050 | 451 | 605 | 900 |
| **52** | 1,230 | 530 | 710 | 1,055 |
| **60** | 1,410 | 609 | 816 | 1,215 |
| **Notes:** | | | | |
| 1. All loads are superimposed at the top of the wall and do not include dead loads of the bond beam or masonry above the assembly. Add 100% of additional dead loads to the gravity loads and subtract 85% of these loads from the uplift loads. | | | | |
| 2. Use 4-foot roof span for assemblies in endwalls. | | | | |
| 3. For total roof dead loads over 10 psf, increase gravity loads by the following amount: (Roof Dead Load - 10 psf) × (Roof Span + 2 ft)/2  **(S6152GlitchAS)** | | | | |
|  | | | | |

**TABLE R609.6.1.2(2)**



**SUPERIMPOSED LOADS MINIMUM RATED LOAD CAPACITY 8-INCH-THICK PREENGINEERED ASSEMBLIES SPANNING OPENINGS OF BOTTOM STORY OF TWO-STORY UILDINGS, SECOND AND BOTTOM STORIES OF THREE-STORY BUILDINGS—WOOD FLOOR SYSTEM3,4,5**

|  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- |
| **FLOOR SPAN1 (FT)** | **MINIMUM RATED GRAVITY LOAD ASSEMBLY (plf)** | | | | | | |
| **ASSEMBLY CLEAR SPAN (FT)** | | | | | | |
| **4** | **6** | **8** | **12** | | **16** | **20** |
| **42** | 210 | 260 | 310 | 410 | | 510 | 610 |
| **12** | 430 | 480 | 530 | 630 | | 730 | 830 |
| **24** | 760 | 810 | 860 | 960 | | 1,060 | 1,160 |
| **36** | 1,090 | 1,140 | 1,190 | 1,290 | | 1,390 | 1,490 |
| **44** | 1,310 | 1,360 | 1,410 | 1,510 | | 1,610 | 1,710 |
| **52** | 1,530 | 1,580 | 1,630 | 1,730 | | 1,830 | 1,930 |
| **60** | 1,750 | 1,800 | 1,850 | 1,950 | | 2,050 | 2,150 |
| **Notes:** | | | | | | | |
| 1. For a wall supporting floors on both sides, enter table with the sum of the 2 full spans. | | | | | | | |
| NOTE: Tabular values are for 1/2 the load of the full span shown. | | | | | | | |
| 2. Use 4 ft building width for assemblies in walls not supporting floors (normally endwalls and interior masonry walls and shearwalls). | | | | | | | |
| 3. The values in this table may be interpolated. | | | | | | | |
| 4. These loads take into account the dead load of any masonry in the wall above the assembly and live and dead loads of the roof and floor supported. Dead load of the assembly is not included in the table and if not included in the preengineered concrete design must be added to the loads in the table. | | | | | | | |
| 5. This table is applicable for all roof dead loads. | | | | | | | |
|  | | | | |

**TABLE R609.6.1.2(3)**

**SUPERIMPOSED LOADS MINIMUM RATED LOAD CAPACITY 8-INCH-THICK PREENGINEERED ASSEMBLIES SPANNING OPENINGS OF BOTTOM STORY OF TWO-STORY BUILDINGS, SECOND AND BOTTOM STORIES OF THREE-STORY BUILDINGS—HOLLOWCORE FLOOR SYSTEM3,4,5**

|  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- |
| **FLOOR SPAN1(FT)** | **MINIMUM RATED GRAVITY LOAD ASSEMBLY (plf)** | | | | | |
| **ASSEMBLY CLEAR SPAN (FT)** | | | | | |
| **4** | **6** | **8** | **12** | **16** | **20** |
| **42** | 290 | 340 | 390 | 490 | 590 | 690 |
| **12** | 670 | 720 | 770 | 870 | 970 | 1,070 |
| **24** | 1,240 | 1,290 | 1,340 | 1,440 | 1,540 | 1,640 |
| **36** | 1,810 | 1,860 | 1,910 | 2,010 | 2,110 | 2,210 |
| **44** | 2,190 | 2,240 | 2,290 | 2,390 | 2,490 | 2,590 |
| **52** | 2,570 | 2,620 | 2,670 | 2,770 | 2,870 | 2,970 |
| **60** | 2,950 | 3,000 | 3,050 | 3,150 | 3,250 | 3,350 |

|  |
| --- |
| **Notes:** |
| 1. For a wall supporting floors on both sides, enter table with the sum of the 2 full spans. |
| NOTE: Tabular values are for 1/2 the load of the full span shown. |
| 2. Use 4 ft building width for assemblies in nonfloor bearing walls (normally endwalls and interior masonry walls and shearwalls). |
| 3. The values in this table may be interpolated. |
| 4. These loads take into account the dead load of any masonry in the wall above the assembly and live and dead loads of the roof and floor supported. Dead load of the assembly is not included in the table and if not included in the preengineered concrete assembly design must be added to the loads in the table. |
| 5. This table is applicable for all roof dead loads. |

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**(S6183GlitchAM)**

**R609.6.1.3** Preengineered assemblies may function as a bond beam over an opening provided that:

1. The bond beam reinforcement is continuous through the assembly.

2. The assembly has an uplift rating that equals or exceeds the appropriate value stipulated in Table R609.6.1.2(1) if the lintel directly supports a roof.  **Exception:** If the reinforcement in the top of the assembly is equal to or greater than the reinforcement required in the bottom of the assembly by the manufacturer, uplift need not be considered.

**R609.6.1.4** Preengineered assemblies spanning openings shall extend a minimum of 4 inches (102 mm) nominal past each side of the opening.

**R609.6.2 Continuous bond beams spanning openings.**

**R609.6.2.1** Under the provisions of this section, bond beams shall:

1. Be 16 inches (406 mm) high nominal over openings, except cast-in-place concrete bond beams, which may be 12 inches (305 mm) high nominal.

2. Have top reinforcement continuous over the wall and opening.

3. Have bottom reinforcement extending past each side of the opening a minimum of 24 inches (610 mm) for concrete walls and 4 inches (102 mm) for masonry walls.

4. Meet the provisions of Tables R609.6.2.1(1), R609.6.2.1(2) and R609.6.2.1(3) as appropriate.

**TABLE R609.6.2.1(1)**

**MAXIMUM CLEAR SPAN CAPACITY OF CONTINUOUS BOND BEAMS ACTING AS LINTELS ONE STORY AND TOP STORY OF MULTISTORY BUILDINGS**

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **ROOF SPAN (FT)** | **MAXIMUM ALLOWABLE CLEAR SPAN (FT-IN)5** | | | | | | | | | | | |
| **BOND BEAM 6” THICK WALL1, 2, 4** | | | | | | **BOND BEAM 8” THICK WALL1, 2, 4** | | | | | |
| **16-1** | **16-2** | **C12-1** | **C12-2** | **C16-1** | **C16-2** | **16-1** | **16-2** | **C12-1** | **C12-2** | **C16-1** | **C16-2** |
| 43 | 16-0 | 17-4 | 16-0 | 20-8 | 18-0 | 24-8 | 16-0 | 18-8 | 15-4 | 20-8 | 17-4 | 23-4 |
| 12 | 12-0 | 13-4 | 12-0 | 15-4 | 14-0 | 18-8 | 12-8 | 14-0 | 11-4 | 16-0 | 13-4 | 18-0 |
| 24 | 8-8 | 8-8 | 9-4 | 10-8 | 10-8 | 14-8 | 10-0 | 11-4 | 8-8 | 12-8 | 10-8 | 14-8 |
| 36 | 6-8 | 6-8 | 8-0 | 8-0 | 9-4 | 11-4 | 8-8 | 8-8 | 7-4 | 10-0 | 8-8 | 12-0 |
| 44 | 6-0 | 6-0 | 7-4 | 7-4 | 8-0 | 10-0 | 7-4 | 7-4 | 6-8 | 8-8 | 8-0 | 11-4 |
| 52 | 5-4 | 5-4 | 6-0 | 6-0 | 8-0 | 8-8 | 6-8 | 6-8 | 6-8 | 8-0 | 7-4 | 10-8 |
| 60 | 4-8 | 4-8 | 6-0 | 6-0 | 7-4 | 8-0 | 6-0 | 6-0 | 6-0 | 7-4 | 7-4 | 10-0 |
| **Notes:** | | | | | | | | | | | | |
| 1. Designation of bond beam types over openings: | | | | | | | | | | | | |
| a. Letter C designates a concrete bond beam. All other bond beams are masonry. | | | | | | | | | | | | |
| b. The first number denotes the nominal height of the bond beam in inches. | | | | | | | | | | | | |
| c. The second number denotes the number of No.5 reinforcing bars in the top and the bottom of the beam. A single (1) No.7 bar may be used in lieu of two No.5 bars. The bottom reinforcing steel shall be located no more than 2 3/4 inches clear distance from the bottom of masonry bond beams and 11/2 inches for concrete bond beams. | | | | | | | | | | | | |
| 2. All bond beams have reinforcement in the top as required by Tables R609.2.2A-1 through 609.2.2A-4 and Tables R609.2.2B-1 through R609.2.2B-8 as appropriate. If two No.5 are required in this table and only one No.5 is required by Tables R609.2.2A-1 through R609.2.2A-4 and Tables R609.2.2B-1 through R609.2.2B-8 as appropriate, the additional bar shall be placed in the top of the bond beam over the opening and shall extend past the opening a minimum of 24 inches. | | | | | | | | | | | | |
| 3. Use 4 foot roof span for lintels in endwalls. | | | | | | | | | | | | |
| 4. The bottom reinforcement in precast lintels may be used to satisfy the continuous bond beam bottom reinforcement requirement. | | | | | | | | | | | | |
| 5. For roof dead loads more than 10 psf: | | | | | | | | | | | | |
| a. For 20 psf roof dead load, multiply allowable clear spans by 0.85. | | | | | | | | | | | | |
| b. For 30 psf roof dead load, multiply allowable clear spans by 0.75. | | | | | | | | | | | | |
| c. Values for other roof dead loads may be interpolated. | | | | | | | | | | | | |

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**TABLE R609.6.2.1(2)**

**MAXIMUM CLEAR SPAN OF CONTINUOUS BOND BEAMS ACTING AS LINTELS**

**BOTTOM STORY OF TWO-STORY BUILDINGS, SECOND AND BOTTOM STORIES**

**OF THREE-STORY BUILDINGS—WOOD FLOOR SYSTEM**

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **BUILDING WIDTH (FT)** | **BOND BEAM 8” THICK WALL1, 2, 4** | | | | | | | |
| **16-1** | **16-2** | **C12-1** | **C12-2** | **C16-1** | | **C16-2** | **C16-3** |
| **MAXIMUM ALLOWABLE CLEAR SPAN (FT-IN)5** | | | | | | | |
| **43** | 11-4 | 13-4 | 10-8 | 14-0 | 12-0 | | 15-4 | 18-0 |
| **12** | 10-0 | 11-4 | 9-4 | 12-0 | 10-8 | | 14-0 | 16-0 |
| **24** | 8-8 | 8-8 | 8-0 | 10-0 | 8-8 | | 12-0 | 12-8 |
| **36** | 6-8 | 6-8 | 6-8 | 8-0 | 8-0 | | 10-8 | 10-8 |
| **44** | 6-0 | 6-0 | 6-0 | 7-4 | 7-4 | | 9-4 | 9-4 |
| **52** | 5-4 | 5-4 | 6-0 | 6-8 | 6-8 | | 8-8 | 8-8 |
| **60** | 4-8 | 4-8 | 5-4 | 6-0 | 6-8 | | 8-0 | 8-0 |
| **Notes:** | | | | | | | | |
| 1. Designation of bond beam over openings: | | | | | | | | |
| a. Letter C designates a concrete bond beam. All other bond beams are masonry. | | | | | | | | |
| b. The first number denotes the nominal height of the bond beam in inches. | | | | | | | | |
| c. The second number denotes the number of No.5 reinforcing bars in the top and the bottom of the beam. One No.7 may be used in lieu of two No.5. The bottom reinforcing steel shall be located no more than 2 3/4 inches clear distance from the bottom of masonry bond beams and 11/2 inches for concrete bond beams. | | | | | | | | |
| 2. All bond beams shall have reinforcement in the top in accordance with Section R609.6.2. | | | | | | | | |
| 3. Use 4-foot floor span for lintels in walls parallel to hollowcore. | | | | | | | | |
| 4. The bottom reinforcement in precast lintels may be used to satisfy the continuous bond beam bottom reinforcement requirement. | | | | | | | | |
| 5. This table is applicable for all roof dead loads. | | | | | | | | |
|  | | | | | |

**TABLE R609.6.2.1(3)**

**MAXIMUM CLEAR SPAN OF CONTINUOUS BOND BEAMS ACTING LINTELS BOTTOM STORY OF TWO-STORY BUILDINGS,**

**SECOND AND BOTTOM STORIES OF THREE-STORY BUILDINGS—HOLLOWCORE SECOND FLOOR**

|  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- |
| **BUILDING WIDTH (FT)** | **BOND BEAM 8” THICK WALL1, 2, 4** | | | | | | |
| **16-1** | **16-2** | **C12-1** | **C12-2** | **C16-1** | **C16-2** | **C16-3** |
| **MAXIMUM ALLOWABLE CLEAR SPAN (FT-IN)5** | | | | | | |
| **43** | 10-8 | 12-0 | 10-0 | 13-4 | 11-4 | 14-8 | 17-4 |
| **12** | 8-8 | 9-4 | 8-0 | 10-8 | 9-4 | 12-0 | 13-4 |
| **24** | 6-0 | 6-0 | 6-0 | 7-4 | 7-4 | 10-0 | 10-0 |
| **36** | 4-8 | 4-8 | 5-4 | 6-0 | 6-0 | 8-0 | 8-0 |
| **44** | 4-0 | 4-0 | 4-8 | 5-4 | 6-0 | 7-4 | 7-4 |
| **52** | 4-0 | 4-0 | 4-8 | 4-8 | 5-4 | 6-8 | 6-8 |
| **60** | 3-4 | 3-4 | 4-0 | 4-0 | 5-4 | 6-0 | 6-0 |
| **Notes:** | | | | | | | |
| 1. Designation of bond beam over openings: | | | | | | | |
| d Letter C designates a concrete bond beam. All other bond beams are masonry. | | | | | | | |
| e. The first number denotes the nominal height of the bond beam in inches. | | | | | | | |
| f. The second number denotes the number of No.5 reinforcing bars in the top and the bottom of the beam. One No.7 may be used in lieu of two No.5. The bottom reinforcing steel shall be located no more than 2 3/4 inches clear distance from the bottom of masonry bond beams and 11/2 inches for concrete bond beams. | | | | | | | |
| 2. All bond beams shall have reinforcement in the top in accordance with Section R609.6.2. | | | | | | | |
| 3. Use 4-foot floor span for lintels in walls parallel to hollowcore. | | | | | | | |
| 4. The bottom reinforcement in precast lintels may be used to satisfy the continuous bond beam bottom reinforcement requirement. | | | | | | | |
| 5. This table is applicable for all roof dead loads. | | | | | | | |

**(S6183GlitchAM)**

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**R609.6.2.2** Top reinforcement required over the opening which is in addition to that required over the wall shall extend past the opening a minimum of 24 inches (610 mm).

**R609.6.2.3** When pre-engineered assemblies are utilized to form the bottom portion of the bond beam over the opening in masonry walls, the bottom reinforcement of the pre-engineered assemblies shall be counted toward the additional bottom reinforcement required over the opening.

**R609.6.3 Bond beams combined with lintels.**

**R609.6.3.1** The provisions of this section shall apply when the lintel, the wall area between the lintel and the bond beam, and the bond beam itself are solid grouted masonry units or cast together as one unit.

**R609.6.3.2** Combined bond beams/lintels shall meet the requirements of the appropriate Table R609.6.3.2(1), R609.6.3.2(2) or R609.6.3.2(3).

**TABLE R609.6.3.2(1)**

**COMBINED BOND BEAM/LINTELS ONE STORY AND TOP STORY OF MULTISTORY BUILDINGS**

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **BOND BEAM HEIGHT** | **ROOF SPAN  (FT)** | **MAXIMUM ALLOWABLE CLEAR SPAN (FT-IN)** | | | | | | | | | | | | | | | |
| **COMBINED BOND BEAM/LINTEL 8" THICK WALL1, 2** | | | | | | | | | | | | | | | |
| **12-1** | **12-2** | **16-1** | **16-2** | **24-1** | **24-2** | **24-3** | **32-2** | **32-3** | **32-4** | **40-2** | **40-3** | **40-4** | **48-3** | **48-4** | **48-5** |
| **6"** | 4 | 11-4 | 12-0 | 14-8 | 16-0 | 18-8 | 22-8 | 23-4 | 27-4 | 29-4 | 30-0 | 29-4 | 34-0 | 35-4 | 38-0 | 39-4 | 40-8 |
| 12 | 8-0 | 8-8 | 11-4 | 12-0 | 14-8 | 17-4 | 17-4 | 18-8 | 22-9 | 23-3 | 24-8 | 24-8 | 28-9 | 29-4 | 31-4 | 33-4 |
| 24 | 6-0 | 6-9 | 8-8 | 8-8 | 11-4 | 14-0 | 14-0 | 18-0 | 18-0 | 18-0 | 20-0 | 22-0 | 20-0 | 26-0 | 26-0 | 26-0 |
| 36 | 4-8 | 4-8 | 6-8 | 6-8 | 10-0 | 10-8 | 10-8 | 14-0 | 14-0 | 14-0 | 17-4 | 17-4 | 17-4 | 20-8 | 20-8 | 20-8 |
| 44 | 4-0 | 4-0 | 6-0 | 6-0 | 9-4 | 9-4 | 9-4 | 12-8 | 12-8 | 12-8 | 15-4 | 15-4 | 15-4 | 18-0 | 18-0 | 18-0 |
| 52 | 3-4 | 3-4 | 5-4 | 5-4 | 8-0 | 8-0 | 8-0 | 11-4 | 11-4 | 11-4 | 14-0 | 14-0 | 14-0 | 16-8 | 16-8 | 16-8 |
| 60 | 3-4 | 3-4 | 4-8 | 4-8 | 7-4 | 7-4 | 7-4 | 10-0 | 10-0 | 10-0 | 12-8 | 12-8 | 12-8 | 15-4 | 15-4 | 15-4 |
| **8"** | 4 | 12-0 | 12-8 | 14-8 | 16-8 | 17-4 | 23-4 | 24-8 | 25-4 | 30-0 | 30-8 | 26-8 | 32-8 | 35-4 | 34-0 | 39-4 | 40-8 |
| 12 | 8-8 | 9-4 | 11-4 | 13-4 | 14-0 | 18-8 | 20-8 | 21-4 | 24-8 | 26-0 | 22-8 | 28-0 | 30-0 | 29-4 | 34-0 | 35-4 |
| 24 | 6-8 | 7-4 | 9-4 | 10-0 | 11-4 | 15-4 | 16-0 | 17-4 | 20-8 | 21-4 | 19-4 | 23-4 | 25-4 | 25-4 | 28-8 | 30-0 |
| 36 | 6-0 | 6-0 | 8-0 | 8-8 | 9-4 | 13-4 | 13-4 | 15-4 | 17-4 | 17-4 | 16-8 | 20-8 | 21-4 | 22-0 | 24-8 | 24-8 |
| 44 | 5-4 | 5-4 | 7-4 | 7-4 | 8-8 | 11-4 | 11-4 | 14-0 | 15-4 | 15-4 | 16-0 | 18-8 | 18-8 | 20-8 | 22-0 | 22-0 |
| 52 | 4-8 | 4-8 | 6-8 | 6-8 | 8-0 | 10-0 | 10-0 | 13-4 | 14-0 | 14-0 | 14-8 | 17-4 | 17-4 | 19-4 | 20-0 | 20-0 |
| 60 | 4-0 | 4-0 | 6-0 | 6-0 | 8-0 | 9-4 | 9-4 | 12-8 | 12-8 | 12-8 | 14-0 | 15-4 | 15-4 | 18-8 | 18-8 | 18-8 |
| **Notes:** | | | | | | | | | | | | | | | | | |
| 1. Designation of combined bond beam/lintels: | | | | | | | | | | | | | | | | | |
| a. The first number denotes the nominal height of the bond beam/lintel in inches. | | | | | | | | | | | | | | | | | |
| b. The second number denotes the number of No.5 reinforcing bars in the bottom of the bond beam/lintel. The equivalent or greater area of reinforcement may be obtained by using reinforcement other than No.5. For example, when three No.5 are required, one No.9 may be used. Also, one No.7 may be used to replace two No.5 or two No.7 to replace four No.5. The bottom reinforcing steel is to be located not more than 23/4 inches clear distance from the bottom of the lintel. | | | | | | | | | | | | | | | | | |
| 2. All bond beams shall have reinforcement in the top in accordance with Tables R609.2.2A-1 through R609.2.2A-4 and Tables R609.2.2B-1 through R609.2.2B-8, as appropriate. | | | | | | | | | | | | | | | | | |

**(S6183GlitchAM)(S6153GlitchAS)**

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**TABLE R609.6.3.2(2)**

**COMBINED BOND BEAM/LINTELS BOTTOM STORY OF TWO-STORY BUILDINGS, SECOND AND BOTTOM STORIES OF THREE-STORY BUILDINGS—WOOD FLOOR SYSTEM**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **FLOOR SPAN  SUPPORTED  (FT)** | **COMBINED BOND BEAM/LINTEL 8" THICK WALL1, 2, 4** | | | | | | | | | |
| **12-2** | **16-2** | **24-2** | **24-3** | **32-2** | **32-3** | **40-3** | **40-4** | **48-3** | **48-4** |
| **MAXIMUM ALLOWABLE CLEAR SPAN (FT-IN)5** | | | | | | | | | |
| 43 | 9-4 | 12-0 | 16-0 | 16-8 | 18-0 | 20-0 | 22-8 | 24-0 | 24-0 | 26-8 |
| 12 | 8-0 | 10-8 | 14-0 | 15-4 | 16-0 | 18-8 | 20-8 | 22-0 | 22-0 | 24-0 |
| 24 | 6-0 | 8-8 | 12-0 | 12-0 | 14-0 | 15-4 | 18-0 | 18-0 | 20-0 | 20-8 |
| 36 | 4-8 | 6-8 | 10-0 | 10-0 | 12-8 | 13-4 | 16-0 | 16-0 | 18-0 | 18-0 |
| 44 | 4-0 | 6-0 | 9-4 | 9-4 | 12-0 | 12-0 | 14-8 | 14-8 | 16-8 | 16-8 |
| 52 | 4-0 | 5-4 | 8-8 | 8-8 | 10-8 | 10-8 | 13-4 | 13-4 | 16-0 | 16-0 |
| 60 | 3-4 | 4-8 | 8-0 | 8-0 | 10-0 | 10-0 | 12-8 | 12-8 | 14-8 | 14-8 |
| **Notes:** | | | | | | | | | | |
| 1. Designation of combined bond beam/lintels: | | | | | | | | | | |
| a. The first number denotes the nominal height of the bond beam/lintel in inches. | | | | | | | | | | |
| b. The second number denotes the number of No.5 reinforcing bars in the bottom of the bond beam/lintel. The equivalent or greater area of reinforcement may be obtained by using reinforcement other than No.5 bars. For example, when three No.5 are required, one No.9 may be used. Also, one No.7 may be used to replace two No.5 or two No.7 may be used to replace four No.5. The bottom reinforcing steel is to be located not more than 23/4 inches clear distance from the bottom of the lintel. | | | | | | | | | | |
| 2. All bond beams shall have reinforcement in the top in accordance with Section R609.6.2. | | | | | | | | | | |
| 3. Use 4 foot floor span for walls parallel to hollowcore (nonloadbearing). | | | | | | | | | | |
| 4. All The bottom reinforcement in precast lintels may be used to satisfy the continuous bond beam bottom reinforcement requirement. | | | | | | | | | | |
| 5. This table is applicable for all roof dead loads. | | | | | | | | | | |

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**TABLE R609.6.3.2(3)**

**COMBINED BOND BEAM/LINTELS BOTTOM STORY OF TWO-STORY BUILDINGS, SECOND AND BOTTOM STORIES OF THREE-STORY BUILDINGS—HOLLOWCORE FLOOR SYSTEM**

|  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **FLOOR SPAN SUPPORTED  (FT)** | **COMBINED BOND BEAM/LINTEL 8" THICK WALL1, 2, 4** | | | | | | | | | |
| **12-2** | **16-2** | **24-2** | **24-3** | **32-2** | **32-3** | **40-3** | **40-4** | **48-3** | **48-4** |
| **MAXIMUM ALLOWABLE CLEAR SPAN (FT-IN)5** | | | | | | | | | |
| 43 | 8-8 | 11-4 | 15-4 | 16-0 | 17-4 | 19-4 | 22-0 | 23-4 | 23-4 | 25-4 |
| 12 | 6-8 | 9-4 | 12-8 | 12-8 | 14-8 | 16-0 | 18-8 | 18-8 | 20-8 | 21-4 |
| 24 | 4-8 | 6-0 | 9-4 | 9-4 | 12-0 | 12-0 | 14-8 | 14-8 | 17-4 | 17-4 |
| 36 | 3-4 | 4-8 | 7-4 | 7-4 | 10-0 | 10-0 | 12-0 | 12-0 | 14-8 | 14-8 |
| 44 | 2-8 | 4-0 | 6-8 | 6-8 | 8-8 | 8-8 | 11-4 | 11-4 | 13-4 | 13-4 |
| 52 | 2-8 | 4-0 | 6-0 | 6-0 | 8-0 | 8-0 | 10-0 | 10-0 | 12-0 | 12-0 |
| 60 | 2-8 | 3-4 | 5-4 | 5-4 | 7-4 | 7-4 | 9-4 | 9-4 | 11-4 | 11-4 |
| **Notes:** | | | | | | | | | | |
| 1. Designation of combined bond beam/lintels: | | | | | | | | | | |
| a. The first number denotes the nominal height of the bond beam/lintel in inches. | | | | | | | | | | |
| b. The second number denotes the number of No.5 reinforcing bars in the bottom of the bond beam/lintel. The equivalent or greater area of reinforcement may be obtained by using reinforcement other than No.5 bars. For example, when three No.5 are required, one No.9 may be used. Also, one No.7 may be used to replace two No.5 or two No.7 may be used to replace four No.5. The bottom reinforcing steel is to be located not more than 23/4 inches clear distance from the bottom of the lintel. | | | | | | | | | | |
| 2. All bond beams shall have reinforcement in the top in accordance with Section R609.6.2. | | | | | | | | | | |
| 3. Use 4-foot floor span for walls parallel to hollowcore (non-loadbearing). | | | | | | | | | | |
| 4. All the bottom reinforcement in precast lintels may be used to satisfy the continuous bond beam bottom reinforcement requirement. | | | | | | | | | | |
| 5. This table is applicable for all roof dead loads. | | | | | | | | | | |

**(S6153GlitchAS)**

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**R609.6.3.3** Top reinforcement which is in addition to that required in the bond beam over the wall shall extend a minimum of 24 inches (610 mm) past each side of the opening. Top bond beam reinforcement shall be continuous over wall and opening.

**R609.6.3.4** Bottom reinforcing shall extend past each side of the opening a minimum of 24 inches (610 mm) for concrete walls and 4 inches (102 mm) for masonry walls. When using a precast lintel, the reinforcing in the precast lintel shall be included when determining the total amount of bottom reinforcement furnished.

**R609.6.3.5** For masonry walls, a cleanout shall be provided in the cells directly above the ends of the lintel when the reinforcing steel in the bottom of the lintel is more than 22 inches (559 mm) below the top of the bond beam.

***Section R611.2 Applicability limits. Change to read as shown:***

**R611.2 Applicability limits.** The provisions of this section shall apply to the construction of exterior concrete walls for buildings not greater than 60 feet (18 288 mm) in plan dimensions, floors with clear spans not greater than 32 feet (9754 mm) and roofs with clear spans not greater than 40 feet (12 192 mm). Buildings shall not exceed 35 feet (10 668 mm) in mean roof height or two stories in height above-grade. Floor/ceiling dead loads shall not exceed 10 pounds per square foot (479 Pa), roof/ceiling dead loads shall not exceed 15 pounds per square foot (718 Pa) and *attic* live loads shall not exceed 20 pounds per square foot (958 Pa). Roof overhangs shall not exceed 2 feet (610 mm) of horizontal projection beyond the exterior wall and the dead load of the overhangs shall not exceed 8 pounds per square foot (383 Pa).   
  
Walls constructed in accordance with the provisions of this section shall be limited to buildings subjected to a maximum *Vasd*, determined in accordance with Section R301.2.1.3, ~~design wind speed~~ of 130 miles per hour (58 m/s) Exposure B, 110 miles per hour (49 m/s) Exposure C and 100 miles per hour (45 m/s) Exposure D. Walls constructed in accordance with the provisions of this section shall be limited to detached one- and two-family *dwellings* and townhouses.   
  
Buildings that are not within the scope of this section shall be designed in accordance with PCA 100 or [ACI 318.](javascript:vo();)

**(S6155GlitchAS)**

***Tables R611.6(1) – R611.6(4). Change to read as follows:***

**TABLE R611.6(1)** **MINIMUM VERTICAL REINFORCEMENT FOR FLAT ABOVE-GRADE WALLSa,b,c,d,e**

|  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **MAXIMUM Vasd (mph) determined in**  **accordance with Section R301.2.1.3 ~~WIND SPEED~~** | | | **MAXIMUM**  **UNSUPPORTED WALL**  **HEIGHT PER STORY**  **(feet)** | **MINIMUM VERTICAL REINFORCEMENT—BAR SIZE AND SPACING (inches)f, g** | | | | | | | |
| **Nominalh wall thickness (inches)** | | | | | | | |
| **Exposure Category** | | | **4** | | **6** | | **8** | | **10** | |
| **B** | **C** | **D** | **Topi** | **Sidei** | **Topi** | **Sidei** | **Topi** | **Sidei** | **Topi** | **Sidei** |

[no change to rest of table]

**TABLE R611.6(2)** **MINIMUM VERTICAL REINFORCEMENT FOR WAFFLE-GRID ABOVE-GRADE WALLSa,b,c,d,e**

|  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- |
| **MAXIMUM Vasd (mph) determined in**  **accordance with Section R301.2.1.3 ~~WIND SPEED~~** | | | **MAXIMUM**  **UNSUPPORTED WALL**  **HEIGHT PER STORY**  **(feet)** | **MINIMUM VERTICAL REINFORCEMENT—BAR SIZE AND SPACING (inches)f, g** | | | |
| **Nominalh wall thickness (inches)** | | | |
| **Exposure Category** | | | **6** | | **8** | |
| **B** | **C** | **D** | **Topi** | **Sidei** | **Topi** | **Sidei** |

[no change to rest of table]

**TABLE R611.6(3)** **MINIMUM VERTICAL REINFORCEMENT FOR 6-INCH SCREEN-GRID ABOVE-GRADE WALLSa,b,c,d,e**

|  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- |
| **MAXIMUM Vasd (mph) determined in**  **accordance with Section R301.2.1.3 ~~WIND SPEED~~** | | | **MAXIMUM**  **UNSUPPORTED WALL**  **HEIGHT PER STORY (feet)** | **MINIMUM VERTICAL REINFORCEMENT—BAR SIZE AND SPACING (inches)f, g** | |
| **Nominalh wall thickness (inches)** | |
| **Exposure Category** | | | **6** | |
| **B** | **C** | **D** | **Topi** | **Sidei** |

[no change to rest of table]

**TABLE R611.6(4)** **MINIMUM VERTICAL REINFORCEMENT FOR FLAT, WAFFLEAND SCREEN-GRID ABOVE-GRADE WALLS DESIGNED CONTINUOUS WITH FOUNDATION STEM WALLSa,b,c,d, e,k,l**

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **MAXIMUM Vasd (mph) determined in**  **accordance with Section R301.2.1.3 ~~WIND SPEED~~** | | | **HEIGHT OF STEM WALLh, i  (feet)** | **MAXIMUM DESIGN LATERAL  SOIL LOAD (psf/ft)** | **MAXIMUM  UNSUPPORTED  HEIGHT OF ABOVE- GRADE WALL (feet)** | **MINIMUM VERTICAL REINFORCEMENT—BAR SIZE AND SPACING (inches)f, g** | | | | | | |
| **Wall type and nominal thicknessj(inches)** | | | | | | |
| **Exposure Category** | | | **Flat** | | | | **Waffle** | | **Screen** |
| **B** | **C** | **D** | **4** | **6** | **8** | **10** | **6** | **8** | **6** |

[no change to rest of table]

**(S6156 GlitchAS)**

***Tables R611.7(1A), (1B) and (1C). Change to read as shown:***

**TABLE R611.7(1A)** **UNREDUCED LENGTH, *UR*, OF SOLID WALL REQUIRED IN EACH EXTERIOR ENDWALL FOR WIND PERPENDICULAR TO RIDGE ONE STORY OR TOP STORY OF TWO-STORYa,c,d,e,f,g**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **SIDEWALL**  **LENGTH**  **(feet)** | **ENDWALL**  **LENGTH**  **(feet)** | **ROOF**  **SLOPE** | **UNREDUCED LENGTH,*UR*, OF SOLID WALL REQUIRED IN ENDWALLS FOR WIND PERPENDICULAR TO RIDGE (feet)** | | | | | | |
| **Maximum Vasd determined in accordance with Section R301.2.1.3 ~~Basic Wind Speed~~ (mph) Exposure** | | | | | | |
| **85B** | **90B** | **100B** | **110B** | **120B** | **130B** | **Minimumb** |
|  |  |  |  |  |  |

[no change to rest of table]

a. [No change]

b. Tabulated lengths in the "minimum” column are based on the requirements of ~~Section 6.1.4.1 of~~ [ASCE 7](javascript:vo();) that the main wind-force resisting system be designed for a minimum service level force of 10 psf multiplied by the area of the building projected onto a vertical plane normal to the assumed wind direction. Tabulated lengths in shaded cells are less than the "minimum” value. Where the minimum controls, it is permitted to be reduced in accordance with Notes c, d and e. See Section R611.7.1.1.

b. – g. [No change]

**TABLE R611.7(1B) UNREDUCED LENGTH, *UR*,OF SOLID WALL REQUIRED IN EACH EXTERIOR ENDWALL FOR WIND PERPENDICULAR TO RIDGE FIRST STORY OF TWO-STORYa,c,d,e,f,g**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **SIDEWALL LENGTH (feet)** | **ENDWALL**  **LENGTH**  **(feet)** | **ROOF SLOPE** | **UNREDUCED LENGTH,*UR*, OF SOLID WALL REQUIRED IN ENDWALLS FOR WIND PERPENDICULAR TO RIDGE (feet)** | | | | | | |
| **Maximum Vasd determined in accordance with Section R301.2.1.3 ~~Basic Wind Speed~~ (mph) Exposure** | | | | | | |
| **85B** | **90B** | **100B** | **110B** | **120B** | **130B** | **Minimumb** |
|  |  |  |  |  |  |

[No change to rest of table]

1. [No change]
2. Tabulated lengths in the "minimum” column are based on the requirements ~~of Section 6.1.4.1~~ of [ASCE 7](javascript:vo();) that the main wind-force resisting system be designed for a minimum service level force of 10 psf multiplied by the area of the building projected onto a vertical plane normal to the assumed wind direction. Tabulated lengths in shaded cells are less than the "minimum” value. Where the minimum controls, it is permitted to be reduced in accordance with Notes c, d and e. See Section R611.7.1.1.
3. – g. [No change]

**TABLE R611.7(1C)** **UNREDUCED LENGTH, *UR*, OF SOLID WALL REQUIRED IN EACH EXTERIOR SIDEWALL FOR WIND PARALLEL TO RIDGEa,c,d,e,f,g**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **SIDEWALL LENGTH (feet)** | **ENDWALL LENGTH  (feet)** | **ROOF  SLOPE** | **UNREDUCED LENGTH,*UR*, OF SOLID WALL REQUIRED IN ENDWALLS FOR WIND PERPENDICULAR TO RIDGE (feet)** | | | | | | |
| **Maximum Vasd determined in accordance with Section R301.2.1.3 ~~Basic Wind Speed~~ (mph) Exposure** | | | | | | |
| **85B** | **90B** | **100B** | **110B** | **120B** | **130B** | **Minimumb** |
|  |  |  |  |  |  |

[No change to rest of table]

1. [No change]
2. Tabulated lengths in the "minimum” column are based on the requirements ~~of Section 6.1.4.1~~ of [ASCE 7](javascript:vo();) that the main wind-force resisting system be designed for a minimum service level force of 10 psf multiplied by the area of the building projected onto a vertical plane normal to the assumed wind direction. Tabulated lengths in shaded cells are less than the "minimum” value. Where the minimum controls, it is permitted to be reduced in accordance with Notes c, d and e. See Section R611.7.1.1.
3. – g. [No change]

**(S6157 GlitchAS)**

***Section R612.1. Change to read as follows:***

**R612.1 General.** This section prescribes performance and construction requirements for exterior windows and doors installed in walls. Windows and doors shall be installed ~~and flashed~~ in accordance with the fenestration manufacturer’s written installation instructions. Window and door openings shall be flashed in accordance with [Section R703.8.](javascript:Next('./icod_irc_2012_7_sec003_par035.htm');) Written installation instructions shall be provided by the fenestration manufacturer ~~for each window or door.~~

***(S5769 AM R1)* (S6017AS)**

***Section R612.2 Performance. Revise to read as shown:***

**R612.2 Performance.** Exterior windows and doors shall be designed to resist the design wind loads specified in Table R301.2(2) adjusted for height and exposure per Table R301.2(3). For testing required in Sections R612.3 and R612.5, design pressures determined from Table R301.2(2) or ASCE 7 are permitted to be multiplied by 0.6.

**[S5701 AS] (S5953 AM R1)(S6045GlitchAM)(S6182GlitchAM)**

**R612.2.1. Custom doors.** Custom (one-of-a-kind) exterior door assemblies shall be tested by an approved testing laboratory or be engineered in accordance with accepted engineering practices.  **(S6045GlitchAS)**

***Section R612.3 Testing and labeling. Revise to read as shown:***

**R612.3 Testing and labeling.** Exterior windows and doors shall be tested by an *approved* independent testing laboratory, and shall be labeled to indicate compliance with AAMA/WDMA/CSA 101/I.S.2/A440 or TAS 202 (HVHZ shall comply with TAS 202).  Exterior side-hinged doors shall be tested and *labeled* as conforming to AAMA/WDMA/CSA 101/I.S.2/A440 or comply with Section R612.5.Exterior windows and doors shall be labeled with a permanent label, marking, or etching providing traceability to the manufacturer and product. The following shall also be required either on a permanent label or on a temporary supplemental label applied by the manufacturer: information identifying the manufacturer, the product model/series number, positive and negative design pressure rating, product maximum size, glazing thickness, impact-resistance rating if applicable, Florida Product Approval number or Miami-Dade Product Approval number, applicable test standard(s), and approved product certification agency, testing laboratory, evaluation entity or Miami-Dade Product Approval.   
  
The labels are limited to one design pressure rating per reference standard. The temporary supplemental label shall remain on the window or door until final approval by the building official.

**Exceptions:**

1. Door assemblies installed in nonhabitable areas where the door assembly and area are designed to accept water infiltration need not be tested for water infiltration.

2. Door assemblies installed where the overhang (OH) ratio is equal to or more than 1 need not be tested for water infiltration. The overhang ratio shall be calculated by the following equation:

OH ratio = OH Length/OH Height

Where:

OH length = The horizontal measure of how far an overhang over a door projects out from door surface.

OH height = The vertical measure of the distance from the door sill to the bottom of the overhang over a door.

3. Pass-through windows for serving from a single-family kitchen, where protected by a roof overhang of 5 feet (1.5 m) or more shall be exempted from the requirements of the water infiltration test.

4.Decorative glazed openings.

**Glass Strength:** Products tested and labeled as conforming to ANSI/AAMA/NWWDA 101/ I.S.2 or ANSI/AAMA/WDMA/101/ I.S.2/NAFS or AAMA/WDMA/CSA 101/I.S.2/A440 or TAS 202 shall not be subject to the requirements of the Florida Building Code. Determination of load resistance of glass for specific loads of products not tested and certified in accordance with Section R 612.3 shall be designed to comply with ASTM E 1300.

**(S6182 GlitchAM)**

**R612.3.1 Comparative analysis**. Structural wind load design pressures for window and door units ~~smaller~~ other than the size tested in accordance with Section R612.3 shall be permitted to be ~~higher~~ different than the design value of the tested unit provided such ~~higher~~ different pressures are determined by accepted engineering analysis or validated by an additional test of the window or door unit to the different design pressure in accordance with section R612.3. All components of the ~~small~~ alternative size unit shall be the same as the tested or labeled unit~~. Where such calculated design pressures are used, they shall be validated by an additional test of the window unit having the highest allowable design pressure.~~

Exceptions:

i Operable windows and doors rated in this manner shall comply with the following:

1. The frame area of the alternate size unit shall not exceed the frame area of the tested approved unit.

2. Shall vary from the tested approved unit only in width, height or load requirements.

3. Shall not exceed 100 percent of the proportional deflection for fiber stress of the intermediate members of the approved unit.

4. Shall not exceed 100 percent of the concentrated load at the juncture of the intermediate members and the frame of the approved unit.

5. Shall not exceed the air and water infiltration resistance of the tested approved unit.

6. Shall not exceed the maximum cyclic pressure of the tested approved unit when tested per TAS 201 and TAS 203 or ASTM E 1886 and ASTM E 1996 where applicable.

ii. Non-operable windows and doors rated in this manner shall comply with the following:

1. The frame area of the alternate size unit shall not exceed the frame area of the tested approved unit.

2. Shall vary from the tested approved unit only in width, height or load requirements.

3. The maximum uniform load distribution (ULD) of any side shall be equal to the uniform load carried by the side divided by the length of the side.

4. The ULD of any member shall not exceed the ULD of the corresponding member of the tested approved unit.

5. The ULD of each member shall be calculated in accordance with standard engineering analysis.

6. Shall not exceed the air and water infiltration resistance of the tested approved unit.

7. Shall not exceed the maximum cyclic pressure of the tested approved unit when tested per TAS 201 and TAS 203 or ASTM E 1886 and ASTM E 1996 where applicable.

**(S5670 AM R1)(S6045GlitchAS)(S6182 Glitch Wilkes)**

***Section R612.4 Garage doors. Revise R612.4 to read as shown:***

**R612.4 Garage doors.** Garage doors shall be tested for determination of structural performance under uniform static air pressure difference in accordance with ANSI/DASMA 108, ASTM E330 Procedure A, or TAS 202.  For garage doors tested in accordance with ASTM E330, acceptance criteria shall be in accordance with ANSI/DASMA 108.  (HVHZ shall comply with TAS 202.)  Design pressures shall be determined from Table R301.2(4) or ASCE 7.  The design pressures, as determined from ASCE 7, are permitted to be multiplied by 0.6. ~~Garage doors shall be tested in accordance with either ASTM E 330 or ANSI/ DASMA 108, and shall meet the acceptance criteria of ANSI/DASMA 108.~~

**[S5334 AS]**

**R612.4.1 Garage door labeling.** Garage doors shall be labeled with a permanent label provided by the garage door manufacturer. The label shall identify the garage door manufacturer, the garage door model/series number, the positive and negative design pressure rating, indicate impact rated if applicable, the installation instruction drawing reference number, the Florida Product Approval or Miami-Dade Product Approval number if applicable, and the applicable test standards. The required garage door components for an approved garage door assembly may be indicated using a checklist form on the label. If a checklist format is used on the label, the door installer or the garage door manufacturer shall mark the selected components on the checklist that are required to assemble an approved garage door system. The installation instructions shall be provided and available on the job site. **[S5329 AS]**

***Section R612.6.1 Fenestration testing and labeling. Change to read as shown:***

**R612.6.1 Fenestration testing andlabeling.** Reserved.~~Fenestration shall be tested by an approved independent laboratory, listed by an approved entity, and bear a label identifying manufacturer, performance characteristics, and approved inspection agency to indicate compliance with the requirements of~~ ~~the following specification:~~

1. ~~ASTM E 1886 and ASTM E 1996; or~~
2. ~~AAMA 506.~~

**(Post-Glitch Comment S2 AS)**

***Section R612.8.Mullions. Add new section to read as shown:***

**R612.8.4 Masonry Rough Openings.** Masonry rough opening dimensions shall be within the tolerances specified at Section R606.16 and in addition shall provide for a window perimeter sealant joint a maximum of ¼ inches in width.

**[S5960 AS]**

***Section R612.9. Add a new section to read as shown:***

**R612.9 ~~R612.3.2~~ Door components.** Door components evaluated by an approved product evaluation entity, certification agency, testing laboratory or engineer may be interchangeable in exterior door assemblies provided that the door component(s) provide equal or greater structural performance as demonstrated by accepted engineering practices.

**(S6046Glitch AS)**

**R612.9.1 Optional exterior door component testing**. With the exception of HVHZ, exterior side-hinged door assemblies not covered by Section R612.6 shall be permitted to have the option to have the components of the assembly tested and rated for structural integrity in accordance with the following ANSI A250.13

Following the structural testing of exterior door components, there shall be no permanent deformation of any perimeter frame or panel member in excess of 0.4 percent of its span after the load is removed. After each specified loading, there shall be no glass breakage, permanent damage to fasteners, hardware parts, or any other damage that causes the door to be inoperable, as applicable.

**[S5722 AS]**

|  |
| --- |
| ***Section R612.10. Add a new section to read as shown:***  **R612.10 Flashing, sealants and weatherstripping.** Flashing and sealants for exterior windows and doors shall comply with Section R703.8 |
| **[S5956 AS]** |

***Section R613.2 Applicability limits. Change to read as follows:***

**R613.2 Applicability limits.** The provisions of this section shall control the construction of exterior structural insulated panel walls and interior load-bearing structural insulated panel walls for buildings not greater than 60 feet (18 288 mm) in length perpendicular to the joist or truss span, not greater than 40 feet (12 192 mm) in width parallel to the joist or truss span and not greater than two stories in height with each wall not greater than 10 feet (3048 mm) high. All exterior walls installed in accordance with the provisions of this section shall be considered as load-bearing walls. Structural insulated panel walls constructed in accordance with the provisions of this section shall be limited to sites subjected to a maximum Vasd, determined in accordance with Section R301.2.1.3, ~~design wind speed~~ of 120 miles per hour (54 m/s), Exposure A or B or 110 mph (49 m/s) Exposure C, and a maximum ground snow load of 70 pounds per foot (3.35 kPa), and Seismic Design Categories A, B, and C.

**(S6158GlitchAS)**

***Section R613.5 Wall construction. Change to read as follows:***

**R613.5 Wall construction.** Exterior walls of SIP construction shall be designed and constructed in accordance with the provisions of this section and Tables R613.5(1) andR613.5(2) and Figures R613.5(1) through R613.5(5). SIP walls shall be fastened to other wood building components in accordance with Section R602~~Tables R602.3(1) through R602.3(4)~~.

Framing shall be attached in accordance with Section R602~~TableR602.3(1)~~ unless otherwise provided for in Section R613.

**(S6159GlitchAS)**

***Tables R613.5(1) and R613.5(2). Change header only to read as shown:***

**TABLE R613.5(1)**

**MINIMUM THICKNESS FOR SIP WALL**

**SUPPORTING SIP LIGHT-FRAME ROOF ONLY (inches)**

|  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- |
| **MAXIMUM** **Vasd DETERMINED IN**  **ACCORDANCE WITH**  **SECTION R301.2.1.3**  ~~WIND SPEED (3-second gust)~~ | **BUILDING WIDTH (feet)** | | | | |
| **24** | **28** | **32** | **36** | **40** |

(remainder of table unchanged)

**TABLE R613.5(2)**

**MINIMUM THICKNESS FOR SIP WALLS SUPPORTING SIP**

**OR LIGHT-FRAME ONE STORY AND ROOF (inches)**

|  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- |
| **MAXIMUM** **Vasd DETERMINED IN ACCORDANCE WITH SECTION R301.2.3**  ~~WIND SPEED (3-second gust)~~ | **BUILDING WIDTH (feet)** | | | | |
| **24** | **28** | **32** | **36** | **40** |

(remainder of table unchanged)

**(S6162GlitchAS)**

***Figures R613.5(3), R613.5(4), R613.5(5) and R613.5.1. Replace in their entirety with the following:***

|  |
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| For SI: 1 inch = 25.4 mm. |

**FIGURE R613.5(3) TRUSSED ROOF TO TOP PLATE CONNECTION**

|  |
| --- |
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| For SI: 1 inch = 25.4 mm. |
| Note: Figures illustrate SIP-specific attachment requirements. Other connections shall be made in accordance with Section R602 as appropriate. |

**FIGURE R613.5(4) SIP WALL TO WALL PLATFORM FRAME CONNECTION**

|  |
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| For SI: 1 inch = 25.4 mm. |
| Note: Figures illustrate SIP-specific attachment requirements. Other connections shall be made in accordance with Section R602 as appropriate. |

**FIGURE R613.5(5) SIP WALL TO WALL BALLOON FRAME CONNECTION (I-Joist floor shown for Illustration only)**

|  |
| --- |
|  |

|  |
| --- |
| For SI: 1 inch = 25.4 mm. |
| Notes: |
| 1. Top plates shall be continuous over header. |
| 2. Lower 2x top plate shall have a width equal to the SIP core width and shall be recessed into the top edge of the panel. Cap plate shall be placed over the recessed top plate and shall have a width equal to the SIPs width. |
| 3. SIP facing surfaces shall be nailed to framing and cripples with 8d common or galvanized box nails spaced 6 inches on center. |
| 4. Galvanized nails shall be hot-dipped or tumbled. Framing shall be attached in accordance to Section R602 unless otherwise provide for in Section R613. |

**FIGURE R613.5.1 SIP WALL FRAMING CONFIGURATION**

***(S6164GlitchAS)***

***Section R613.5.3 Wall bracing. Change to read as follows*:**

**R613.5.3 Wall bracing.** SIP walls used for wall bracing shall be designed for wind loads in accordance with Section R301.1 or Section R602.~~SIP walls shall be braced in accordance with Section R602.10.~~ SIP walls shall be considered continuous wood structural panel sheathing for purposes of computing required bracing. SIP ~~walls shall meet the requirements of Section R602.10.4 except that SIPs~~ corners shall be fabricated as shown in Figure R613.9. When SIP walls are used for wall bracing, the SIP bottom plate shall be attached to wood framing below in accordance with Section R602.3~~Table R602.3(1)~~.

**(S6160GlitchAS)**

***Section R613.10 Headers. Change to read as follows:***

**R613.10 Headers.** SIP headers shall be designed and constructed in accordance with Table R613.10 and FigureR613.5.1. SIPs headers shall be continuous sections without splines. Headers shall be at least 11 7/8 inches (302 mm) deep. Headers longer than 4 feet (1219 mm) shall be constructed in accordance with Section R602 ~~R602.7~~.

**R613.10.1 Wood structural panel box headers.** Wood structural panel box headers shall be allowed where SIP headers are not applicable. Wood structural panel box headers shall be constructed in accordance with Section R602 Figure ~~R602.7.2 and Table R602.7.2~~.

**(S6161GlitchAS)**

***Section R614. Add a new section on Combined Concrete, Masonry, or ICF and Wood Exterior Wall Construction to read as follows:***

**SECTION R614**

**COMBNED CONCRETE, MASONRY, OR ICF AND**

**WOOD EXTERIOR WALL CONSTRUCTION**

**R614.1 General.** This section prescribes construction requirements for individual building elements where one or more exterior walls above the foundation contain multiple construction types. Where specific construction requirements are not specifically prescribed in this section, the requirements in the applicable sections of each material shall govern.

**R614.2 Concrete, masonry, or ICF first story wood frame second and third story.**

**R614.2.1 Foundation.** The foundation system shall be designed in accordance with Chapter 4.

**R614.2.2 First-story construction.** The concrete, masonry or ICF first-story shall be in accordance with Chapter 6 for the applicable first-story construction method**.**

**R614.2.3 Floor systems.** The second- and third-story floor system shall be in accordance with Chapter 5.

**R614.2.4 Second- and third-story construction.** The second-and third-story walls, ceilings and roof shall be in accordance with the appropriate sections in Chapters 6, 8, and 9.

**R614.2.5 Shear wall connections.** Second-story shearwalls shall be connected to first-story walls in accordance with Tables 3.2A, 3.2B, 3.2C, A-3.24, 4 3.28 or A-3.2C of the AF&PA Wood Frame Construction Manual for One- and Two-Family Dwellings as applicable.

**R614.3 Wood frame gable endwalls above concrete, masonry, or ICF walls.** This condition is not permitted unless there is a ceiling diaphragm in accordance with Figures 3.7a and 3.15 of the AF&PA Wood Frame Construction Manual for One- and Two-Family Dwellings.

**R614.3.1 Gable construction.** Gable construction shall be in accordance with the AF&PA Wood Frame Construction Manual for One- and Two-Family Dwellings.

**R614.3.2 Wall construction. Concrete, masonry or ICF wall construction shall be in accordance with Chapter 6.**

**R614.3.3 Gable connection.** The connection of the wood frame gable endwall to the concrete, masonry or ICF wall shall be in accordance with Figures R614.3(1) and R614.3(2), or Figure R609.4.

**[S6017 AS]**

***Tables R614.3(1) and R614.3(2). Add to read as shown:***

|  |
| --- |
|  |
| **Note:** For Table 207B, [Section 207.3.1](http://ecodes.cyberregs.com/cgi-exe/cpage.dll?pg=x&rp=/pseudo.htm&sid=2013062006295122434&aph=0&cid=iccf&uid=iccf0002&clrA=005596&clrV=005596&clrX=005596&ref=/indx/ST/fl/st/b400v10/st_fl_st_b400v10_6.htm&pseudo=UN1%2C%2CST%2CSTF2012021013342915527%2Cb=207%2C(3)(1)#b=207~(3)(1)) and [Section 208.6.4](http://ecodes.cyberregs.com/cgi-exe/cpage.dll?pg=x&rp=/pseudo.htm&sid=2013062006295122434&aph=0&cid=iccf&uid=iccf0002&clrA=005596&clrV=005596&clrX=005596&ref=/indx/ST/fl/st/b400v10/st_fl_st_b400v10_6.htm&pseudo=UN1%2C%2CST%2CSTF2012021013342915527%2Cb=208%2C(6)(4)#b=208~(6)(4))(1), see IBHS Guidelines for Hurricane Resistant Residential Construction. Ceiling diaphragms where provided shall comply with IBHS [Section 207.2.](http://ecodes.cyberregs.com/cgi-exe/cpage.dll?pg=x&rp=/pseudo.htm&sid=2013062006295122434&aph=0&cid=iccf&uid=iccf0002&clrA=005596&clrV=005596&clrX=005596&ref=/indx/ST/fl/st/b400v10/st_fl_st_b400v10_6.htm&pseudo=UN1%2C%2CST%2CSTF2012021013342915527%2Cb=207%2C(2)#b=207~(2)) | |

**FIGURE R614.3(1) DIRECT TRUSS TO CONCRETE, MASONRY OR ICF WALL CONNECTION FOR GYPSUM BOARD CEILING DIAPHRAGM**

|  |
| --- |
|  |

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| --- |
| **Note:** For Table 207B and [Section 207.3.1](http://ecodes.cyberregs.com/cgi-exe/cpage.dll?pg=x&rp=/pseudo.htm&sid=2013062006295122434&aph=0&cid=iccf&uid=iccf0002&clrA=005596&clrV=005596&clrX=005596&ref=/indx/ST/fl/st/b400v10/st_fl_st_b400v10_6.htm&pseudo=UN1%2C%2CST%2CSTF2012021013342915527%2Cb=207%2C(3)(1)#b=207~(3)(1)), see IBHS Guidelines for Hurricane Resistant Residential Construction. Ceiling diaphragms where provided shall comply with IBHS [Section 207.2.](http://ecodes.cyberregs.com/cgi-exe/cpage.dll?pg=x&rp=/pseudo.htm&sid=2013062006295122434&aph=0&cid=iccf&uid=iccf0002&clrA=005596&clrV=005596&clrX=005596&ref=/indx/ST/fl/st/b400v10/st_fl_st_b400v10_6.htm&pseudo=UN1%2C%2CST%2CSTF2012021013342915527%2Cb=207%2C(2)#b=207~(2)) |

**FIGURE R614.3(2) DIRECT TRUSS TO CONCRETE, MASONRY OR ICF WALL FOR GYPSUM BOARD CEILING DIAPHRAGM**

**(S6165GlitchAS)**

***Section R615. Add a new major section on Impact-Resistant Coverings to read as shown:***

**SECTION R615  
IMPACT-RESISTANT COVERINGS**

**R615.1** Impact resistant coverings shall be tested at 1.5 times the design pressure (positive or negative) expressed in pounds per square feet as determined by the *Florida Building Code, Building* Section 1609 for which the specimen is to be tested. The design pressures, as determined from Section 1609 of the *Florida Building Code, Building* or ASCE 7, are permitted to be multiplied by 0.6.

**R615.1.1** Impact resistant coverings shall be labeled in accordance with the provisions of Section R615.

**R615.2. Labels.** A permanent label shall be provided by the product approval holder on all impact resistant coverings.

**R615.2.1** The following information shall be included on the labels on impact resistant coverings:

1. Product approval holder name and address.

2. All applicable methods of approval. Methods of approval include, but, are not limited to Miami-Dade NOA; Florida Building Commission, TDI Product Evaluation; ICC-ES.

3. The test standard or standards specified at Section R301.2.1.2, including standards referenced within the test standards specified at Section R301.2.1.2 used to demonstrate code compliance.

4. For products with a Florida Product Approval Number or a Miami-Dade County Building and Neighborhood Compliance Department Notice of Acceptance Number (NOA), such numbers shall be included on the label.

**R615.3 Location of label.** The location of the label on the impact resistant covering shall be as follows:

1. Accordions: Bottom of the locking bar or center mate facing the exterior or outside.

2. Rollup: On the bottom of the hood facing the exterior or outside or on the bottom slat facing the exterior or outside.

3. Bahama Awning or Colonial Hinged: On the bottom, placed on the back of the shutter.

4. Panels: For metal and plastic panels the label may be embossed or printed spaced not more than every three (3) lineal feet on each panel. The label shall be applied by the holder of the product approval and shall face the exterior or outside.

5. Framed products: The label shall be on the side or bottom facing the exterior or outside.

6. Labels on all other products shall face the exterior or outside.

**R615.4 Installation.** All impact resistant coverings shall be installed in accordance with the manufacturer’s installation instructions and in accordance with the product approval. Installation instructions shall be provided and shall be available to inspection personnel on the job site. Opening protection components, fasteners, and other parts evaluated by an approved product evaluation entity, certification agency, testing laboratory, architect, or engineer and approved by the holder of the product approval may be interchangeable in opening protection assemblies provided that the opening protection component(s) provide equal or greater structural performance and durability as demonstrated by testing in accordance with approved test standards.

**[S5723 AS]**

**CHAPTER 7**

**WALL COVERINGS**

***Section R701.1 Application. Revise to add exception as shown:***

**R701.1 Application.** The provisions of this chapter shall control the design and construction of the interior and exterior wall covering for all buildings.

**Exception:** Buildings and structures located within the High-Velocity Hurricane Zone shall comply with the provisions of Chapter 44.

***Section R702.3.5Application . Change to read as shown:***

**R702.3.5 Application.** Maximum spacing of supports and the size and spacing of fasteners used to attach gypsum board shall comply with Table R702.3.5. Gypsum sheathing shall be attached to exterior walls in accordance with Section R602~~Table R602.3(1)~~. Gypsum board shall be applied at right angles or parallel to framing members. All edges and ends of gypsum board shall occur on the framing members, except those edges and ends that are perpendicular to the framing members. Interior gypsum board shall not be installed where it is directly exposed to the weather or to water.

**(S6166GlitchAS)**

***Section R703.1.3. Add a new section to read as shown:***

**R703.1.3 Load resistance.** All exterior walls, wall coverings and soffits shall be capable of resisting the design pressures specified in Table R301.2(2) for walls.

**[S5704 AS]**

***Section R703.3. Add a new section to read as shown:***

**R703.3.3 Attachment.** Wood, hardboard and wood structural panel siding shall be attached in accordance with the AF&PA WFCM .

**R703.3.4 Minimum thickness.**  Wood, hardboard and wood structural panel siding shall be of the minimum thickness specified in the AF&PA WFCM .

**[S5709 AS]**

***Section R703.4 Attachments. Revise to read as shown:***

**R703.4 Attachments.** Unless specified otherwise, all wall coverings shall be securely fastened ~~in accordance with Table R703.4 or~~ with ~~other~~ *approved* aluminum, stainless steel, zinc-coated or other *approved* corrosion-resistive fasteners. Where wind pressures determined in accordance with Table R301.2(2) ~~the basic wind speed in accordance with Figure R301.2(4)A is 110 miles per hour (49 m/s) or higher, the attachment of wall coverings shall be designed to resist the component and cladding loads specified in Table R301.2(2)~~, adjusted for height and exposure in accordance with Table R301.2(3) do not exceed 30 psf, wall coverings are permitted to be installed in accordance with Table R703.4.

**[S5705 AS]**

***Table R703.4 Weather-Resistant Siding Attachment and Minimum Thickness. Revise to read as shown:***

**TABLE R703.4**

**WEATHER-RESISTANT SIDING ATTACHMENT AND MINIMUM THICKNESS**

|  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **SIDING MATERIAL** | **NOMINAL THICKNESSa (inches)** | **JOINT TREAT-MENT** | **WATER-RESISTIVE BARRIER REQUIRED** | | **Type of Supports for the Siding Material and Fastenersb,c,d** | | | | | | |
| **Wood or wood structural panel sheathing into stud** | **Fiber- board sheathing into stud** | **Gypsum sheathing into stud** | **Foam plastic sheathing into stud** | **Direct to studs** | **Number or spacing of fasteners** | |
| Horizontal aluminum | [No change to IRC table] | | | | | | | | | | |
| Anchored veneer: brick, concrete, masonry or stone | [No change to IRC table] | | | | | | | | | | |
| Adhered veneer: concrete, stone or masonry | [No change to IRC table] | | | | | | | | | | |
| Hardboardk  Panel siding-vertical | See Section R703.3.3 ~~7/16~~ | -- | Yes | ~~Note m~~ | | ~~Note m~~ | ~~Note m~~ | ~~Note m~~ | ~~Note m~~ | ~~6” panel edges~~  ~~12” inter.sup.~~~~n~~ | |
| See Section R703.3.4 | | | | | | | |
| Hardboardk Lap-siding-horizontal | See Section R703.3.3 ~~7/16~~ | Note q ~~p~~ | Yes | ~~Note o~~ | | ~~Note o~~ | ~~Note o~~ | ~~Note o~~ | ~~Note o~~ | ~~Same as stud spacing 2 per bearing~~ | |
| See Section R703.3.4 | | | | | | | |
| Steelh | 29 ga. | Lap | Yes | 0.113 nail  1 ¾”  ~~Staple 1 ¾”~~ | | 0.113 nail  2 ¾”  ~~Staple 2 ½”~~ | 0.113 nail  2 ½ ”  ~~Staple 2¼”~~ | 0.113 nail  1 ¾”  ~~Staple~~~~v~~ | Not allowed | Same as stud spacing | |
| ~~Particleboard panels~~ | ~~3/8-1/2~~ | ~~–~~ | ~~Yes~~ | ~~6d box nail (2” x 0.099”)~~ | | ~~6d box nail (2” x 0.099”)~~ | ~~6d box nail (2” x 0.099”)~~ | ~~box nail~~ | ~~6d box nail (2” x 0.099”)~~  ~~3/8 not allowed~~ | ~~6” panel edges~~  ~~12” inter. Sup.~~~~o~~ | |
| ~~5/8~~ | ~~–~~ | ~~Yes~~ | ~~6d box nail (2” x 0.099”)~~ | | ~~8d box nail (2 1/2” x 0.113”)~~ | ~~8d box nail (2 1/2” x 0.113”)~~ | ~~box nail~~ | ~~6d box nail (2” x 0.099”)~~ |
| Wood structural panel ANSI/APA-PRP 210 sidingi (exterior grade) | [No change] | [No change] | [No change] | [No change] | | [No change] | [No change] | [No change] | [No change] | [No change] | |
| Wood structural panel lapsiding | [No change] | [No change] | [No change] | [No change] | | [No change] | [No change] | [No change] | [No change] | [No change] | |
| Vinyl sidingm | 0.035 | Lap | Yes | See Section ~~R703.3.4~~ R703.11. | | | | | | | |
| ~~0.120 nail (shank) with a .313 head or 16-gage staple with 3/8 to ½–inch crown~~~~y,z~~ | | ~~0.120 nail (shank) with a .313 head or 16-gage staple with 3/8 to ½–inch crown~~~~y,z~~ | ~~0.120 nail (shank) with a .313 head or 16-gage staple with 3/8 to ½–inch crown~~~~y,z~~ | ~~0.120 nail (shank) with a .313 head or 16-gage staple with 3/8 to ½–inch crown~~~~y,z~~ | ~~Not allowed~~ | | ~~16” on center.....oor test report~~ |
| Woodj rustic, drop | 3/8 Min | Lap | Yes | Fastener penetration into stud-1” | | | | | 0.113 nail 2 ½” ~~Staple-2~~” | | Face nailing up to 6” widths, 1 nail per bearing; 8” widths and over, 2 nails per bearing |
| Shiplap | 19/32 Average |
| Bevel | 7/16 |
| Butt tip | 3/16 |
| Fiber cement panel sidingr | [No change] | [No change] | [No change] | [No change] | | [No change] | [No change] | [No change] | [No change] | | [No change] |
| Fiber cement lap sidingr | [No change] | [No change] | [No change] | [No change] | | [No change] | [No change] | [No change] | ~~6d corrosion-resistant nail~~~~w~~ ~~or…nail~~~~r~~ | | Note w~~t~~ |

For SI: 1 inch = 25.4 mm.

a-b No change

c. Reserved.

d-i No change.

m. Reserved.

n. No change

o. Reserved.

P – z No change .

**[S5706 AS]**

***Section R703.5.3 Attachment. Revise to read as shown:***

**R703.5.3 Attachment.** Wood shakes and shingles, and attachment and supports shall be capable of resisting the wind pressures determined in accordance with Table R310.2(2).Where wind pressures determined in accordance with Table R301.2(2) do not exceed 30 psf, ~~E~~ each shake or shingle shall be held in place by two hot-dipped zinc-coated, stainless steel, or aluminum nails or staples. The fasteners shall be long enough to penetrate the sheathing or furring strips by a minimum of 1/2 inch (13 mm) and shall not be overdriven.  Where pressures determined in accordance with Table R301.2(2) exceed 30 psf, the attachment shall be designed to resist the prescribed wind pressures.

**R703.5.3.1 Staple attachment. Reserved** ~~Staples shall not be less than 16 gage and shall have a crown width of not less than 7/16 inch (11 mm), and the crown of the staples shall be parallel with the butt of the shake or shingle. In single-course application, the fasteners shall be concealed by the course above and shall be driven approximately 1 inch (25 mm) above the butt line of the succeeding course and 3/4 inch (19 mm) from the edge. In double-course applications, the exposed shake or shingle shall be face-nailed with two casing nails, driven approximately 2 inches (51 mm) above the butt line and 3/4 inch (19 mm) from each edge. In all applications, staples shall be concealed by the course above. With shingles wider than 8 inches (203 mm) two additional nails shall be required and shall be nailed approximately 1 inch (25 mm) apart near the center of the shingle.~~

**[S5710 AS]**

***Section R703.7 Stone and masonry veneer, general. Revise to read as shown:***

**R703.7 Stone and masonry veneer, general.** Stone and masonry veneer shall be installed in accordance with this chapter, Table R703.4 and Figure R703.7. These veneers installed over a backing of wood or cold-formed steel shall be limited to the first *story* above-grade plane and shall not exceed 5 inches (127 mm) in thickness. See Section R602.3 ~~R602.10~~ for wall bracing requirements for masonry veneer for wood-framed construction and Section R301.2.1 ~~R603.9.5~~ for wall bracing requirements for masonry veneer for cold-formed steel construction.  The provisions of this section are limited to areas where the Vasd as determined in accordance with Section R301.2.1.3, is equal to or less than 130 mph.

**[S5711 AS](S6167GlitchAS)**

***Section R703.7.4.1 Size and spacing. Revise to read as shown:***

**R703.7.4.1 Size and spacing.** Veneer ties, if strand wire, shall not be less in thickness than No. 9 U.S. gage [(0.148 inch) (4 mm)] wire and shall have a hook embedded in the mortar joint, or if sheet metal, shall be not less than No. 22 U.S. gage by [(0.0299 inch) (0.76 mm)] 7/8 inch (22 mm) corrugated. Each tie shall support not more than 2.67 square feet (0.25 m2) of wall area and shall be spaced not more than 32 inches (813 mm) on center horizontally and 24 inches (635 mm) on center vertically.

**Exception:**

1.    In Seismic Design Category D0, D1 or D2 or townhouses in Seismic Design Category C or in wind areas of more than 30 pounds per square foot pressure (1.44 kPa), each tie shall support not more than 2 square feet (0.2 m2) of wall area.

2.    Where the Vasd as determined in accordance with Section R301.2.1.3 exceeds 110 mph (176.99 km/h) or is less than or equal to 130 mph (208 km/h), each tie shall support not more than 1.8 square feet (0.167 m2) of wall area and anchors shall be spaced at a maximum 18 inches (457 mm) horizontally and vertically.

**[S5711 AS]**

***Section R703.8 Flashing. Revise to read as shown:***

**R703.8 Flashing.** *Approved* corrosion-resistant flashing shall be applied shingle-fashion in a manner to prevent entry of water into the wall cavity or penetration of water to the building structural framing components. Self-adhered membranes used as flashing shall comply with AAMA 711. All exterior fenestration products shall be sealed at the juncture with the building wall with a sealant complying with [AAMA 800](javascript:vo();) or [ASTM C 920](javascript:vo();) Class 25 Grade NS or greater for proper joint expansion and contraction, [ASTM C 1281](javascript:vo();), [AAMA 812](javascript:vo();), or other approved standard as appropriate for the type of sealant. The flashing shall extend to the surface of the exterior wall finish. *Approved* corrosion-resistant flashings shall be installed at all of the following locations:

1. Exterior window and door openings. Flashing at exterior window and door openings shall extend to the surface of the exterior wall finish or to the water-resistive barrier for subsequent drainage. Flashing at exterior window and door openings shall be installed in accordance with one or more of the following:

1.1. The fenestration manufacturer’s installation and flashing instructions, or for applications not addressed in the fenestration manufacturer’s instructions, in accordance with the flashing manufacturer’s instructions. Where flashing instructions or details are not provided, pan flashing shall be installed at the sill of exterior window and door openings. Pan flashing shall be sealed or sloped in such a manner as to direct water to the surface of the exterior wall finish or to the water-resistive barrier for subsequent drainage. Openings using pan flashing shall also incorporate flashing or protection at the head and sides.

1.2. In accordance with the flashing design or method of a registered design professional.

1.3. In accordance with other approved methods.

1.4. In accordance with FMA/AAMA 100, FMA/AAMA 200, FMA/WDMA 250, or FMA/AAMA/WDMA 300.

 2. At the intersection of chimneys or other masonry construction with frame or stucco walls, with projecting lips on both sides under stucco copings.

3. Under and at the ends of masonry, wood or metal copings and sills.

4. Continuously above all projecting wood trim.

5. Where exterior porches, decks or stairs attach to a wall or floor assembly of wood-frame construction.

6. At wall and roof intersections.

7. At built-in gutters.

**[S5750-R1 AM]**

***Change R703.9 as shown:***

**R703.9 Exterior insulation and finish system (EIFS)/EIFS with drainage.** Exterior Insulation and Finish System (EIFS) shall be designed or tested to meet the wind pressures specified in Table R301.2(2) and shall comply with this chapter and Sections R703.9.1 and R703.9.3. EIFS with drainage shall comply with this chapter and Sections R703.9.2, R703.9.3 and R703.9.4.

***Section R703.11.2 Foam plastic sheathing. Revise to read as shown:***

**R703.11.2 Foam plastic sheathing.** Vinyl siding used with foam plastic sheathing shall be installed in accordance with Section ~~R703.11.2.1,  or~~ R703.11.2.2~~,~~ or R703.11.2.3.

**Exception:** Where the foam plastic sheathing is applied directly over wood structural panels, 1fiberboard, gypsum sheathing or other *approved* backing capable of independently resisting the design wind pressure, the vinyl siding shall be installed in accordance with Section R703.11.1.

**R703.11.2.1 Basic wind speed not exceeding 90 miles per hour and Exposure Category B.** Reserved.~~Where the basic wind speed does not exceed 90 miles per hour (40 m/s), the Exposure Category is B and gypsum wall board or equivalent is installed on the side of the wall opposite the foam plastic sheathing, the minimum siding fastener penetration into wood framing shall be 11/4 inches (32 mm) using minimum 0.120-inch diameter nail (shank) with a minimum 0.313-inch diameter head, 16 inches on center. The foam plastic sheathing shall be minimum 1/2-inch-thick (12.7 mm) (nominal) extruded polystyrene per ASTM C 578, 1/2-inch-thick (12.7 mm) (nominal) polyisocyanurate per ASTM C 1289, or 1- inch-thick (25 mm) (nominal) expanded polystyrene per ASTM C 578.~~

**R703.11.2.2  Design wind pressure rating. ~~Basic wind speed exceeding 90 miles per hour or Exposure Categories C and D.~~** ~~Where the basic wind speed exceeds 90 miles per hour (40 m/s) or the Exposure Category is C or D, or all conditions of Section R703.11.2.1 are not met, the adjusted design pressure rating for the assembly shall meet or exceed the loads listed in Tables R301.2(2) adjusted for height and exposure using Table R301.2(3).~~ ~~Where the V~~~~ult~~ ~~wind speed does not exceed 140 mph,~~ The design wind pressure rating of the vinyl siding for installation over solid sheathing as provided in the vinyl siding manufacturer’s product specifications shall be adjusted for installation over foam plastic sheathing for the following wall assembly conditions:

1.  Ultimate wind speeds, Vult, greater than 115 mph and less than 130 mph:

~~1~~ a. For wall assemblies with foam plastic sheathing on the exterior side and gypsum wall board or equivalent on the interior side of the wall, the vinyl siding’s design wind pressure rating shall be multiplied by 0.39.

~~2~~ b. For wall assemblies with foam plastic sheathing on the exterior side and no gypsum wall board or equivalent on the interior side of wall, the vinyl siding’s design wind pressure rating shall be multiplied by 0.27.

The adjusted design pressure rating for the assembly shall meet or exceed the loads listed in Tables R301.2(2) adjusted for height and exposure using Table R301.2(3). Design pressures in Table R301.2(2) are permitted to be multiplied by 0.6.

2. Ultimate wind speeds, Vult, greater than 130 mph and less than 140 mph:

a. The vinyl siding’s design wind pressure rating shall be multiplied by 0.27.

The adjusted design pressure rating for the assembly shall meet or exceed the loads listed in Tables R301.2(2) adjusted for height and exposure using Table R301.2(3). Design pressures in Table R301.2(2) are permitted to be multiplied by 0.6.

3 Ultimate wind speeds, Vult, equal to or greater than 140 mph:

Foam sheathing shall be installed over a sheathing material designed and attached to separately resist 100% of the wind load.

**R703.11.2.3 Manufacturer specification.** [No change to text]

**[S5947 AS](S6269GlitchAS)**

**CHAPTER 8**

**ROOF-CEILING CONSTRUCTION**

***Add an exception to R801.1 as shown:***

**R801.1 Application.** The provisions of this chapter shall control the design and construction of the roof-ceiling system for all buildings (see Section R301.2.1.1).

**Exception:** Buildings and structures located within the High-Velocity Hurricane Zone shall comply with the provisions of Chapter 44.

***Section R802.1 Identification. Revise to read as shown:***

**R802.1 General Requirements.** Roof and ceiling framing of wood construction shall be designed and constructed in accordance with the provisions of this Section.

**R802.1.1 [IRC802.1] Identification.** *[No change to text]*

**R802.1.1.1 [IRC802.1.1] Blocking.** *[No change to text]*

**R802.1.7**[I**RC802.10] Wood trusses.**

**R802.1.7.1 [IRC802.10.1] Truss design drawings.** Truss design drawings, prepared in conformance with Section R802.1.7.1, ~~R802.10.1~~ shall be provided to the building official and approved prior to installation. Truss design drawings shall include, at a minimum, the information specified below. Truss design drawing shall be provided with the shipment of trusses delivered to the jobsite.

1. Ultimate design wind speed, Vult, and exposure category.

2. Slope or depth, span and spacing.

3. Location of all joints.

4. Required bearing widths.

5. Design loads as applicable.

      5.1 Top chord live load (as determined from Section R301.6).

      5.2 Top chord dead load.

      5.3 Bottom chord live load.

      5.4 Bottom chord dead load.

      5.5 Concentrated loads and their points of application.

      5.6 Controlling wind and earthquake loads.

6. Adjustments to lumber and joint connector design values for conditions of use.

7. Each reaction force and direction.

8. Joint connector type and description (e.g., size, thickness or gauge) and the dimensioned location of each joint connector except where symmetrically located relative to the joint interface.

9. Lumber size, species and grade for each member.

10. Connection requirements for:

      10.1 Truss to truss girder.

      10.2 Truss ply to ply.

      10.3 Field splices.

11. Calculated deflection ratio and/or maximum description for live and total load.

12. Maximum axial compression forces in the truss members to enable the building designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces shall be shown on the truss design drawing or on supplemental documents.

|  |
| --- |
| 13. Required permanent truss member bracing location.  **R802.1.7.2 ~~R802.10.2~~ Design.** Wood trusses shall be designed in accordance with accepted engineering practice. The design and manufacture of metal-plate-connected wood trusses shall comply with ANSI/TPI 1. The truss design drawings shall be prepared by a registered professional where required by Florida Statutes ~~the statutes of the jurisdiction in which the project is to be constructed in accordance with Section R106.1~~.  **R802.1.7.2.1 ~~R802.10.2.~~ Applicability limits. Reserved.**  **R802.1.7.3 ~~R802.10.3~~ Bracing.** *[No other change to IRC text]*  **R802.1.7.4 ~~R802.10.4~~ Alterations to trusses.** *[No other change to IRC text]*  **R802.1.7.5 ~~R802.10.5~~   Truss to wall connection.** Trusses shall be connected to wall plates by the use of approved connectors having a resistance to design uplift, lateral and shear forces. Trusses shall be installed in accordance with the manufacturer’s design and specifications. ~~For roof assemblies subject to wind uplift pressures of 20 pounds per square foot (0.958 kN/m2) or greater, as established in Table R301.2(2), adjusted for height and exposure per Table R301.2(3), see Section R802.2.9.~~  **R802.1.8  ~~R802.7~~ Cutting and notching.** *[No other change to IRC text]*  **R802.1.8.1 ~~R802.7.1~~ Sawn lumber.** *[No other change to IRC text]*  **R802.1.8.1.1 ~~R802.7.1.1~~ Cantilevered portions of rafters.** *[No other change to IRC text]*  **R802.1.8.1.2  ~~R802.7.1.2~~ Ceiling joist taper cut.** *[No other change to IRC text]*  **R802.1.8.2 ~~R802.7.2~~Engineered wood products.** [No other change to IRC text].  **R802.2 Design and construction.**Roof-ceilings of light-frame wood construction shall be designed and constructed in accordance with the provisions of Section R301.2.1.1 and Section R802.1.  **Exceptions:**  1.      For rafter connections to the top plate, straps and/or clips shall extend such that the top nail is within 1 inch of the top of the rafter with one or more nails installed on the opposite side of the rafter.  2.      Roof sheathing shall be at a minimum attached in accordance with Section R803.2.3.  ~~The framing details required in Section R802 apply to roofs having a minimum slope of three units vertical in 12 units horizontal (25-percent slope) or greater. Roof-ceilings shall be designed and constructed in accordance with the provisions of this chapter and Figures R606.11(1), R606.11(2) and R606.11(3) or in accordance with AFPA/NDS. Components of roof-ceilings shall be fastened in accordance with Table R602.3(1).~~  ***Delete the remainder of Section R802 and show as "Reserved."***  **(S5989 AS)** |
| ***Section R803.2.3 Installation. Change to read as shown:***  **R803.2.3 Installation.** Wood structural panel used as roof sheathing shall be installed with joints staggered in accordance with Section R802.3.1 ~~or not staggered in accordance with Table R602.3(1), or APA E30~~ for wood roof framing or in accordance with AISI 230 ~~with Table R804.3~~ for steel roof framing.  **R803.2.3.1 Sheathing fastenings.** Wood structural panel sheathing shall be fastened to roof framing with 8d annular ring-shank nails at 6 inches on center at edges and 6 inches on center at intermediate framing. Ring-shank nails shall have the following minimum dimensions:  1. 0.113 inch nominal root shank diameter  2. Ring diameter of 0.010 ~~0.012~~ over shank diameter  3. 16 to 20 rings per inch  4. 0.280 inch full round head diameter  5. 2-3/8 inch nail length  Where roof framing with a specific gravity, 0.42 = G < 0.49 is used, spacing of ring-shank fasteners shall be 4 inches on center in nailing zone 3 in accordance with Figure R803.2.3.1 where Vult is 165 ~~160~~ mph or greater.  **Exceptions:**  1. Where roof framing with a specific gravity, 0.42 = G < 0.49 is used, spacing of ring-shank fasteners shall be permitted at 12 inches on center at intermediate framing in nailing zone 1 for any Vult and in nailing zone 2 for Vult less than or equal to 140 mph in accordance with Figure R803.2.3.1 ~~2304.7.2.1.1~~.  2. Where roof framing with a specific gravity, G = 0.49 is used, spacing of ring-shank fasteners shall be permitted at 12 inches on center at intermediate framing in nailing zone 1 for any Vult and in nailing zone 2 for Vult less than or equal to 150 mph in accordance with Figure R803.2.3.1 ~~2304.7.2.1.1~~.  3. Where roof framing with a specific gravity, G = 0.49 is used, 8d common or 8d hot dipped galvanized box nails at 6 inches on center at edges and 6 inches on center at intermediate framing shall be permitted for Vult less than or equal to 130 mph in accordance with Figure R803.2.3.1 ~~2304.7.2.1.1~~.  4. Where roof diaphragm requirements necessitate a closer fastener spacing.  **FIGURE R803.2.3.1 ROOF SHEATHING NAILING ZONES** |
|  |

**[S5673 AM R1]**

***Section R804 Steel Roof Framing. Revise to read as shown:***

**SECTION R804**

**STEEL ROOF FRAMING**

**RESERVED**

**[S6003 AS]**

***Section R806.5 Unvented attic and unvented enclosed rafter assemblies. Change to read as shown:***

**R806.5 Unvented attic and unvented enclosed rafter assemblies.** Unvented attic assemblies (spaces between the ceiling joists of the top story and the roof rafters) and unvented enclosed rafter assemblies (spaces between ceilings that are applied directly to the underside of roof framing members/rafters and the structural roof sheathing at the top of the roof framing members/rafters shall be permitted if all the following conditions are met:

1. [No change]
2. [No change]
3. [No change]
4. In Climate Zones 5,6,7 and 8 of Table R301.1 of the *Florida Building Code, Energy Conservation*, any air-impermeable insulation shall be a Class II vapor retarder, or shall have a Class III vapor retarder coating or covering in direct contact with the underside of the insulation.
5. [No change]

**(Code correlation)**

***Table R806.5 Insulation for Condensation Control. Change to read as shown:***

**Table R806.5 Insulation for Condensation Control**

**[No change to table]**

1. Contributes to but does not supersede the requirements in Section R403.2.1 of the *Florida Building Code, Energy Conservation ~~N1103.2.1~~*.

**(Code correlation)**

**CHAPTER 9 ROOF ASSEMBLIES**

***R901.1 Scope. Add exception as shown:***

**R901.1 Scope.** The provisions of this chapter shall govern the design, materials, construction and quality of roof assemblies.

**Exception:** Buildings and structures located within the High-Velocity Hurricane Zone shall comply with the provisions of Chapter 44.

**SECTION 903**

**WEATHER PROTECTION**

***Section R903.2.1 Locations. Change to read as shown:***

**R903.2.1 Locations.** Flashings shall be installed at wall and roof intersections, wherever there is a change in roof slope or direction and around roof openings~~. A flashing shall be installed to divert the water away from where the eave of a sloped roof intersects a vertical sidewall.~~ Where flashing is of metal, the metal shall be corrosion resistant with a thickness of not less than ~~0.019 inch (0.5 mm) (No. 26 galvanized sheet)~~ provided in Table R903.2.1.

**Exception:** Flashing is not required at hip and ridge junctions.

**TABLE R903.2.1**

**METAL FLASHING MATERIAL**

|  |  |  |  |
| --- | --- | --- | --- |
| **MATERIAL** | **GAGE MINIMUM THICKNESS (INCHES)** | **GAGE** | **WEIGHT (lbs per sq ft)** |
| Copper | 0.024 | 1 (16 oz) |  |
| Aluminum | 0.024 |  |  |
| Stainless steel | 28 |  |  |
| Galvanized steel | 0.0179 | 26 (zinc coated G90) | 26 (zinc coated G90) |
| Aluminum zinc coated steel | 0.0179 | 26 (AZ50 alum zinc) | 26 (AZ50 alum zinc) |
| Zinc alloy | 0.027 |  |  |
| Lead | 2.5 (40 oz) |  |  |
| Painted terne | 1.25 (20 oz) |  |  |

**(R5473 AS)**

***Section R903.2.3. Add a section to read as shown:***

**R903.2.3** **Membrane** **flashings.** All membrane flashing shall be installed according to the roof assembly manufacturer’s published literature.

**(R5475 AS)**

***Section R903.4 Roof drainage. Change to read as shown:***

**R903.4 Roof drainage.** Unless roofs are sloped to drain over roof edges, roof drains shall be installed at each low point of the roof. Where required for roof drainage, scuppers shall be placed level with the roof surface in a wall or parapet. The scupper shall be located as determined by the roof slope and contributing roof area.

**(R5478 AS)**

***Section R903.4.1 Secondary (emergency overflow) drains or scuppers. Change to read as follows:***

**R903.4.1 ~~Secondary (emergency overflow) drains or scuppers.~~ Overflow drains and scuppers.** When other means of drainage of overflow water is not provided, overflow scuppers shall be placed in walls or parapets not less than 2 inches (51 mm) nor more than 4 inches (102 mm) above the finished roof covering and shall be located as close as practical to required vertical leaders or downspouts or wall and parapet scuppers. An overflow scupper shall be sized in accordance with the *Florida* *Building* *Code,* *Plumbing.*

~~Where roof drains are required, secondary emergency overflow roof drains or scuppers shall be provided where the roof perimeter construction extends above the roof in such a manner that water will be entrapped if the primary drains allow buildup for any reason. Overflow drains having the same size as the roof drains shall be installed with the inlet flow line located 2 inches (51 mm) above the low point of the roof, or overflow scuppers having three times the size of the roof drains and having a minimum opening height of 4 inches (102 mm) shall be installed in the adjacent parapet walls with the inlet flow located 2 inches (51 mm) above the low point of the roof served. The installation and sizing of overflow drains, leaders and conductors shall comply with Sections 1106 and 1108 as applicable of the~~ *~~International Plumbing Code~~*~~.~~

Overflow drains shall discharge to an *approved* location and shall not be connected to roof drain lines.

**(R5476 AS)**

***Section R903.4.2. Add a new section to read as follows:***

**R903.4.2 One** **and** **two** **family dwellings, and** **private garages.** When gutters and leaders are placed on the outside of buildings, the gutters and leaders shall be constructed of metal or approved plastic for outdoor exposure with lapped, soldered or caulked joints and shall be securely fastened to the building with a corrosion resistant fastening device of similar or compatible material to the gutters and downspouts.

**(R5477 AS)**

**SECTION R904**

**MATERIALS**

***Section R904. Change to read as follows:***

**R904.1 Scope.** The requirements set forth in this section shall apply to the application of roof covering materials specified herein. Roof assemblies shall be applied in accordance with this chapter and the manufacturer’s installation instructions. Installation of roof assemblies shall comply with the applicable provisions of Section R905.

**R904.2 Compatibility of materials.** Roof assemblies shall be of materials that are compatible with each other and with the building or structure to which the materials are applied.

**R904.3 Material specifications and physical characteristics.** Roof covering materials shall conform to the applicable standards listed in this chapter. In the absence of applicable standards or where materials are of questionable suitability, testing by an *approved* testing agency shall be required by the *building official* to determine the character, quality and limitations of application of the materials.

***Section R904.4 Product identification. Change to read as shown. Renumber Product identification to R904.5 as shown below.***

**R904.4 Fasteners.**

**R904.4.1** **Nails.** Nails shall be corrosion resistant nails conforming to ASTM F 1667. The corrosion resistance shall meet ASTM A 641, Class I or an equal corrosion resistance by coating, electro galvanization, mechanical galvanization, hot dipped galvanization, stainless steel, nonferrous metal and alloys or other suitable corrosion-resistant material. Metal or plastic cap nails shall have a head diameter of not less than 1 inch (25.4 mm) with a thickness of at least 32-gauge sheet metal, Metal tin-tabs shall be not less than 15/8 inches (41 mm) and not more than 2 inches (51 mm) in diameter and of not less than 32 gage (0.010 inch) sheet metal in compliance with the corrosion resistance requirements.

**R904.4.2** **Screws.** Wood screws shall conform to ANSI/ASME B 18.6.1. Screws shall be corrosion resistant by coating, galvanization, stainless steel, nonferrous metal or other suitable corrosion resistant material. The corrosion resistance shall be demonstrated through one of the following methods:

1. Corrosion resistance equivalent to ASTM A 641, Class 1;

2. Corrosion resistance in accordance with TAS 114, Appendix E;

3. Corrosion resistant coating exhibiting not more than 5 percent red rust after 1000 hours exposure in accordance with ASTM B 117.

**R904.4.3** **Clips.**Clips shall be corrosion resistant clips. The corrosion resistance shall meet 0.90 ounce per square foot (0.458 kg/m2) measured according ASTM A 90/A 90M, TAS 114 Appendix E or an equal corrosion resistance coating, electro galvanization, mechanical galvanization, hot dipped galvanization, stainless steel, nonferrous metals and alloys or other suitable corrosion resistant material. Stainless steel clips shall conform to ASTM A167, Type 304.

**~~R904.4~~ R904.5 Product identification.** Roof covering materials shall be delivered in packages bearing the manufacturer’s identifying marks and *approved* testing agency *labels* when required. Bulk shipments of materials shall be accompanied by the same information issued in the form of a certificate or on a bill of lading by the manufacturer.

**(R5300 AS)**

**SECTION R905**

**REQUIREMENTS FOR ROOF COVERINGS**

***Section R905.2.2 Slope. Change to read as shown:***

**R905.2.2 Slope.** Asphalt shingles shall be used only on roof slopes of two units vertical in 12 units horizontal (2:12) or greater. For roof slopes from two units vertical in 12 units horizontal (2:12) ~~up to~~ and less than four units vertical in 12 units horizontal (4:12), double underlayment application is required in accordance with Section R905.2.7.

**(R5484 AS)**

***Section R905.2.3 Underlayment. Change to read as shown:***

**R905.2.3** **Underlayment.** Unless otherwise noted, required underlayment shall conform ~~to~~ with ASTM D 226 Type I or Type II, ASTM D 4869, Type ~~I~~ II or Type IV, or ASTM D 6757.

Self-adhering polymer modified bitumen sheet shall comply with ASTM D 1970.

**(R5485 AS)**

***R905.2.4.1 Wind resistance of asphalt shingles. Change to read as shown:***

**R905.2.4.1 Wind resistance of asphalt shingles.** Asphalt shingles shall be ~~tested in accordance with ASTM D 7158. Asphalt shingles shall meet the classification requirements of Table R905.2.4.1(1) for the appropriate maximum basic wind speed. Asphalt shingle packaging shall bear a~~ *~~label~~* ~~to indicate compliance with ASTM D 7158 and the required classification in Table R905.2.4.1(1)~~ installed in accordance with Section R905.2.6. and R905.2.6.1.

~~Exception: Asphalt shingles not included in the scope of ASTM D 7158 shall be tested and~~ *~~labeled~~* ~~to indicate compliance with ASTM D 3161 and the required classification in Table R905.2.4.1(2).~~

**TABLE R905.2.4.1(1)**

**~~CLASSIFICATION OF ASPHALT ROOF SHINGLES PER ASTM D 7158~~**

**Reserved.**

|  |  |
| --- | --- |
| **~~MAXIMUM BASIC WIND SPEED FROM FIGURE 301.2(4)A (mph)~~** | **~~CLASSIFICATION REQUIREMENT~~** |
| ~~85~~ | ~~D, G or H~~ |
| ~~90~~ | ~~D, G or H~~ |
| ~~100~~ | ~~G or H~~ |
| ~~110~~ | ~~G or H~~ |
| ~~120~~ | ~~G or H~~ |
| ~~130~~ | ~~H~~ |
| ~~140~~ | ~~H~~ |
| ~~150~~ | ~~H~~ |
| ~~For SI: 1 mile per hour = 0.447 m/s~~. | |
|  | |

**TABLE R905.2.4.1(2)**

**CLASSIFICATION OF ASPHALT SHINGLES PER ASTM D 3161**

Reserved.

|  |  |  |
| --- | --- | --- |
| **~~MAXIMUM BASIC WIND SPEED FROM FIGURE 301.2(4)A (mph)~~** | | **~~CLASSIFICATION REQUIREMENT~~** |
| ~~85~~ | | ~~A, D or F~~ |
| ~~90~~ | | ~~A, D or F~~ |
| ~~100~~ | | ~~A, D or F~~ |
| ~~110~~ | | ~~F~~ |
| ~~120~~ | | ~~F~~ |
| ~~130~~ | | ~~F~~ |
| ~~140~~ | | ~~F~~ |
| ~~150~~ | | ~~F~~ |
| ~~For SI: 1 mile per hour = 0.447 m/s.~~ | |

**(R5486 AS)**

***Section R905.2.6.1. Add a new section to read as follows:***

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **R905.2.6.1 Wind Resistance of Asphalt Shingles.** Asphalt Shingles shall be classified in accordance with ASTM D 3161, TAS 107 or ASTM D 7158 to resist the basic wind speed per Figure R301.2(4). Shingles classified as ASTM D 3161 Class D or classified as ASTM D 7158 Class G are acceptable for use in the 100-mph wind zone. Shingles classified as ASTM D 3161 Class F, TAS107 or ASTM D 7158 Class H are acceptable for use in all wind zones. Asphalt shingle wrappers shall indicate compliance with one of the required classifications as shown in Table R905.2.6.1.  **TABLE R905.2.6.1**  **WIND RESISTANCE OF ASPHALT SHINGLES**   |  |  |  |  | | --- | --- | --- | --- | | **Classification of Asphalt Shingles** | | | | | **Maximum Basic**  **Wind Speed,** **Vult,**  **From Figure**  **R301.2(4)** | **Vasd** **as**  **determined in**  **accordance**  **with Section**  **R301.2.1.3** | **ASTM D 7158** | **ASTM D 3161** | | 110 | 85 | D, G or H | ~~A,~~ D or F | | 116 | 90 | D, G or H | ~~A,~~ D or F | | 129 | 100 | G or H | ~~A,~~ D or F | | 142 | 110 | G or H | F | | 155 | 120 | G or H | F | | 168 | 130 | H | F | | 181 | 140 | H | F | | 194 | 150 | H | F | |
| **(R5487 AM R2)** |
| |  | | --- | |  | |

***Section R905.2.7 Underlayment application. Change to read as shown:***

**R905.2.7 Underlayment application.** Underlayment shall be installed using one of the following methods: ~~For roof slopes from two units vertical in 12 units horizontal (17-percent slope), up to  four units vertical in 12 units horizontal (33-percent slope), underlayment shall be two layers applied in the following manner. Apply a 19-inch (483 mm) strip of underlayment felt parallel to with and starting at the eaves, fastened sufficiently to hold in place. Starting at the eave, apply 36-inch-wide (914 mm) sheets of underlayment, overlapping successive sheets 19 inches (483 mm), and fastened sufficiently to hold in place. Distortions in the underlayment shall not interfere with the ability of the shingles to seal.  End laps shall be offset by 6 feet (1829 mm).~~

~~For roof slopes of four units vertical in 12 units horizontal (33-percent slope) or greater, underlayment  shall be one layer applied in the following manner. Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches (51 mm) fastened sufficiently to hold in place. Distortions in the underlayment shall not interfere with the ability of the shingles to seal. End laps shall be offset by 6 feet (1829 mm).~~

1.      For roof slopes from two units vertical in 12 units horizontal (17-percent slope), and less than four units vertical in 12 units horizontal (33-percent slope). Underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type II or Type IV or ASTM D 6757 and shall be two layers applied in the following manner. Apply a 19-inch (483 mm) strip of underlayment felt parallel to and starting at the eaves, fastened sufficiently to hold in place. Starting at the eave, apply 36-inch-wide (914 mm) sheets of underlayment, overlapping successive sheets 19 inches (483 mm), and fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with one row in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations.

2.      For roof slopes of four units vertical in 12 units horizontal (33-percent slope) or greater. Underlayment shall comply with ASTM D 226, Type II or ASTM D 4869, Type IV or ASTM D 6757 and shall be one layer applied in the following manner. Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches (51 mm), fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with two staggered rows in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations End laps shall be offset by 6 feet (1829 mm).

3.      As an alternative, the entire roof deck shall be covered with an approved self-adhering polymer modified bitumen sheet meeting [ASTM D 1970](javascript:vo();) or an approved self-adhering synthetic underlayment installed in accordance with the manufacturer’s installation instructions.

**(R5555 AS)**

**R905.2.7.1 Ice barrier.** Reserved. ~~In areas where there has been a history of ice forming along the eaves causing a backup of water as designated in Table R301.2(1), an ice barrier that consists of a least two layers of underlayment cemented together or of a self-adhering polymer modified bitumen sheet, shall be used in lieu of normal underlayment and extend from the lowest edges of all roof surfaces to a point at least 24 inches (610 mm) inside the exterior wall line of the building.~~**~~Exception:~~** ~~Detached~~ *~~accessory structures~~* ~~that contain no~~ *~~conditioned floor area~~*~~.~~

**(R5488 AS)**

**R905.2.7.2 Underlayment and high winds.** Reserved.

~~Underlayment applied in areas subject to high winds [above 110 mph (49 m/s) in accordance with Figure R301.2(4)A] shall be applied with corrosion-resistant fasteners in accordance with manufacturer’s installation instructions. Fasteners are to be applied along the overlap not farther apart than 36 inches (914 mm) on center.   
Underlayment installed where the basic wind speed equals or exceeds 120 mph (54 m/s) shall comply with ASTM D 226 Type II, ASTM D 4869 Type IV, or ASTM D 6757. The underlayment shall be attached in a grid pattern of 12 inches (305 mm) between side laps with a 6-inch (152 mm) spacing at the side laps. Underlayment shall be applied in accordance with Section R905.2.7 except all laps shall be a minimum of 4 inches (102 mm). Underlayment shall be attached using metal or plastic cap nails with a head diameter of not less than 1 inch (25.4 mm) with a thickness of at least 32-gauge sheet metal. The cap-nail shank shall be a minimum of 12 gauge (0.105 inches) with a length to penetrate through the roof sheathing or a minimum of~~ ~~3~~~~/~~~~4~~ ~~inch (19 mm) into the roof sheathing.~~**~~Exception:~~** ~~As an alternative, adhered underlayment complying with ASTM D 1970 shall be permitted.~~

**(R5489 AS)**

***Section R905.2.8 Flashing. Change to read as shown:***

**R905.2.8.1 Base and ~~cap~~ counter flashing.** Base and ~~cap~~ counter flashing shall be installed ~~in accordance with manufacturer’s installation instructions. Base flashing shall be of either corrosion-resistant metal of minimum nominal 0.019-inch (0.5 mm) thickness or mineral surface roll roofing weighing a minimum of 77 pounds per 100 square feet (4 kg/m~~~~2~~~~). Cap flashing shall be corrosion-resistant metal of minimum nominal 0.019-inch (0.5 mm) thickness~~ as follows:

1. In accordance with manufacturer’s installation instructions, or

2. A continuous metal minimum 4 inch by 4 inch "L” flashing shall be set in approved flashing cement and set flush to base of wall and over the underlayment. Both horizontal and vertical metal flanges shall be fastened 6 inches (152 mm) on center with approved fasteners. All laps shall be a minimum of 4 inches (102 mm) fully sealed in approved flashing cement. Flashing shall start at the lower portion of roof to ensure water-shedding capabilities of all metal laps. The entire edge of the horizontal flange shall be sealed covering all nail penetrations with approved flashing cement and membrane. Shingles shall overlap the horizontal flange and shall be set in approved flashing cement.   
  
Base flashing shall be of either corrosion-resistant metal provided in Section R905.2.8.1 or mineral surface roll roofing weighing a minimum of 77 pounds per 100 square feet (3.76 kg/m2). Counter flashing shall be corrosion-resistant metal with a minimum thickness provided in Table R903.2.1.

**(R5491 AS)**

**R905.2.8.2 Valleys.** Valley linings shall be installed in accordance with the manufacturer’s installation instructions before applying shingles. Valley linings of the following types shall be permitted:

1. For open valleys (valley lining exposed) lined with metal, the valley lining shall be at least ~~24~~ 16 inches (~~610~~ 406 mm) wide and of any of the corrosion-resistant metals in Table ~~R905.2.8.2~~ R903.2.1 .

2. For open valleys, valley lining of two plies of mineral surfaced roll roofing, complying with ASTM D 3909 or ASTM D 6380 Class M, shall be permitted. The bottom layer shall be 18 inches (457 mm) and the top layer a minimum of 36 inches (914 mm) wide.

3. For closed valleys (valley covered with shingles), valley lining of one ply of smooth roll roofing complying with ASTM D 6380 Class S and at least 36 inches wide (914 mm) or valley lining as described in Item 1 or 2 above shall be permitted. Self-adhering polymer modified bitumen underlayment complying with ASTM D 1970 shall be permitted in lieu of the lining material

**TABLE R905.2.8.2**

**VALLEY LINING MATERIAL**

Reserved.

|  |  |  |  |  |
| --- | --- | --- | --- | --- |
| **~~MATERIAL~~** | **~~MINIMUM THICKNESS  (inches)~~** | | **~~GAGE~~** | **~~WEIGHT  (pounds)~~** |
| ~~Cold-rolled copper~~ | ~~0.0216 nominal~~ | | ~~—~~ | ~~ASTM B 370, 16 oz. per square foot~~ |
| ~~Lead-coated copper~~ | ~~0.0216 nominal~~ | | ~~—~~ | ~~ASTM B 101, 16 oz. per square foot~~ |
| ~~High-yield copper~~ | ~~0.0162 nominal~~ | | ~~—~~ | ~~ASTM B 370, 12 oz. per square foot~~ |
| ~~Lead-coated high-yield copper~~ | ~~0.0162 nominal~~ | | ~~—~~ | ~~ASTM B 101, 12 oz. per square foot~~ |
| ~~Aluminum~~ | ~~0.024~~ | | ~~—~~ | ~~—~~ |
| ~~Stainless steel~~ | ~~—~~ | | ~~28~~ | ~~—~~ |
| ~~Galvanized steel~~ | ~~0.0179~~ | | ~~26 (zinc coated G90)~~ | ~~—~~ |
| ~~Zinc alloy~~ | ~~0.027~~ | | ~~—~~ | ~~—~~ |
| ~~Lead~~ | ~~—~~ | | ~~—~~ | ~~2~~~~1~~~~/~~~~2~~ |
| ~~Painted terne~~ | ~~—~~ | | ~~—~~ | ~~20~~ |
| ~~For SI: 1 inch = 25.4 mm, 1 pound = 0.454 kg.~~ | |

**(R5492 AS)**

**R905.2.8.3 Sidewall flashing.** ~~Base f~~ Flashing against a vertical sidewall shall be by the step-flashing method or continuous "L” flashing method. ~~continuous or step flashing and shall be a minimum of 4 inches (102 mm) in height and 4 inches (102 mm) in width and shall direct water away from the vertical sidewall onto the roof and/or into the gutter. Where siding is provided on the vertical sidewall, the vertical leg of the flashing shall be continuous under the siding. Where anchored masonry veneer is provided on the vertical sidewall, the base flashing shall be provided in accordance with this section and counterflashing shall be provided in accordance with Section R703.7.2.2. Where exterior plaster or adhered masonry veneer is provided on the vertical sidewall, the base flashing shall be provided in accordance with this section and Section R703.6.3~~.

**(R5493 AS)**

**R905.2.8.5 Drip edge.** Provide drip edge at eaves and gables of shingle roofs. Overlap to be a minimum of 3 inches (76 mm). Eave drip edges shall extend 1/2 inch (13 mm) below sheathing and extend back on the roof a minimum of 2 inches (51 mm). Drip edge at eaves shall be permitted to be installed either over or under the underlayment. If installed over the underlayment, there shall be a minimum 4 inch (51 mm) width of roof cement installed over the drip edge flange. Drip edge shall be mechanically fastened a maximum of 12 inches (305 mm) on center. Where the *Vasd* as determined in accordance with Section R301.2.1.3 is 110 mph (177 km/h) or greater or the mean roof height exceeds 33 feet (10 058 mm), drip edges shall be mechanically fastened a maximum of 4 inches (102 mm) on center.

~~A drip edge shall be provided at eaves and gables of shingle roofs. Adjacent pieces of drip edge shall be overlapped a minimum of 2 i0nches (51 mm). Drip edges shall extend a minimum of 0.25 inch (6.4 mm) below the roof sheathing and extend up the roof deck a minimum of 2 inches (51 mm). Drip edges shall be mechanically fastened to the roof deck at a maximum of 12 inches (305 mm) o.c. with fasteners as specified in Section R905.2.5. Underlayment shall be installed over the drip edge along eaves and under the underlayment on gables. Unless specified differently by the shingle manufacturer, shingles are permitted to be flush with the drip edge.~~

**(R5494 AS)**

***Section R905.3 Clay and concrete tile. Change to read as follows:*  
  
R905.3** **Clay** **and** **concrete** **tile.** The installation of clay and concrete shall ~~comply with the provisions of this section~~ be in accordance with the manufacturer’s installation instructions, or recommendations of FRSA/TRI Florida High Wind Concrete and Clay Roof Tile Installation Manual, Fifth Edition where the Vasd is determined in accordance with Section R301.2.1.3 or the recommendations of RAS 118, 119 or 120.

**R905.3.1** **Deck requirements.** Concrete and clay tile shall be installed only over solid sheathing ~~or~~, except where the roof covering is specifically designed and tested in accordance with Chapter 16, *Florida* *Building* *Code, Building* to be applied over spaced structural sheathing boards.

**R905.3.2** **Deck** **slope.** Clay and concrete roof tile shall be installed on roof slopes ~~of two and one-half units vertical in 12 units horizontal (2~~~~1~~~~/~~~~2~~~~:12) or greater. For roof slopes from two and one-half units vertical in 12 units horizontal (2~~~~1~~~~/~~~~2~~~~:12) to four units vertical in 12 units horizontal (4:12), double underlayment application is required in accordance with Section R905.3.3.~~ in accordance with the recommendations of FRSA/TRI Florida High Wind Concrete and Clay Roof Tile Installation Manual, Fifth Edition where the Vasd is determined in accordance with Section R301.2.1.3 or the recommendations of RAS 118, 119 or 120.

**R905.3.3** **Underlayment.** ~~Unless otherwise noted, r~~ Required underlayment shall conform ~~to~~ with ASTM D 226, Type II; ASTM D 2626, Type ~~I~~ II; ASTM D 1970 or ASTM D 6380, Class M mineral surfaced roll roofing and shall be installed in accordance with FRSA/TRI Florida High Wind Concrete and Clay Roof Tile Installation Manual, Fifth Edition where the Vasd is determined in accordance with Section R301.2.1.3 or the recommendations of RAS 118, 119 or 120.

**R905.3.3.1 ~~Low~~ ~~s~~ Slope and** **underlayment requirements ~~roofs~~.** Refer to FRSA/TRI Florida High Wind Concrete and Clay Roof Tile Installation Manual, Fifth Edition where the Vasd is determined in accordance with Section R301.2.1.3 or RAS 118, 119 or 120 for underlayment and slope requirements for specific roof tile systems.  
~~For roof slopes from two and one-half units vertical in 12 units horizontal (2~~~~1~~~~/~~~~2~~~~:12), up to four units vertical in 12 units horizontal (4:12), underlayment shall be a minimum of two layers underlayment applied as follows:~~

~~1. Starting at the eave, a 19-inch (483 mm) strip of underlayment shall be applied parallel with the eave and fastened sufficiently in place.~~

~~2. Starting at the eave, 36-inch-wide (914 mm) strips of underlayment felt shall be applied, overlapping successive sheets 19 inches (483 mm), and fastened sufficiently in place.~~

**R905.3.3.2 High slope roofs.** Reserved.  
~~For roof slopes of four units vertical in 12 units horizontal (4:12) or greater, underlayment shall be a minimum of one layer of underlayment felt applied shingle fashion, parallel to and starting from the eaves and lapped 2 inches (51 mm), fastened sufficiently in place.~~

**R905.3.3.3 Underlayment and high winds.** Reserved.  
~~Underlayment applied in areas subject to high wind [above 110 miles per hour (49 m/s) in accordance with Figure R301.2(4)A] shall be applied with corrosion-resistant fasteners in accordance with manufacturer’s installation instructions. Fasteners are to be applied along the overlap not farther apart than 36 inches (914 mm) on center.   
Underlayment installed where the basic wind speed equals or exceeds 120 mph (54 m/s) shall be attached in a grid pattern of 12 inches (305 mm) between side laps with a 6-inch (152 mm) spacing at the side laps. Underlayment shall be applied in accordance with Section R905.2.7 except all laps shall be a minimum of 4 inches (102 mm). Underlayment shall be attached using metal or plastic cap nails with a head diameter of not less than 1 inch (25.4 mm) with a thickness of at least 32-gauge sheet metal. The cap-nail shank shall be a minimum of 12 gauge (0.105 inches) with a length to penetrate through the roof sheathing or a minimum of~~ ~~3~~~~/~~~~4~~~~-inch (19 mm) into the roof sheathing.~~**Exception:** ~~As an alternative, adhered underlayment complying with ASTM D 1970 shall be permitted.~~

**R905.3.4 Clay tile.** Clay roof tile shall comply with ASTM C 1167.

**R905.3.5 Concrete tile.** Concrete roof tile shall comply with ASTM C 1492.

**R905.3.6 Fasteners.** Nails shall be corrosion-resistant and not less than 11 gage, 5/16-inch (7.95 mm) head, and of sufficient length to penetrate the deck a minimum of 3/4 inch (19.1 mm) or through the thickness of the deck, whichever is less, or in accordance with the FRSA/TRI Florida High Wind Concrete and Clay Roof Tile Installation Manual, Fifth Edition  where the Vasd is determined in accordance with Section R301.2.1.3 or in accordance with the recommendations of RAS 118, 119 or 120. Attaching wire for clay or concrete tile shall not be smaller than 0.083 inch (2.1 mm). ~~Perimeter fastening areas include three tile courses but not less than 36 inches (914 mm) from either side of hips or ridges and edges of eaves and gable rakes.~~

**R905.3.7 Application.** Tile shall be applied in accordance with this chapter and the manufacturer’s installation instructions, or recommendations of the FRSA/TRI Florida High Wind Concrete and Clay Roof Tile Installation Manual, Fifth Edition based on the following:

**Attachment.** Clay and concrete roof tiles shall be fastened in accordance with FRSA/TRI Florida High Wind Concrete and Clay Roof Tile Installation Manual, Fifth Edition where the Vasd is determined in accordance with Section R301.2.1.3.

~~1. Climatic conditions.~~

~~2. Roof slope.~~

~~3. Underlayment system.~~

~~4. Type of tile being installed.~~

~~Clay and concrete roof tiles shall be fastened in accordance with this section and the manufacturer’s installation instructions. Perimeter tiles shall be fastened with a minimum of one fastener per tile. Tiles with installed weight less than 9 pounds per square foot (0.4 kg/m~~~~2~~~~) require a minimum of one fastener per tile regardless of roof slope. Clay and concrete roof tile attachment shall be in accordance with the manufacturer’s installation instructions where applied in areas where the wind speed exceeds 100 miles per hour (45 m/s) and on buildings where the roof is located more than 40 feet (12 192 mm) above~~ *~~grade~~*~~. In areas subject to snow, a minimum of two fasteners per tile is required. In all other areas, clay and concrete roof tiles shall be attached in accordance with Table R905.3.7.~~

**R905.3.7.1** **Hip** **and** **ridge** **tiles.** Hip and ridge tiles shall be installed in accordance with FRSA/TRI Florida High Wind Concrete and Clay Roof Tile Installation Manual, Fifth Edition where the Vasd is determined in accordance with Section R301.2.1.3.

**TABLE R905.3.7**

**CLAY AND CONCRETE TILE ATTACHMENT**

Reserved.

|  |  |  |
| --- | --- | --- |
| **~~HEATHING~~** | **~~ROOF SLOPE~~** | **~~NUMBER OF FASTENERS~~** |
| ~~Solid without battens~~ | ~~All~~ | ~~One per tile~~ |
| ~~Spaced or solid with battens and slope < 5:12~~ | ~~Fasteners not  required~~ | ~~—~~ |
| ~~Spaced sheathing without battens~~ | ~~5:12 = slope < 12:12~~ | ~~One per tile/every other row~~ |
| ~~12:12 = slope < 24:12~~ | ~~One per tile~~ |

**R905.3.8 Flashing.** At the juncture of roof vertical surfaces, flashing and counterflashing shall be provided in accordance with this chapter and the manufacturer’s installation instructions or recommendations of the FRSA/TRI Florida High Wind Concrete and Clay Roof Tile Installation Manual, Fifth Edition where the Vasd is determined in accordance with Section R301.2.1.3 or the recommendations of RAS 118, 119 or 120. ~~and, where of metal, shall not be less than 0.019 inch (0.5 mm) (No. 26 galvanized sheet gage) corrosion-resistant metal. The valley flashing shall extend at least 11 inches (279 mm) from the centerline each way and have a splash diverter rib not less than 1 inch (25 mm) high at the flow line formed as part of the flashing. Sections of flashing shall have an end lap of not less than 4 inches (102 mm). For roof slopes of three units vertical in 12 units horizontal (25-percent slope) and greater, valley flashing shall have a 36-inch-wide (914 mm) underlayment of one layer of Type I underlayment running the full length of the valley, in addition to other required underlayment. In areas where the average daily temperature in January is 25°F (-4°C) or less, metal valley flashing underlayment shall be solid-cemented to the roofing underlayment for slopes less than seven units vertical in 12 units horizontal (58-percent slope) or be of self-adhering polymer modified bitumen sheet.~~

**(R5304 AM R1)**

***Section R905.4 Metal roof shingles. Change to read as follows:***

**R905.4 Metal roof shingles.** The installation of metal roof shingles shall comply with the provisions of this section.

**R905.4.1 Deck requirements.** Metal roof shingles shall be applied to a solid or closely fitted deck, except where the roof covering is specifically designed to be applied to spaced sheathing.

**R905.4.2 Deck slope.** Metal roof shingles shall not be installed on roof slopes below three units vertical in 12 units horizontal (25-percent slope).

**R905.4.3 Underlayment.** Underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type ~~I~~ II or Type ~~II~~ IV or ASTM D 1970 or ASTM D 6757. ~~Underlayment shall be installed in accordance with the manufacturer’s installation instructions.~~

**R905.4.3.1 Ice barrier.** Reserved.

~~In areas where there has been a history of ice forming along the eaves causing a backup of water as designated in Table R301.2(1), an ice barrier that consists of at least two layers of underlayment cemented together or a self-adhering polymer modified bitumen sheet shall be used in place of normal underlayment and extend from the lowest edges of all roof surfaces to a point at least 24 inches (610 mm) inside the exterior wall line of the building.   
Exception: Detached~~ *~~accessory structures~~* ~~that contain no~~ *~~conditioned floor area~~*~~.~~

**R905.4.3.2 Underlayment and high winds.** Reserved.

~~Underlayment applied in areas subject to high winds [above 110 mph (49 m/s) in accordance with Figure R301.2(4)A] shall be applied with corrosion-resistant fasteners in accordance with manufacturer’s installation instructions. Fasteners are to be applied along the overlap not farther apart than 36 inches (914 mm) on center.   
Underlayment installed where the basic wind speed equals or exceeds 120 mph (54 m/s) shall comply with ASTM D 226 Type II. The underlayment shall be attached in a grid pattern of 12 inches (305 mm) between side laps with a 6-inch (152 mm) spacing at the side laps. Underlayment shall be applied in accordance with Section R905.2.7except all laps shall be a minimum of 4 inches (102 mm). Underlayment shall be attached using metal or plastic cap nails with a head diameter of not less than 1 inch (25.4 mm) with a thickness of at least 32-gauge sheet metal. The cap-nail shank shall be a minimum of 12 gauge (0.105 inches) with a length to penetrate through the roof sheathing or a minimum of~~ ~~3~~~~/~~~~4~~ ~~inch (19 mm) into the roof sheathing.   
Exception: As an alternative, adhered underlayment complying with ASTM D 1970 shall be permitted.~~

**R905.4.3.3 Underlayment Application**. Underlayment shall be installed using one of the following methods:

 1.      Two layer underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type II or Type IV or ASTM D 6757: Apply a 19-inch (483 mm) strip of underlayment felt parallel to and starting at the eaves, fastened sufficiently to hold in place. Starting at the eave, apply 36-inch-wide (914 mm) sheets of underlayment, overlapping successive sheets 19 inches (483 mm), and fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with one row in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations.

2.      One layer underlayment shall comply with ASTM D 226, Type II or ASTM D 4869, Type IV or ASTM D 6757: Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches (51 mm), fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with two staggered rows in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations End laps shall be offset by 6 feet (1829 mm).

 3.      As an alternative, the entire roof deck shall be covered with an approved self-adhering polymer modified bitumen sheet meeting [ASTM D 1970](javascript:vo();) or an approved self-adhering synthetic underlayment installed in accordance with the manufacturer’s installation instructions.

**(R5556 AS)(R5307 AM R1)**

**R905.4.4 Material standards.** Metal roof shingle roof coverings shall comply with Table ~~R905.10.3(1)~~ R905.4.4. The materials used for metal roof shingle roof coverings shall be naturally corrosion resistant or ~~be made~~ provided with corrosion ~~resistant~~ resistance in accordance with the standards and minimum thicknesses specified in the standards listed in Table ~~R905.10.3(2)~~ R905.4.4.

**TABLE 905.4.4**

**METAL ROOF COVERINGS**

|  |  |  |
| --- | --- | --- |
| **ROOF COVERING TYPE** | **STANDARD** | **STANDARD APPLICATION RATE/THICKNESS** |
| Aluminum | ASTM B 209 | 0.024-inch minimum thickness for roll-formed panels and 0.019-inch minimum thickness for press-formed shingles |
| Aluminum-zinc coated steel | ASTM A 792 | 0.013-inch minimum thickness, AZ 50 (coated minimum application rate) |
| Cold-rolled copper | ASTM B 370 | Minimum 16 oz./sq. ft. and 12 oz./sq. ft. high yield copper for metal-sheet roof covering systems: 12 oz./sq. ft. for preformed metal shingle systems |
| Copper | ASTM B 370 | 16 oz./sq. ft. for metal-sheet roof-covering systems; 12 oz./sq. ft. for preformed metal shingle systems. |
| Galvanized steel | ASTM A 653 | 0.013-inch minimum thickness, G-90 zinc-coateda |
| Hard lead | - | 2 lbs./sq. ft. |
| Lead-coated copper | ASTM B 101 | - |
| Prepainted steel | ASTM A 755 | 0.0142 inch minimum thickness |
| Soft lead | - | 3 lbs./sq. ft. |
| Stainless steel | ASTM A 240 | 300 Series alloys |
| Steel | ASTM A 924/ ASTM A 924M | - |
| Terne and terne-coated stainless | - | Terne coating of 40 lbs. per double base box, field painted where applicable in accordance with manufacturer’s installation instructions |
| Zinc | - | 0.027 inch minimum thickness; 99.995% electrolytic high grade zinc with alloy additives of copper (0.08% - 0.20%), titanium (0.07% - 0.12%) and aluminum (0.015%) |

 For SI: 1 ounce per square foot = 0.0026 kg/m2, 1 pound per square foot = 4.882 kg/m2, 1 inch = 25.4 mm, 1 pound = 0.454 kg.

 a. For Group U buildings, the minimum coating thickness for ASTM A 653 galvanized steel roofing shall be G.

**(R5508 AS)**

**R905.4.5 Application.** Metal roof shingles shall ~~be secured to the roof in accordance with this chapter and the~~ *~~approved~~* ~~manufacturer’s installation instructions~~ installed in accordance with the approved manufacturer’s installation instructions. The product approval shall state the allowable uplift resistance for the attachment system. The installation of metal roof shingles shall be limited to roofs where the allowable uplift resistance is equal to or greater than the design uplift pressure for the roof listed in Table R301.2(2).

**R905.4.6 Flashing**. Roof valley flashing shall be of corrosion-resistant metal of the same material as the roof covering or shall comply with the standards in Table ~~R905.10.3(1)~~ R905.4.4.. ~~The~~ V~~v~~alley flashing shall extend at least 8 inches (203 mm) from the centerline each way and shall have a splash diverter rib not less than 3/4 inch (19 mm) high at the flow line formed as part of the flashing. Sections of flashing shall have an end lap of not less than 4 inches (102 mm). The metal valley flashing shall have a 36-inch-wide (914 mm) underlayment directly under it consisting of one layer of underlayment running the full length of the valley, in addition to underlayment required for metal roof shingles. ~~In areas where the average daily temperature in January is 25°F (-4°C) or less, the metal valley flashing underlayment shall be solid cemented to the roofing underlayment for roof slopes under seven units vertical in 12 units horizontal (58-percent slope) or self-adhering polymer modified bitumen sheet.~~

**(R5307 AM R1)**

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| ***Section R905.5 Mineral-surfaced roll roofing. Change to read as shown:***   |  | | --- | | **R905.5 Mineral-surfaced roll roofing.** The installation of mineral-surfaced roll roofing shall comply with this section.  **R905.5.1 Deck requirements.** Mineral-surfaced roll roofing shall be fastened to solidly sheathed roofs.  **R905.5.2 Deck slope.** Mineral-surfaced roll roofing shall not be applied on roof slopes below one unit vertical in 12 units horizontal (8-percent slope).  **R905.5.3 Underlayment.** Underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type II or Type IV or ASTM D 1970 or ASTM D 6757.  **R905.5.3.1 Ice barrier.**Reserved.~~In areas where there has been a history of ice forming along the eaves causing a backup of water as designated in Table R301.2(1), an ice barrier that consists of at least two layers of underlayment cemented together or a self-adhering polymer modified bitumen sheet shall be used in place of normal underlayment and extend from the lowest edges of all roof surfaces to a point at least 24 inches (610 mm) inside the exterior wall line of the building.  Exception: Detached~~ *~~accessory structures~~* ~~that contain no~~ *~~conditioned floor area~~*~~.~~  **R905.5.3.2 Underlayment and high winds.** Reserved.  ~~Underlayment applied in areas subject to high winds [above 110 mph (49 m/s) in accordance with Figure R301.2(4)A] shall be applied with corrosion-resistant fasteners in accordance with manufacturer’s installation instructions. Fasteners are to be applied along the overlap not farther apart than 36 inches (914 mm) on center.  Underlayment installed where the basic wind speed equals or exceeds 120 mph (54 m/s) shall comply with ASTM D 226 Type II. The underlayment shall be attached in a grid pattern of 12 inches (305 mm) between side laps with a 6-inch (152 mm) spacing at the side laps. Underlayment shall be applied in accordance with Section R905.2.7except all laps shall be a minimum of 4 inches (102 mm). Underlayment shall be attached using metal or plastic cap nails with a head diameter of not less than 1 inch (25.4 mm) with a thickness of at least 32-gauge sheet metal. The cap-nail shank shall be a minimum of 12 gauge (0.105 inches) with a length to penetrate through the roof sheathing or a minimum of~~ ~~3~~~~/~~~~4~~ ~~inch (19 mm) into the roof sheathing.~~  **~~Exception:~~** ~~As an alternative, adhered underlayment complying with ASTM D 1970 shall be permitted~~**~~.~~**  **R905.5.3.3 Underlayment Application.** Underlayment shall be installed using one of the following methods:  1.      Two layer underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type II or Type IV or ASTM D 6757: Apply a 19-inch (483 mm) strip of underlayment felt parallel to and starting at the eaves, fastened sufficiently to hold in place. Starting at the eave, apply 36-inch-wide (914 mm) sheets of underlayment, overlapping successive sheets 19 inches (483 mm), and fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with one row in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations.  2.      One layer underlayment shall comply with ASTM D 226, Type II or ASTM D 4869, Type IV or ASTM D 6757: Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches (51 mm), fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with two staggered rows in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations End laps shall be offset by 6 feet (1829 mm).  3.      As an alternative, the entire roof deck shall be covered with an approved self-adhering polymer modified bitumen sheet meeting [ASTM D 1970](javascript:vo();) or an approved self-adhering synthetic underlayment installed in accordance with the manufacturer’s installation instructions.  **R905.5.4 Material standards.** Mineral-surfaced roll roofing shall conform to ASTM D 3909 or ASTM D 6380, Class M or Class WS.  **R905.5.5 Application.** Mineral-surfaced roll roofing shall be installed in accordance with this chapter and the manufacturer’s installation instructions.  **(R5308 AM R1)** | |  |   ***Section 905.6 Slate and slate-type shingles. Change to read as shown:***  **R905.6 Slate and slate-type shingles.** The installation of slate and slate-type shingles shall comply with the provisions of this section.  **R905.6.1 Deck requirements.** Slate shingles shall be fastened to solidly sheathed roofs.  **R905.6.2 Deck slope.** Slate shingles shall be used only on slopes of four units vertical in 12 units horizontal (33-percent slope) or greater.  **R905.6.3 Underlayment.** Underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869,Type ~~I~~ II or Type ~~II~~ IV or ASTM D 1970 or ASTM D 6757. Underlayment shall be installed in accordance with the manufacturer’s installation instructions.  **R905.6.3.1 Ice barrier.** Reserved. ~~In areas where there has been a history of ice forming along the eaves causing a backup of water as designated in Table R301.2(1), an ice barrier that consists of at least two layers of underlayment cemented together or a self-adhering polymer modified bitumen sheet shall be used in place of normal underlayment and extend from the lowest edges of all roof surfaces to a point at least 24 inches (610 mm) inside the exterior wall line of the building.  Exception: Detached~~ *~~accessory structures~~* ~~that contain no~~ *~~conditioned floor area~~*~~.~~  **R905.6.3.2 Underlayment and high winds.** Reserved.  ~~Underlayment applied in areas subject to high winds [above 110 mph (49 m/s) in accordance with Figure R301.2(4)A] shall be applied with corrosion-resistant fasteners in accordance with manufacturer’s installation instructions. Fasteners are to be applied along the overlap not farther apart than 36 inches (914 mm) on center.  Underlayment installed where the basic wind speed equals or exceeds 120 mph (54 m/s) shall comply with ASTM D 226 Type II. The underlayment shall be attached in a grid pattern of 12 inches (305 mm) between side laps with a 6-inch (152 mm) spacing at the side laps. Underlayment shall be applied in accordance with Section R905.2.7except all laps shall be a minimum of 4 inches (102 mm). Underlayment shall be attached using metal or plastic cap nails with a head diameter of not less than 1 inch (25.4 mm) with a thickness of at least 32-gauge sheet metal. The cap-nail shank shall be a minimum of 12 gauge (0.105 inches) with a length to penetrate through the roof sheathing or a minimum of~~ ~~3~~~~/~~~~4~~ ~~inch (19 mm) into the roof sheathing.  Exception: As an alternative, adhered underlayment complying with ASTM D 1970 shall be permitted.~~  **R905.6.3.3 Underlayment Application**. Underlayment shall be installed using one of the following methods:  1.      Two layer underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type II or Type IV or ASTM D 6757: Apply a 19-inch (483 mm) strip of underlayment felt parallel to and starting at the eaves, fastened sufficiently to hold in place. Starting at the eave, apply 36-inch-wide (914 mm) sheets of underlayment, overlapping successive sheets 19 inches (483 mm), and fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with one row in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations.  2.      One layer underlayment shall comply with ASTM D 226, Type II or ASTM D 4869, Type IV or ASTM D 6757: Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches (51 mm), fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with two staggered rows in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations End laps shall be offset by 6 feet (1829 mm).  3.      As an alternative, the entire roof deck shall be covered with an approved self-adhering polymer modified bitumen sheet meeting [ASTM D 1970](javascript:vo();) or an approved self-adhering synthetic underlayment installed in accordance with the manufacturer’s installation instructions.  **R905.6.4 Material standards.** Slate shingles shall comply with ASTM C 406.  **R905.6.5 Application**. Minimum headlap for slate shingles shall be in accordance with Table R905.6.5. Slate shingles shall be secured to the roof with two fasteners per slate. Slate shingles shall be installed in accordance with this chapter and the manufacturer’s installation instructions.  **TABLE R905.6.5**  **SLATE SHINGLE HEADLAP**   |  |  | | --- | --- | | **SLOPE** | **HEADLAP (inches)** | | 4:12 ≤ slope <8:12 | 4 | | 8:12 ≤slope < 0:12 | 3 | | Slope ≤ 20:12 | 2 | | FrSI1 ch 25.4 mm. |   **R905.6.6 Flashing**. Flashing and counter flashing shall be made with sheet metal. Valley flashing shall be a minimum of ~~15~~ 16 inches (~~381~~ 406 mm) wide. Valley and flashing metal shall be a minimum ~~uncoated~~ thickness ~~of 0.0179-inch (0.5 mm) zinc coated G90~~ as provided in Table R903.2.1 for nonferrous metal or stainless steel. Chimneys, stucco or brick walls shall have a minimum of two plies of felt for a cap flashing consisting of a 4-inch-wide (102 mm) strip of felt set in plastic cement and extending 1 inch (25 mm) above the first felt and a top coating of plastic cement. The felt shall extend over the base flashing 2 inches (51 mm).  **R905.6.7** Slate and slate-type shingles shall be installed in accordance with this chapter and the manufacturer’s installation instructions. The product approval shall state the allowable uplift resistance for the attachment system. The installation of slate and slate-type shingles shall be limited to roofs where the allowable uplift resistance is equal to or greater than the design uplift pressure for the roof listed in Table R301.2(2)**.**  **(R5312 AM R1)**  ***Section R905.7 Wood shingles. Change to read as shown:***  **R905.7 Wood shingles.** The installation of wood shingles shall comply with the provisions of this section.  **R905.7.1 Deck requirements.** Wood shingles shall be installed on solid or spaced sheathing. Where spaced sheathing is used, sheathing boards shall not be less than 1-inch by 4-inch (25.4 mm by 102 mm) nominal dimensions and shall be spaced on centers equal to the weather exposure to coincide with the placement of fasteners.  **R905.7.1.1 Solid sheathing required.** Reserved. ~~In areas where the average daily temperature in January is 25°F (-4°C) or less, solid sheathing is required on that portion of the roof requiring the application of an ice barrier.~~  **R905.7.2 Deck slope.** Wood shingles shall be installed on slopes of three units vertical in 12 units horizontal (25-percent slope) or greater.  **R905.7.3 Underlayment.** Underlayment shall comply with ASTM D 226, Type I or ASTM D 4869, Type I or II.  **R905.7.3.1 Ice barrier.** Reserved. ~~In areas where there has been a history of ice forming along the eaves causing a backup of water as designated in Table R301.2(1), an ice barrier that consists of at least two layers of underlayment cemented together or a self-adhering polymer modified bitumen sheet shall be used in lieu of normal underlayment and extend from the lowest edges of all roof surfaces to a point at least 24 inches (610 mm) inside the exterior wall line of the building.   Exception: Detached~~ *~~accessory structures~~* ~~that contain no~~ *~~conditioned floor area~~*~~.~~  **R905.7.3.2 Underlayment and high winds.** Reserved. ~~Underlayment applied in areas subject to high winds [above 110 mph (49 m/s) in accordance with Figure R301.2(4)A] shall be applied with corrosion-resistant fasteners in accordance with manufacturer’s installation instructions. Fasteners are to be applied along the overlap not farther apart than 36 inches (914mm) on center.   Underlayment installed where the basic wind speed equals or exceeds 120 mph (54 m/s) shall comply with ASTM D 226 Type II or ASTM D 4869 Type IV. The underlayment shall be attached in a grid pattern of 12 inches (305 mm) between side laps with a 6-inch (152 mm) spacing at the side laps. Underlayment shall be applied in accordance with Section R905.2.7 except all Head laps shall be a minimum of 4 inches (102 mm). Underlayment shall be attached using metal or plastic cap nails with a head diameter of not less than 1 inch (25.4 mm) with a thickness of at least 32-gauge sheet metal. The cap-nail shank shall be a minimum of 12 gauge (0.105 inches) with a length to penetrate through the roof sheathing or a minimum of~~ ~~3~~~~/~~~~4~~ ~~inch (19 mm) into the roof sheathing.   Exception: As an alternative, adhered underlayment complying with ASTM D 1970 shall be permitted.~~  **R905.7.3.3 Underlayment Application.** Underlayment shall be installed using one of the following methods:  1.      Two layer underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type II or Type IV: Apply a 19-inch (483 mm) strip of underlayment felt parallel to and starting at the eaves, fastened sufficiently to hold in place. Starting at the eave, apply 36-inch-wide (914 mm) sheets of underlayment, overlapping successive sheets 19 inches (483 mm), and fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with one row in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm).  2.      One layer underlayment shall comply with ASTM D 226, Type II or ASTM D 4869, Type IV: Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches (51 mm), fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with two staggered rows in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). End laps shall be offset by 6 feet (1829 mm).  **(S6168GlitchAS)**  ***Section R905.7 Wood shingles. Change to read as shown:***  **R905.7.4 Material standards.** Wood shingles shall be of naturally durable wood and comply with the requirements of Table R905.7.4.  **TABLE R905.7.4**  **WOOD SHINGLE MATERIAL REQUIREMENTS**   |  |  |  | | --- | --- | --- | | **MATERIAL** | **MINIMUM GRADES** | **APPLICABLE GRADING RULES** | | Wood shingles of naturally durable wood | 1, 2 or 3 | Cedar Shake and Shingle Bureau |   **R905.7.5 ~~Application~~ Attachment.** Wood shingles shall be installed according to this chapter and the manufacturer’s installation instructions. ~~Wood shingles shall be laid with a side lap not less than 1~~~~1~~~~/~~~~2~~ ~~inches (38 mm) between joints in courses, and no two joints in any three adjacent courses shall be in direct alignment. Spacing between shingles shall not be less than~~ ~~1~~~~/~~~~4~~ ~~inch to~~ ~~3~~~~/~~~~8~~ ~~inch (6 mm to 10 mm). Weather exposure for wood shingles shall not exceed those set in Table R905.7.5. Fasteners for wood shingles shall be corrosion resistant with a minimum penetration of~~ ~~1~~~~/~~~~2~~ ~~inch (13 mm) into the sheathing. For sheathing less than~~ ~~1~~~~/~~~~2~~ ~~inch (13 mm) in thickness, the fasteners shall extend through the sheathing. Wood shingles shall be attached to the roof with two fasteners per shingle, positioned no more than~~ ~~3~~~~/~~~~4~~ ~~inch (19 mm) from each edge and no more than 1 inch (25 mm) above the exposure line.~~ Attachment in accordance with Table R905.7.5 shall be used for roofs with a mean roof height of 40 feet or less and in regions with a *Vasd* as determined in accordance with Section R301.2.1.3 to be 100 mph or less    **TABLE R905.7.5**  **WOOD SHINGLE ~~WEATHER EXPOSURE AND ROOF SLOPE~~ AND SHAKE INSTALLATION**   |  |  |  |  |  | | --- | --- | --- | --- | --- | | **~~ROOFING MATERIAL~~** | **~~LENGTH (inches)~~** | **~~GRADE~~** | **~~EXPOSURE (inches)~~** | | | **~~3:12 pitch to~~** | **~~4:12 pitch or steeper~~** | | ~~Shingles of naturally durable  wood~~ | ~~16~~ | ~~No. 1~~ | ~~3~~~~3~~~~/~~~~4~~ | ~~5~~ | | ~~No. 2~~ | ~~3~~~~1~~~~/~~~~2~~ | ~~4~~ | | ~~No. 3~~ | ~~3~~ | ~~3~~~~1~~~~/~~~~2~~ | | ~~18~~ | ~~No. 1~~ | ~~4~~~~1~~~~/~~~~4~~ | ~~5~~~~1~~~~/~~~~2~~ | | ~~No. 2~~ | ~~4~~ | ~~4~~~~1~~~~/~~~~2~~ | | ~~No. 3~~ | ~~3~~~~1~~~~/~~~~2~~ | ~~4~~ | | ~~24~~ | ~~No. 1~~ | ~~5~~~~3~~~~/~~~~4~~ | ~~7~~~~1~~~~/~~~~2~~ | | ~~No. 2~~ | ~~5~~~~1~~~~/~~~~2~~ | ~~6~~~~1~~~~/~~~~2~~ | | ~~No. 3~~ | ~~5~~ | ~~5~~~~1~~~~/~~~~2~~ |  |  |  |  | | --- | --- | --- | | ROOF ITEM | WOOD SHINGLES | WOOD SHAKES | | 1. Deck Requirements | Shingles shall be applied to roofs with solid or spaced sheathing. Where spaced sheathing is used, sheathing boards shall not be 4 less than 1”× 4” nominal dimensions and shall be spaced on center equal to the weather exposure to coincide with the placement of fasteners. | Shakes shall be applied to roofs with solid or spaced sheathing. Where spaced sheathing is used, sheathing boards shall not be less than 1” × 4” nominal dimensions and shall be spaced on center equal to the weather exposure to coincide with the placement of fasteners. When 1” × 4” spaced sheathing is installed at 10 inches, boards must be installed between the sheathing boards. | | 2. Interlayment | No requirements. | Interlayment shall comply with ASTM D 226, Type 1. | | 3. Underlayment | Underlayment shall comply with ASTM D 226, Type 1. | No requirements. | | 4. Application | ~~---~~ | ~~---~~ | | Attachment | Fasteners for wood shingles shall be corrosion resistant with a minimum penetration of 3/4 inch into the sheathing. For sheathing less than 1/2 inch thick, the fasteners shall extend through the sheathing a minimum of 3/8 inch. | Fasteners for wood shakes shall be corrosion resistant with a minimum penetration of 3/4 inch into the sheathing. For sheathing less than 1/2 inch thick, the fasteners shall extend through the sheathing a minimum of 3/8 inch. | | No. of fasteners | Wood shingles shall be attached to the roof with two fasteners per shingle, positioned no more than 3/4 inch (19.1 mm) from each edge and no more than 11/2 inch (38.1 mm) above the exposure line. | Wood shakes shall be attached to the roof with two fasteners per shake, positioned no more than 1 inch (25.4 mm) from each edge and no more than 11/2 inches (38.1 mm) above the exposure line |   For SI: 1 inch = 25.4 mm  **R905.7.6 ~~Valley flashing~~ Attachment for *Vasd* as determined in accordance with Section R301.2.1.3 greater than 100 mph.** ~~Roof flashing shall be not less than No. 26 gage [0.019 inches (0.5 mm)] corrosion-resistant sheet metal and shall extend 10 inches (254 mm) from the centerline each way for roofs having slopes less than 12 units vertical in 12 units horizontal (100-percent slope), and 7 inches (178 mm) from the centerline each way for slopes of 12 units vertical in 12 units horizontal and greater. Sections of flashing shall have an end lap of not less than 4 inches (102 mm).~~ Wood shingles installed in accordance with Table R905.7.5 and the requirements of R905.7.6 have an allowable uplift resistance of 45 psf. The installation of wood shingles shall be limited to roofs where the allowable uplift resistance is equal to or greater than the design uplift pressure for the roof listed in Table R301.2(2).  **R905.7.6.1 Fasteners.**  **R905.7.6.1.1 Nails. N**ails to attach the wood shakes shall be 3d stainless-steel ring-shank nails. The nails shall have sufficient length to penetrate through the wood shakes and shall penetrate through the sheathing.  **R905.7.6.1.2 Screws.** Screws to attach the battens to the framing shall be No. 8 by 21/2 inches (64 mm) long corrosion resistant wood screws. Wood screws shall be corrosion resistant screw s conforming to ANSI/ASME B 18.6.1. The corrosion resistance shall meet ASTM A 641, Class 1 or an equal corrosion resistance by coating, electro galvanization, mechanical galvanization, stainless steel, nonferrous metal or other suitable corrosion resistant material.  **R905.7.6.1.3 Wood battens.** 1 Ã— 4 wood battens shall be attached to the wood joists with 2 screws per joist. The first batten shall be located 6 inches (152 mm) from the outer edge of the wood joist. Second batten shall be spaced 1-1/4 inches (32 mm) from the first batten. The remaining battens shall be spaced a maximum 2 inches (51 mm) apart, except the last one which shall be spaced no greater than 3/4 inches (19 mm) from the previous batten.  **R905.7.6.1.4 Shingles.** Shingles shall be attached to the battens with 2 nails for each shingle placed 11/2 inch (38 mm) above the exposure line. The nails shall be 3/4 to 1 inch (19 to 25 mm) from the shingle edges.  **R905.7.7 ~~Label required~~ Application.** ~~Each bundle of shingles shall be identified by a~~ *~~label~~* ~~of an~~ *~~approved~~* ~~grading or inspection bureau or agency.~~ Wood shingles shall be installed according to this chapter and the manufacturer’s installation instructions. Weather exposure for wood shingles shall not exceed those set in Table R905.7.7.  **TABLE R905.7.7**  **WOOD SHINGLE WEATHER EXPOSURE AND ROOF SLOPE**   |  |  |  |  |  | | --- | --- | --- | --- | --- | | **ROOFING MATERIAL** | **LENGTH (inches)** | **GRADE** | **EXPOSURE (inches)** | | | **3:12 pitch to < 4:12** | **4:12 pitch or steeper** | | Shingles of naturally durable wood | 16 | No. 1 | 33/4 | 5 | | No. 2 | 31/2 | 4 | | No. 3 | 3 | 31/2 | | 18 | No. 1 | 41/4 | 51/2 | | No. 2 | 4 | 41/2 | | No. 3 | 31/2 | 4 | | 24 | No. 1 | 53/4 | 71/2 | | No. 2 | 51/2 | 61/2 | | No. 3 | 5 | 51/2 |   For SI: 1 inch = 25.4 mm.  **R905.7.8 Flashing.** At the juncture of the roof and vertical surfaces, flashing and counter flashing shall be provided in accordance with the manufacturer’s installation instructions, and where of metal, shall not be less than 0.017-inch (0.48 mm) (No. 26 galvanized sheet gage) corrosion-resistant metal.  **R905.7.8.1 Valley flashing.** Roof flashing shall be not less than No. 26 gage [0.017 inches (0.48 mm)] corrosion-resistant sheet metal and shall extend 10 inches (254 mm) from the centerline each way for roofs having slopes less than 12 units vertical in 12 units horizontal (100-percent slope), and 7 inches (178 mm) from the centerline each way for slopes of 12 units vertical in 12 units horizontal and greater. Sections of flashing shall have an end lap of not less than 4 inches (102 mm).  **R905.7.9 Label required.** Each bundle of shingles shall be identified by a label of an approved grading or inspection bureau or agency.  **(R5315 AM R1)**  ***Section 9R905.8 Wood shakes. Change to read as shown:***  **R905.8 Wood shakes.** The installation of wood shakes shall comply with the provisions of this section.  **R905.8.1 Deck requirements.** Wood shakes shall be used only on solid or spaced sheathing. Where spaced sheathing is used, sheathing boards shall not be less than 1-inch by 4-inch (25 mm by 102 mm) nominal dimensions and shall be spaced on centers equal to the weather exposure to coincide with the placement of fasteners. Where 1-inch by 4-inch (25 mm by 102 mm) spaced sheathing is installed at 10 inches (254 mm) on center, additional 1-inch by 4-inch (25 mm by 102 mm) boards shall be installed between the sheathing boards.  **R905.8.1.1 Solid sheathing required.** Reserved. ~~In areas where the average daily temperature in January is 25°F (-4°C) or less, solid sheathing is required on that portion of the roof requiring an ice barrier~~  **R905.8.2** **Deck** **slope.** Wood shakes shall only be used on slopes of ~~three~~ four (4) units vertical in twelve (12) units horizontal (~~25~~ 33-percent slope) or greater.  **R905.8.3 Underlayment.** Underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type ~~I or~~ II or Type IV.  **R905.8.3.1 Ice barrier.**Reserved.~~In areas where there has been a history of ice forming along the eaves causing a backup of water as designated in Table R301.2(1), an ice barrier that consists of at least two layers of underlayment cemented together or a self-adhering polymer modified bitumen sheet shall be used in place of normal underlayment and extend from the lowest edges of all roof surfaces to a point at least 24 inches (610 mm) inside the exterior wall line of the building.  Exception: Detached~~ *~~accessory structures~~* ~~that contain no~~ *~~conditioned floor area~~*~~.~~  **R905.8.3.2 Underlayment and high winds.** Reserved.  ~~Underlayment applied in areas subject to high winds [above 110 mph (49 m/s) in accordance with Figure R301.2(4)A] shall be applied with corrosion-resistant fasteners in accordance with manufacturer’s installation instructions. Fasteners are to be applied along the overlap not farther apart than 36 inches (914 mm) on center.  Underlayment installed where the basic wind speed equals or exceeds 120 mph (54 m/s) shall comply with ASTM D 226 Type II. The underlayment shall be attached in a grid pattern of 12 inches (305 mm) between side laps with a 6-inch (152 mm) spacing at the side laps. Underlayment shall be applied in accordance with Section R905.2.7except all laps shall be a minimum of 4 inches (102 mm). Underlayment shall be attached using metal or plastic cap nails with a head diameter of not less than 1 inch (25.4 mm) with a thickness of at least 32-gauge sheet metal. The cap-nail shank shall be a minimum of 12 gauge (0.105 inches) with a length to penetrate through the roof sheathing or a minimum of~~ ~~3~~~~/~~~~4~~ ~~inch (19 mm) into the roof sheathing.~~  **~~Exception:~~** ~~As an alternative, adhered underlayment complying with ASTM D 1970 shall be permitted~~**~~.~~**  **R905.8.3.3 Underlayment Application.** Underlayment shall be installed using one of the following methods:  1.      Two layer underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type II or Type IV: Apply a 19-inch (483 mm) strip of underlayment felt parallel to and starting at the eaves, fastened sufficiently to hold in place. Starting at the eave, apply 36-inch-wide (914 mm) sheets of underlayment, overlapping successive sheets 19 inches (483 mm), and fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with one row in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm).  2.      One layer underlayment shall comply with ASTM D 226, Type II or ASTM D 4869, Type IV: Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches (51 mm), fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with two staggered rows in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). End laps shall be offset by 6 feet (1829 mm).  **R905.8.3.4  Interlayment.** Interlayment shall comply with ASTM D 226, Type I.  **R905.8.4** **~~Interlayment~~ Attachment.** Attachment in accordance with Table R905.7.5 shall be used for roofs with a mean roof height of 40 feet or less and in regions with a *Vasd*, as determined in accordance with Section R301.2.1.3, of 100 mph or less.  ~~Interlayment shall comply with ASTM D 226, Type I.~~  **R905.8.5 Material standards.** Wood shakes shall comply with the requirements of Table R905.8.5.  **TABLE R905.8.5**  **WOOD SHAKE MATERIAL REQUIREMENTS**   |  |  |  | | --- | --- | --- | | **MATERIAL** | **MINIMUM GRADES** | **APPLICABLE  GRADING RULES** | | Wood shakes of naturally durable wood | 1 | Cedar Shake and Shingle Bureau | | Taper sawn shakes of naturally durable wood | 1 or 2 | Cedar Shake and Shingle Bureau | | Preservative-treated shakes and shingles of naturally durable wood | 1 | Cedar Shake and Shingle Bureau | | Fire-retardant-treated shakes and shingles of naturally durable wood | 1 | Cedar Shake and Shingle Bureau | | Preservative-treated taper sawn shakes of Southern pine treated in accordance with AWPA Standard U1 (Commodity Specification A, Use Category 3B and Section 5.6) | 1 or 2 | Forest Products Laboratory of the  Texas Forest Services |   **R905.8.6 Application.** Reserved. ~~Wood shakes shall be installed according to this chapter and the manufacturer’s installation instructions. Wood shakes shall be laid with a side lap not less than 1~~~~1~~~~/~~~~2~~ ~~inches (38 mm) between joints in adjacent courses. Spacing between shakes in the same course shall be~~ ~~3~~~~/~~~~8~~ ~~inch to~~ ~~5~~~~/~~~~8~~ ~~inch (9.5 mm to 15.9 mm) for shakes and tapersawn shakes of naturally durable wood and shall be~~ ~~3~~~~/~~~~8~~ ~~inch to~~ ~~5~~~~/~~~~8~~ ~~inch (9.5 mm to 15.9 mm) for preservative-treated taper sawn shakes. Weather exposure for wood shakes shall not exceed those set forth in Table R905.8.6. Fasteners for wood shakes shall be corrosion-resistant, with a minimum penetration of~~ ~~1~~~~/~~~~2~~ ~~inch (12.7 mm) into the sheathing. For sheathing less than~~ ~~1~~~~/~~~~2~~ ~~inch (12.7 mm) thick, the fasteners shall extend through the sheathing. Wood shakes shall be attached to the roof with two fasteners per shake, positioned no more than 1 inch (25 mm) from each edge and no more than 2 inches (51 mm) above the exposure line.~~  **~~TABLE R905.8.6~~**  **~~WOOD SHAKE WEATHER EXPOSURE AND ROOF SLOPE~~**  Reserved.   |  |  |  |  | | --- | --- | --- | --- | | ~~ROOFING MATERIAL~~ | ~~LENGTH  (inches)~~ | ~~GRADE~~ | ~~EXPOSURE (inches)~~ | | ~~4:12 pitch or steeper~~ | | ~~Shakes of naturally durable wood~~ | ~~18~~ | ~~No. 1~~ | ~~7~~~~1~~~~/~~~~2~~ | | ~~24~~ | ~~No. 1~~ | ~~10~~~~a~~ | | ~~Preservative-treated taper  sawn shakes of Southern Yellow Pine~~ | ~~18~~ | ~~No. 1~~ | ~~7~~~~1~~~~/~~~~2~~ | | ~~24~~ | ~~No. 1~~ | ~~10~~ | | ~~18~~ | ~~No. 2~~ | ~~5~~~~1~~~~/~~~~2~~ | | ~~24~~ | ~~No. 2~~ | ~~7~~~~1~~~~/~~~~2~~ | | ~~Taper-sawn shakes of naturally durable wood~~ | ~~18~~ | ~~No. 1~~ | ~~7~~~~1~~~~/~~~~2~~ | | ~~24~~ | ~~No. 1~~ | ~~10~~ | | ~~18~~ | ~~No. 2~~ | ~~5~~~~1~~~~/~~~~2~~ | | ~~24~~ | ~~No. 2~~ | ~~7~~~~1~~~~/~~~~2~~ | | ~~For SI: 1 inch = 25.4 mm.~~ | | | | | | | ~~a. For 24-inch by~~ ~~3~~~~/~~~~8~~~~-inch handsplit shakes, the maximum exposure is 7~~~~1~~~~/~~~~2~~ ~~inches.~~ | | | | | |   **R905.8.7 ~~Shake placement~~ Attachment for** ***Vasd* as** **determined** **in** **accordance with Section R301.2.1.3** **greater than** **100** **mph.**Wood shakes installed in accordance with Table R905.7.5 and the requirements of R905.8.7 have an allowable uplift resistance of 90 psf. The installation of wood shakes shall be limited to roofs where the allowable uplift resistance is equal to or greater than the design uplift pressure for the roof listed in Table R301.2(2). ~~The starter course at the eaves shall be doubled and the bottom layer shall be either 15-inch (381 mm), 18-inch (457 mm) or 24-inch (610 mm) wood shakes or wood shingles. Fifteen-inch (381 mm) or 18-inch (457 mm) wood shakes may be used for the final course at the ridge. Shakes shall be interlaid with 18-inch-wide (457 mm) strips of not less than No. 30 felt shingled between each course in such a manner that no felt is exposed to the weather by positioning the lower edge of each felt strip above the butt end of the shake it covers a distance equal to twice the weather exposure~~.  **R905.8.7.1 Fasteners.**  **R905.8.7.1.1** **Nails**. Nails to attach the wood shakes shall be 6d stainless-steel ring-shank nails. The nails shall have sufficient length to penetrate through the wood shakes and shall penetrate through the sheathing.  **R905.8.7.1.2** **Screws.** Screws to attach the battens to the framing shall be No. 8 by 2 1/2 inches long corrosion resistant wood screws. Wood screws shall be corrosion resistant screws conforming to ANSI/ASME B 18.6.1. The corrosion resistance shall meet ASTM A 641, Class 1 or an equal corrosion resistance by coating, electro galvanization, mechanical galvanization, stainless steel, nonferrous metal or other suitable corrosion resistant material.  **R905.8.7.1.3 Wood** **battens.** 1 × 6 wood battens shall be attached to the wood joists with 2 screws per joist. The first batten shall be located 6 inches from the outer edge of the wood joist. The second batten shall be spaced 1-1/4 inches from the first batten. The remaining battens shall be spaced a maximum 2 inches apart, except the last one, which shall be spaced no greater than 3/4 inches from the previous batten.  **R905.8.7.1.4** **Shakes.** Shakes shall be attached to the battens with 2 nails for each shake placed 11/2 inch above the exposure line. The nails shall be 3/4 to 1 inch from the shake edges.  **R905.8.8 ~~Valley flashing~~ Application.** Wood shakes shall be laid with a side lap not less than 11/2 inches (38 mm) between joints in adjacent courses. Spacing between shakes in the same course shall be 3/8 to 5/8 inches (9.5 to 15.9 mm) for shakes and taper sawn shakes of naturally durable wood and shall be 1/4 to 3/8 inch (6.4 to 9.5 mm) for preservative taper sawn shakes. Weather exposure for wood shakes shall not exceed those set in Table R905.8.8.  ~~Roof valley flashing shall not be less than No. 26 gage [0.019 inch (0.5 mm)] corrosion-resistant sheet metal and shall extend at least 11 inches (279 mm) from the centerline each way. Sections of flashing shall have an end lap of not less than 4 inches (102 mm).~~  **TABLE R905.8.8**  **WOOD SHAKE WEATHER EXPOSURE AND ROOF SLOPE**   |  |  |  |  |  | | --- | --- | --- | --- | --- | | **ROOFING MATERIAL** | **LENGTH**  **(inches)** | **GRADE** | **EXPOSURE (inches)** | | | **4:12 pitch or steeper** | | | Shakes of naturally durable wood | 18 | No. 1 | 71/2 | | | 24 | No. 1 | 10a | | | Preservative-treated taper sawn shakes of Southern Yellow Pine | 18 | No. 1 | 71/2 | | | 24 | No. 1 | 10 | | | 18 | No. 2 | 51/2 | | | 24 | No. 2 | 71/2 | | | Taper-sawn shakes of naturally durable wood | 18 | No. 1 | 71/2 | | | 24 | No. 1 | 10 | | | 18 | No. 2 | 51/2 | | | 24 | No. 2 | 71/2 | | | For SI: 1 inch = 25.4 mm. | | | | | | a. For 24-inch by 3/8-inch handsplit shakes, the maximum exposure is 71/2 inches. | | | | |   **R905.8.9 Label required.** Each bundle of shakes shall be identified by a *label* of an *approved* grading or inspection bureau or agency.  **R905.8.10** **Flashing.** At the juncture of the roof and vertical surfaces, flashing and counter flashing shall be provided in accordance with the manufacturer’s installation instructions, and where of metal, shall not be less than 0.017-inch (0.48 mm) (No. 26 galvanized sheet gage) corrosion-resistant metal.  **R905.8.10.1 Valley flashing.** Valley flashing shall extend at least 11 inches (279 mm) from the centerline each way and have a splash diverter rib not less than 1 inch (25 mm) high at the flow line formed as part of the flashing. Sections of flashing shall have an end lap of not less than 4 inches (102 mm). For roof slopes of four (4) units vertical in twelve (12) units horizontal (33-percent slope) and over, the valley flashing shall have a 36-inch-wide (914 mm) underlayment of one layer of ASTM D 226 Type I underlayment running the full length of the valley, in addition to other required underlayment per Table R903.2.1 Valley flashing and flashing metal shall be a minimum thickness as provided in Table R903.2.1 for nonferrous metal or stainless steel.  **(R5314 AM R1)**  ***Section R905.9 Built-up roofs. Change to read as shown:***  **R905.9 Built-up roofs.** The installation of built-up roofs shall comply with the provisions of this section.  **R905.9.1 Slope.** Built-up roofs shall have a design slope of a minimum of one-fourth unit vertical in 12 units horizontal (2-percent slope) for drainage, except for coal-tar built-up roofs, which shall have a design slope of a minimum one-eighth unit vertical in 12 units horizontal (1-percent slope).  **R905.9.2 Material standards.** Built-up roof covering materials shall comply with the standards in Table R905.9.2 or UL 55A.  **R905.9.2.1** Red rosin paper shall be used when the membrane is applied directly to a wood deck or cementitious fiber decks.  **TABLE R905.9.2**  **BUILT-UP ROOFING MATERIAL STANDARDS**   |  |  | | --- | --- | | **MATERIAL STANDARD** | **STANDARD** | | Acrylic coatings used in roofing | ASTM D 6083 | | Aggregate surfacing | ASTM D 1863 | | Asphalt adhesive used in roofing | ASTM D 3747 | | Asphalt cements used in roofing | ASTM D 2822; D 3019; D 4586 | | Asphalt-coated glass fiber base sheet | ASTM D 4601 | | Asphalt coatings used in roofing | ASTM D 1227; D 2823; D 2824; D 4479 | | Asphalt glass felt | ASTM D 2178 | | Asphalt primer used in roofing | ASTM D 41 | | Asphalt-saturated and asphalt-coated organic felt base sheet | ASTM D 2626 | | Asphalt-saturated organic felt (perforated) | ASTM D 226 | | Asphalt used in roofing | ASTM D 312 | | Coal-tar cements used in roofing | ASTM D 4022; D 5643 | | Coal-tar primer used in roofing, dampproofing and waterproofing | ASTM D 43 | | Coal-tar saturated organic felt | ASTM D 227 | | Coal-tar used in roofing | ASTM D 450, Type I or II | | Glass mat, coal tar | ASTM D 4990 | | Glass mat, venting type | ASTM D 4897 | | Mineral-surfaced inorganic cap sheet | ASTM D 3909 | | Thermoplastic fabrics used in roofing | ASTM D 5665; D 5726 |   **R905.9.3 Application.** [No change to IRC text] ***(R5303 AS)***  ***Section 905.10 Metal roof panels. Change to read as shown:***  **R905.10 Metal roof panels.** The installation of metal roof panels shall comply with the provisions of this section.  **R905.10.1 Deck requirements.** Metal roof panel roof coverings shall be applied to solid or spaced sheathing, except where the roof covering is specifically designed to be applied to spaced supports.  **R905.10.2 Slope.** Minimum slopes for metal roof panels shall comply with the following:  1. The minimum slope for lapped, nonsoldered-seam metal roofs without applied lap sealant shall be three units vertical in 12 units horizontal (25-percent slope).  2. The minimum slope for lapped, nonsoldered-seam metal roofs with applied lap sealant shall be one-half vertical unit in 12 units horizontal (4-percent slope). Lap sealants shall be applied in accordance with the *approved* manufacturer’s installation instructions.  3. The minimum slope for standing-seam roof systems shall be one-quarter unit vertical in 12 units horizontal (2-percent slope).  **R905.10.3 Material standards.** Metal-sheet roof covering systems that incorporate supporting structural members shall be designed in accordance with the *~~International Building Code~~  Florida* *Building Code,* *Building.* Metal-sheet roof coverings installed over structural decking shall comply with Table R905.~~10.3(1)~~ 4.4. The materials used for metal-sheet roof coverings shall be naturally corrosion resistant or provided with corrosion resistance in accordance with the standards and minimum thicknesses shown in Table R905.~~10.3(2)~~ 4.4.  **TABLE R905.10.3(1)**  **METAL ROOF COVERING STANDARDS**  **Reserved**.   |  |  |  | | --- | --- | --- | | **~~ROOF COVERING TYPE~~** | **~~STANDARD APPLICATION RATE/THICKNESS~~** | | | ~~Galvanized steel~~ | ~~ASTM A 653 G90 Zinc coated~~ | | | ~~Stainless steel~~ | ~~ASTM A 240, 300 Series alloys~~ | | | ~~Steel~~ | ~~ASTM A 924~~ | | | ~~Lead-coated copper~~ | ~~ASTM B 101~~ | | | ~~Cold-rolled copper~~ | ~~ASTM B 370 minimum 16 oz/sq ft and 12 oz/sq ft high-yield copper for metal-sheet roof-covering systems; 12 oz/sq ft for preformed metal shingle systems.~~ | | | ~~Hard lead~~ | ~~2 lb/sq ft~~ | | | ~~Soft lead~~ | ~~3 lb/sq ft~~ | | | ~~Aluminum~~ | ~~ASTM B 209, 0.024 minimum thickness for roll-formed panels and 0.019-inch minimum thickness for pressformed shingles.~~ | | | ~~Terne (tin) and terne-coated stainless~~ | ~~Terne coating of 40 lb per double base box, field painted where applicable in accordance with manufacturer’s installation instructions.~~ | | | ~~Zinc~~ | ~~0.027 inch minimum thickness: 99.995% electrolytic high-grade zinc with alloy additives of copper (0.08 - 0.20%), titanium (0.07% - 0.12%) and aluminum (0.015%).~~ | | | ~~For SI: 1 ounce per square foot = 0.305 kg/m~~~~2~~~~, 1 pound per square foot = 4.214 kg/m~~~~2~~~~, 1 inch = 25.4 mm, 1 pound = 0.454 kg.~~ | | |   **TABLE R905.10.3(2)**  **MINIMUM CORROSION RESISTANCE**  Reserved.   |  |  |  | | --- | --- | --- | | ~~55% aluminum-zinc alloy coated steel~~ | ~~ASTM A 792 AZ 50~~ | | | ~~5% aluminum alloy-coated steel~~ | ~~ASTM A 875 GF60~~ | | | ~~Aluminum-coated steel~~ | ~~ASTM A 463 T2 65~~ | | | ~~Galvanized steel~~ | ~~ASTM A 653 G-90~~ | | | ~~Prepainted steel~~ | ~~ASTM A 755~~~~a~~ | | | ~~a. Paint systems in accordance with ASTM A 755 shall be applied over steel products with corrosion-resistant coatings complying with ASTM A 792, ASTM A 875, ASTM A 463, or ASTM A 653.~~ | | |   **R905.10.4** **Attachment.** Metal roof panels shall be secured to the supports in accordance with this chapter and the manufacturer’s installation instructions. Metal roofing fastened directly to steel framing shall be attached by approved fasteners. ~~In the absence of manufacturer’s installation instructions, t~~ The following fasteners shall be used:  1. Galvanized fasteners shall be used for galvanized roofs.  2. Hard ~~C~~ copper ~~,brass, bronze,~~ or copper alloy or 300 series stainless steel fasteners shall be used for copper roofs.  3. Stainless steel fasteners are acceptable for all types of metal roofs.  4. Aluminum-zinc coated fasteners are acceptable for aluminum-zinc coated roofs.  **R905.10.5 Underlayment.** Underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type II or Type IV or ASTM D 1970 or ASTM D 6757.  **R905.10.5.1 Underlayment and high winds.** Reserved.  ~~Underlayment applied in areas subject to high winds [above 110 mph (49 m/s) in accordance with Figure R301.2(4)A] shall be applied with corrosion-resistant fasteners in accordance with manufacturer’s installation instructions. Fasteners are to be applied along the overlap not farther apart than 36 inches (914 mm) on center.   Underlayment installed where the basic wind speed equals or exceeds 120 mph (54 m/s) shall comply with ASTM D 226 Type II. The underlayment shall be attached in a grid pattern of 12 inches (305 mm) between side laps with a 6-inch (152 mm) spacing at the side laps. Underlayment shall be applied in accordance with Section R905.2.7except all laps shall be a minimum of 4 inches (102 mm). Underlayment shall be attached using metal or plastic cap nails with a head diameter of not less than 1 inch (25.4 mm) with a thickness of at least 32-gauge sheet metal. The cap-nail shank shall be a minimum of 12 gauge (0.105 inches) with a length to penetrate through the roof sheathing or a minimum of~~ ~~3~~~~/~~~~4~~ ~~inch (19 mm) into the roof sheathing.~~  **~~Exception:~~** ~~As an alternative, adhered underlayment complying with ASTM D 1970 shall be permitted~~**~~.~~**  **R905.10.5.2 Underlayment Application.** Underlayment shall be installed using one of the following methods:  1.      Two layer underlayment shall comply with ASTM D 226, Type I or Type II or ASTM D 4869, Type II or Type IV or ASTM D 6757: Apply a 19-inch (483 mm) strip of underlayment felt parallel to and starting at the eaves, fastened sufficiently to hold in place. Starting at the eave, apply 36-inch-wide (914 mm) sheets of underlayment, overlapping successive sheets 19 inches (483 mm), and fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with one row in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations.  2.      One layer underlayment shall comply with ASTM D 226, Type II or ASTM D 4869, Type IV or ASTM D 6757: Underlayment shall be applied shingle fashion, parallel to and starting from the eave and lapped 2 inches (51 mm), fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with two staggered rows in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations End laps shall be offset by 6 feet (1829 mm).  3.      As an alternative, the entire roof deck shall be covered with an approved self-adhering polymer modified bitumen sheet meeting [ASTM D 1970](javascript:vo();) or an approved self-adhering synthetic underlayment installed in accordance with the manufacturer’s installation instructions.  **(R5306 AM R1)**   |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | | ***Section R905.11 Modified bitumen roofing. Change to read as shown:***  **R905.11 Modified bitumen roofing.** The installation of modified bitumen roofing shall comply with the provisions of this section.  **R905.11.1 Slope.** Modified bitumen membrane roofs shall have a design slope of a minimum of one-fourth unit vertical in 12 units horizontal (2-percent slope) for drainage.  **R905.11.2 Material standards.** Modified bitumen roof coverings shall comply with the standards in Table R905.11.2.  **TABLE R905.11.2**  **MODIFIED BITUMEN ROOFING MATERIAL STANDARDS**   |  |  | | --- | --- | | **MATERIAL** | **STANDARD** | | Acrylic coating | ASTM D 6083 | | Asphalt adhesive | ASTM D 3747 | | Asphalt cement | ASTM D 3019 | | Asphalt coating | ASTM D 1227; D 2824 | | Asphalt primer | ASTM D 41 | | Modified bitumen roof membrane | ASTM D 6162; D 6163; D 6164; D 6222; D 6223; D 6298; **D 6509** CSB 37â€"GP€"56M |   **R905.11.3 Application.** Modified bitumen roofs shall be installed according to this chapter and the manufacturer’s installation instructions. The approved allowable uplift resistance for the modified bitumen roof shall be equal to or greater than the uplift resistance for the roof based on Table R301.2(2).  **(R5309 AS)**  ***Section R905.12 Thermoset single-ply roofing. Change to read as shown:***  **R905.12 Thermoset single-ply roofing.** The installation of thermoset single-ply roofing shall comply with the provisions of this section.  **R905.12.1 Slope.** Thermoset single-ply membrane roofs shall have a design slope of a minimum of one-fourth unit vertical in 12 units horizontal (2-percent slope) for drainage.  **R905.12.2 Material standards.** Thermoset single-ply roof coverings shall comply with ASTM D 4637, ASTM D 5019 or CGSB 37-GP-52M.  **R905.12.3 Application.** Thermoset single-ply roofs shall be installed according to this chapter and the manufacturer’s installation instructions. The approved allowable uplift resistance for the thermoset single-ply membrane roof shall be equal to or greater than the uplift resistance for the roof based on Table R301.2(2).  **(R5310 AS)**  ***Section R905.13 Thermoplastic sing-ply roofing. Change to read as shown:***  **R905.13 Thermoplastic single-ply roofing.** The installation of thermoplastic single-ply roofing shall comply with the provisions of this section.  **R905.13.1 Slope.** Thermoplastic single-ply membrane roofs shall have a design slope of a minimum of one-fourth unit vertical in 12 units horizontal (2-percent slope).  **R905.13.2 Material standards.** Thermoplastic single-ply roof coverings shall comply with ASTM D 4434, ASTM D 6754, ASTM D 6878 or CGSB CAN/CGSB 37.54.  **R905.13.3 Application.** Thermoplastic single-ply roofs shall be installed according to this chapter and the manufacturer’s installation instructions. The approved allowable uplift resistance for the thermoplastic single-ply roof shall be equal to or greater than the uplift resistance for the roof based on Table R301.2(2).  **(R5311 AS)**  ***Section R905.14 Sprayed polyurethane foam roofing. Change to read as shown:***  **R905.14 Sprayed polyurethane foam roofing.** The installation of sprayed polyurethane foam roofing shall comply with the provisions of this section.  **R905.14.1 Slope.** Sprayed polyurethane foam roofs shall have a design slope of a minimum of one-fourth unit vertical in 12 units horizontal (2-percent slope) for drainage.  **R905.14.2 Material standards.** Spray-applied polyurethane foam insulation shall comply with ASTM C 1029, Type III or IV.  **R905.14.3 Application.** Foamed-in-place roof insulation shall be installed in accordance with this chapter and the manufacturer’s installation instructions. A liquid-applied protective coating that complies with ~~Table R905.14.3~~ Section R905.15 shall be applied no less than 2 hours nor more than 72 hours following the application of the foam. The approved allowable uplift resistance for the sprayed polyurethane foam roofing shall be equal to or greater than the uplift resistance for the roof based on Table R301.2(2). | |  |   **TABLE R905.14.3**  **PROTECTIVE COATING MATERIAL STANDARDS**  Reserved.   |  |  | | --- | --- | | **~~MATERIAL~~** | **~~STANDARD~~** | | ~~Acrylic coating~~ | ~~ASTM D 6083~~ | | ~~Silicone coating~~ | ~~ASTM D 6694~~ | | ~~Moisture-cured polyurethane coating~~ | ~~ASTM D 6947~~ |   **R905.14.4 Foam plastics.** Foam plastic materials and installation shall comply with Section R316.  **(R5313 AM R3)**  ***Section R905.15 Liquid-applied roofing. Change to read as shown:***  **R905.15 Liquid-applied roofing.** The installation of liquid-applied roofing shall comply with the provisions of this section.  **R905.15.1 Slope.** Liquid-applied roofing shall have a design slope of a minimum of one-fourth unit vertical in 12 units horizontal (2-percent slope).  **R905.15.2 Material standards.** Liquid-applied roofing shall comply with ASTM C 836, C 957, D 1227, D 3468, D 6083, D 6694 or D 6947.  **R905.15.3 Application.** Liquid-applied roof coatings shall be installed according to this chapter and the manufacturer’s installation instructions. The approved allowable uplift resistance for the liquid-applied coatings shall be equal to or greater than the uplift resistance for the roof based on Table R301.2(2).  **(R5305 AS)**  ***Section R905.16 Photovoltaic modules/shingles. Change to read as shown:***  **R905.16** **Building integrated ~~P~~**  **photovoltaic roofing** **modules/shingles.** The installation of building integrated photovoltaic roofing modules/shingles shall comply with the provisions of this section.  **R905.16.1** **Material** **standards.** Building integrated ~~P~~ photovoltaic roofing modules/shingles shall be listed and labeled in accordance with UL 1703.  **R905.16.2** **Attachment.** Building integrated ~~P~~ photovoltaic roofing modules/shingles shall be attached in accordance with the manufacturer’s installation instructions.  **R905.16.3 Wind resistance.** Building integrated ~~P~~ photovoltaic roofing modules/shingles shall be tested in accordance with procedures and acceptance criteria in ASTM D 3161 or TAS 107. Building integrated ~~P~~ photovoltaic roofing modules/shingles shall comply with the classification requirements of Table R905.2 ~~4.1(2)~~ .6.1 for the appropriate maximum basic wind speed. Building integrated ~~P~~ photovoltaic roofing modules/shingle packaging shall bear a label to indicate compliance with the procedures in ASTM D 3161 or TAS 107 and the required classification from Table R905.2 ~~4.1(2)~~ .6.1.  **(R5552 AS)**  ***Section R905.17. Add a section to read as shown:***  **R905.17 Photovoltaic systems.** Rooftop mounted photovoltaic systems shall be designed in accordance with this section.  **R905.17.1 Wind resistance.** Rooftop mounted photovoltaic systems shall be designed for wind loads for component and cladding in accordance with Chapter 16 using an effective wind area based on the dimensions of a single unit frame.  **R905.17.2 Fire classification.** Rooftop mounted photovoltaic systems shall have the same fire classification as the roof assembly required by Section R902.  **R905.17.3 Installation.**Rooftop mounted photovoltaic systems shall be installed in accordance with the manufacturer's installation instructions.  **R905.17.4 Photovoltaic panels and modules.** Photovoltaic panels and modules mounted on top of a roof shall be listed and labeled in accordance with UL 1703 and shall be installed in accordance with the manufacturer's installation instructions.  **(R5553 AS)** |
|  |

**SECTION R907**

**REROOFING**

***Section R907 Reroofing. Change to read as shown:***

**R907.1 General.** Materials and methods of application used for re-covering or replacing an existing roof covering shall comply with the requirements of Chapter 9 of the *Florida Building Code, Residential*.

**Exception:** Reroofing shall not be required to meet the minimum design slope requirement of one-quarter unit vertical in 12 units horizontal (2-percent slope) in Section R905 for roofs that provide positive roof drainage.

**R907.2 Structural and construction loads.** The structural roof components shall be capable of supporting the roof covering system and the material and equipment loads that will be encountered during installation of the roof covering system.

**R907.3 Recovering versus replacement.** New roof coverings shall not be installed without first removing all existing layers of roof coverings where any of the following conditions ~~exist~~ occur:

1. Where the existing roof or roof covering is water-soaked or has deteriorated to the point that the existing roof or roof covering is not adequate as a base for additional roofing.

2. Where the existing roof covering is wood shingle or shake, slate, clay, cement or asbestos-cement tile.

3. Where the existing roof has two or more applications of any type of roof covering.

4. When blisters exist in any roofing, unless blisters are cut or scraped open and remaining materials secured down before applying additional roofing.

5. Where the existing roof is to be used for attachment for a new roof system and compliance with the securement provisions of Section R905 cannot be met.  
  
**Exceptions:**

1. Complete and separate roofing systems, such as standing-seam metal roof systems, that are designed to transmit the roof loads directly to the building’s structural system and that do not rely on existing roofs and roof coverings for support, shall not require the removal of existing roof coverings.

2. Reserved. ~~Installation of metal panel, metal shingle and concrete and clay tile roof coverings over existing wood shake roofs shall be permitted when the application is in accordance with Section R907.4.~~

3. The application of new protective coating over existing spray polyurethane foam roofing systems shall be permitted without tear-off of existing roof coverings.

4. Reserved. ~~Where the existing roof assembly includes an ice barrier membrane that is adhered to the roof deck, the existing ice barrier membrane shall be permitted to remain in place and covered with an additional layer of ice barrier membrane in accordance with Section R905.~~

5. Roof Coating. Application of elastomeric and or maintenance coating systems over existing asphalt shingles shall be in accordance with the shingle manufacturer’s approved installation instructions.

**R907.4 Roof recovering.** Reserved. ~~Where the application of a new roof covering over wood shingle or shake roofs creates a combustible concealed space, the entire existing surface shall be covered with gypsum board, mineral fiber, glass fiber or other~~ *~~approved~~* ~~materials securely fastened in place.~~   
  
**R907.5 Reinstallation of materials.** Existing slate, clay or cement tile shall be permitted for reinstallation, except that damaged, cracked or broken slate or tile shall not be reinstalled. Any existing flashings, edgings, outlets, vents or similar devices that are a part of the assembly shall be replaced when rusted, damaged or deteriorated. Aggregate surfacing materials shall not be reinstalled.

**R907.6 Flashings.** Flashings shall be reconstructed in accordance with *approved* manufacturer’s installation instructions. Metal flashing to which bituminous materials are to be adhered shall be primed prior to installation.

**R907.7** When a roof covering on an existing site-built single-family residential structure is removed and replaced, the following procedures shall be permitted to be performed by the roofing contractor:

(a) Roof-decking attachment shall be as required by Section R907.7.1.

(b) A secondary water barrier shall be provided as required by Section R907.7.2.   
  
**Exception**: Single family residential structures permitted subject to the *Florida Building Code* are not required to comply with this section.

**R907.7.1 Roof decking attachment for site-built single-family residential structures.** For site-built single-family residential structures the fastening shall be in accordance with Section R907.7.1.1 or R907.7.1.2 as appropriate for the existing construction. 8d nails shall be a minimum of 0.113 inch (2.9 mm) in diameter and shall be a minimum of 21/4 inch (57 mm) long to qualify for the provisions of this section for existing nails regardless of head shape or head diameter.

**R907.7.1.1** Roof decking consisting of sawn lumber or wood planks up to 12” wide and secured with at least two nails (minimum size 8d) to each roof framing member it crosses shall be deemed to be sufficiently connected. Sawn lumber or wood plank decking secured with smaller fasteners than 8d nails or with fewer than two nails (minimum size 8d) to each framing member it crosses shall be deemed sufficiently connected if fasteners are added such that two clipped head, round head, or ring shank nails (minimum size 8d) are in place on each framing member it crosses.

**R907.7.1.2** For roof decking consisting of wood structural panels, fasteners and spacing required in columns 3 and 4 of Table R907.7.1.2 are deemed to comply with the indicated design wind speed range. Wood structural panel connections retrofitted with a two part urethane based closed cell adhesive sprayed onto the joint between the sheathing and framing members are deemed to comply provided testing using the manufacturer’s recommended application on panels connected with 6d smooth shank nails at no more than a 6-inch edge and 12-inch field spacing demonstrate an uplift resistance of a minimum of 200 psf.   
  
Supplemental fasteners as required by Table R907.7.1.2 shall be 8d ring shank nails with round heads and the following minimum dimensions:

1. 0.113-inch nominal shank diameter.

2. ~~Difference between root and ring diameter a minimum of 5% of root nail diameter~~. Ring diameter a minimum of 0.010-inch over shank diameter~~.~~

3. 16 to 20 rings per inch.

4. A minimum 0.280-inch full round head diameter.

5. Ring shank to extend a minimum of 11/2 inches from the tip of the nail.

6. Minimum 2 3/8 ~~-~~~~1~~~~/~~~~4~~ inch nail length.

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**TABLE R907.7.1.2**

**SUPPLEMENT FASTENERS AT PANEL EDGES AND INTERMEDIATE FRAMING**

|  |  |  |  |
| --- | --- | --- | --- |
| **EXISTING FASTENERS** | **EXISTING SPACING** | **Vasd** **110 MPH OR LESS SUPPLEMENTAL FASTENER SPACING SHALL BE NO GREATER THAN** | **Vasd** **GREATER THAN 110 MPH SUPPLEMENTAL FASTENER SPACING SHALL BE NO GREATER THAN** |
| Staples or 6d | Any | 6?o.c.b | 6?o.c.b |
| 8d clipped head, round head, smooth or ring shank | 6?o.c. or less | None necessary | None necessary |
| 8d clipped head, round head, smooth or ring shank | Greater than 6?o.c. | 6?o.c.a | 6?o.c.a |
| For SI: 1 inch = 25.4 mm. | | | |
| a. Maximum spacing determined based on existing fasteners and supplemental fasteners. | | | |
| b. Maximum spacing determined based on supplemental fasteners only. | | | |
| c. Vasd shall be determined in accordance with Section 1609.3.1 of the *Florida Building Code, Building* or Section R301.2.1.3 of the *Florida Building Code, Residential.* | | | |

**R907.7.2 Roof secondary water barrier for site-built single family residential structures.** A secondary water barrier shall be installed using one of the following methods when roof covering is removed and replaced:

1. In either HVHZ or Non-HVHZ regions:

a) All joints in structural panel roof sheathing or decking shall be covered with a minimum 4 inch (102 mm) wide strip of self-adhering polymer modified bitumen tape applied directly to the sheathing or decking. The deck and self-adhering polymer modified bitumen tape shall be covered with one of the underlayment systems approved for the particular roof covering to be applied to the roof.

b) The entire roof deck shall be covered with an approved asphalt impregnated 30# felt underlayment or approved synthetic underlayment installed with nails and tin-tabs in accordance with Sections R4402.7.2, R4402.7.3, or R4402.7.4 of the *Florida Building Code, Residential.* (No additional underlayment shall be required over the top of this sheet.) The synthetic underlayment shall be fastened in accordance with the manufacturer’s recommendations.

2. Outside the High Velocity Hurricane Zone:

a) The entire roof deck shall be covered with an approved self-adhering polymer modified bitumen sheet meeting ASTM D 1970 or an approved self-adhering synthetic underlayment installed in accordance with the manufacturer’s installation instructions. No additional underlayment shall be required on top of this sheet for new installations.

b) An underlayment system approved for the particular roof covering shall be applied with the following modification:

(1) For roof slopes that require one layer of underlayment, a layer of approved asphalt impregnated ASTM D 226 Type I or Type II, ASTM D 4869, Type II or Type IV underlayment or approved synthetic underlayment shall be installed. The felt is to be fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs attached to a nailable deck with two staggered rows in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). Synthetic underlayment shall be fastened in accordance with this section and the manufacturer’s recommendations.

(2) For roof slopes that require two layers of underlayment, an approved asphalt impregnated ASTM D 226 Type I or Type II, ASTM D 4869, Type II or Type IV underlayment shall be installed in a shingle–fashion and lapped 19 inch (483 mm) and fastened with 1 inch (25 mm) round plastic cap, metal cap nails or nails and tin-tabs, attached to a nailable deck with one row in the field of the sheet with a maximum fastener spacing of 12 in. o.c. (305 mm), and one row at the overlaps fastened 6 in. o.c. (152 mm). An approved synthetic underlayment shall be installed in accordance with this section and the manufacturer’s installation instruction. (No additional underlayment shall be required over the top of this sheet.)   
  
**Exceptions:**

1. Roof slopes < 2:12 having a continuous roof system shall be deemed to comply with Section R907.7.2 requirements for a secondary water barrier.

2. Clay and concrete tile roof systems installed as required by the *Florida Building Code, Residential* are deemed to comply with the requirements of Section R907.7.2 for Secondary Water Barriers.

**R907.8** When a roof covering on an existing site-built-single-family residential structure is removed and replaced on a building that is located in the wind-borne debris region as defined in the *Florida Building Code, Building* and that has an insured value of $300,000 or more or, if the building is uninsured or for which documentation of insured value is not presented, has a just valuation for the structure for purposes of ad valorem taxation of $300,000 or more:

(a) Roof to wall connections shall be improved as required by Section R907.8.1

(b) Mandated retrofits of the roof-to-wall connection shall not be required beyond a 15 percent increase in the cost of re-roofing.   
  
**Exception:** Single-family residential structures permitted subject to the *Florida Building Code* are not required to comply with this section.

**R907.8.1 Roof-to-wall connections for site-built single-family residential structures.** Where required by Section R907.8, the intersection of roof framing with the wall below shall provide sufficient resistance to meet the uplift loads specified in Table R907.8.1 either because of existing conditions or through retrofit measures. As an alternative to an engineered design, the prescriptive retrofit solutions provided in Sections R907.8.1.1 through R907.8.1.7 shall be accepted as meeting the mandated roof-to-wall retrofit requirements.

**Exceptions:**

1. Where it can be demonstrated (by code adoption date documentation and permit issuance date) that roof-to-wall connections and/or roof-to-foundation continuous load path requirements were required at the time of original construction.

2. Roof-to-wall connections shall not be required unless evaluation and installation of connections at gable ends or all corners can be completed for 15 percent of the cost of roof replacement.

**TABLE R907.8.1**

**REQUIRED UPLIFT CAPACITIES FOR ROOF-TO-WALL CONNECTIONSa, b**

**(POUNDS PER LINEAR FOOT)**

|  |  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- | --- |
|  | **Ultimate Design Wind Speed, Vult** | **Roof Span (feet)** | | | | | | | **Overhangs** |
| **12** | **20** | **24** | **28** | **32** | **36** | **40** |
| Within 6 feet of building corner | 85 | -69.85 | -116.42 | -139.70 | -162.99 | -186.27 | -209.55 | -232.84 | -27 |
| 90 | -82.67 | -137.78 | -165.34 | -192.90 | -220.45 | -248.01 | -275.57 | -30.3 |
| 100 | -110.51 | -184.18 | -221.01 | -257.85 | -294.68 | -331.52 | -368.36 | -37.4 |
| 110 | -141.27 | -235.45 | -282.55 | -329.64 | -376.73 | -423.82 | -470.91 | -45.3 |
| 120 | -174.97 | -291.62 | -349.94 | -408.26 | -466.59 | -524.91 | -583.23 | -53.9 |
| 130 | -211.60 | -352.66 | -423.19 | -493.72 | -564.26 | -634.79 | -705.32 | -63.2 |
| 140 | -251.15 | -418.59 | -502.31 | -586.02 | -669.74 | -753.46 | -837.18 | -73.3 |
| 150 | -293.64 | -489.40 | -587.28 | -685.16 | -783.04 | -880.92 | -978.80 | -84.2 |
| 170 | -387.40 | -645.67 | -774.81 | -903.94 | -1033.08 | -1162.21 | -1291.35 | -108 |
| Greater than 6 feet from building corner | 85 | -39.10 | -65.17 | -78.20 | -91.24 | -104.27 | -117.30 | -130.34 | -27 |
| 90 | -48.20 | -80.33 | -96.39 | -112.46 | -128.52 | -144.59 | -160.66 | -30.3 |
| 100 | -67.95 | -113.24 | -135.89 | -158.54 | -181.19 | -203.84 | -226.49 | -37.4 |
| 110 | -89.78 | -149.63 | -179.55 | -209.48 | -239.40 | -269.33 | -299.25 | -45.3 |
| 120 | -113.68 | -189.47 | -227.37 | -265.26 | -303.16 | -341.05 | -378.94 | -53.9 |
| 130 | -139.67 | -232.78 | -279.34 | -325.90 | -372.45 | -419.01 | -465.57 | -63.2 |
| 140 | -167.74 | -279.56 | -335.47 | -391.38 | -447.29 | -503.21 | -559.12 | -73.3 |
| 150 | -197.88 | -329.80 | -395.76 | -461.72 | -527.68 | -593.64 | -659.60 | -84.2 |
| 170 | -264.41 | -440.68 | -528.81 | -616.95 | -705.08 | -793.22 | -881.35 | -108 |

|  |  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- | --- |
| **~~ULTIMATE DESIGN WIND SPEED, V~~~~ult~~** | **~~ROOF SPAN (feet)~~** | | | | | | | **~~OVERHANGS~~** |
| **~~12~~** | **~~20~~** | **~~24~~** | **~~28~~** | **~~32~~** | **~~36~~** | **~~40~~** |
| ~~Within 6 feet of building corner~~ | ~~85~~ | ~~-69.85~~ | ~~-116.42~~ | ~~-139.70~~ | ~~-162.99~~ | ~~-186.27~~ | ~~-209.55~~ | ~~-232.84~~ |
| ~~90~~ | ~~-82.67~~ | ~~-137.78~~ | ~~-165.34~~ | ~~-192.90~~ | ~~-220.45~~ | ~~-248.01~~ | ~~-275.57~~ |
| ~~100~~ | ~~-110.51~~ | ~~-184.18~~ | ~~-221.01~~ | ~~-257.85~~ | ~~-294.68~~ | ~~-331.52~~ | ~~-368.36~~ |
| ~~110~~ | ~~-141.27~~ | ~~-235.45~~ | ~~-282.55~~ | ~~-329.64~~ | ~~-376.73~~ | ~~-423.82~~ | ~~-470.91~~ |
| ~~120~~ | ~~-174.97~~ | ~~-291.62~~ | ~~-349.94~~ | ~~-408.26~~ | ~~-466.59~~ | ~~-524.91~~ | ~~-583.23~~ |
| ~~130~~ | ~~-211.60~~ | ~~-352.66~~ | ~~-423.19~~ | ~~-493.72~~ | ~~-564.26~~ | ~~-634.79~~ | ~~-705.32~~ |
| ~~140~~ | ~~-251.15~~ | ~~-418.59~~ | ~~-502.31~~ | ~~-586.02~~ | ~~-669.74~~ | ~~-753.46~~ | ~~-837.18~~ |
| ~~150~~ | ~~-293.64~~ | ~~-489.40~~ | ~~-587.28~~ | ~~-685.16~~ | ~~-783.04~~ | ~~-880.92~~ | ~~-978.80~~ |
| ~~170~~ | ~~-387.40~~ | ~~-645.67~~ | ~~-774.81~~ | ~~-903.94~~ | ~~-1033.08~~ | ~~-1162.21~~ | ~~-1291.35~~ |
| ~~Greater than 6 feet from building corner~~ | ~~85~~ | ~~-39.10~~ | ~~-65.17~~ | ~~-78.20~~ | ~~-91.24~~ | ~~-104.27~~ | ~~-117.30~~ | ~~-130.34~~ |
| ~~90~~ | ~~-48.20~~ | ~~-80.33~~ | ~~-96.39~~ | ~~-112.46~~ | ~~-128.52~~ | ~~-144.59~~ | ~~-160.66~~ |
| ~~100~~ | ~~-67.95~~ | ~~-113.24~~ | ~~-135.89~~ | ~~-158.54~~ | ~~-181.19~~ | ~~-203.84~~ | ~~-226.49~~ |
| ~~110~~ | ~~-89.78~~ | ~~-149.63~~ | ~~-179.55~~ | ~~-209.48~~ | ~~-239.40~~ | ~~-269.33~~ | ~~-299.25~~ |
| ~~120~~ | ~~-113.68~~ | ~~-189.47~~ | ~~-227.37~~ | ~~-265.26~~ | ~~-303.16~~ | ~~-341.05~~ | ~~-378.94~~ |
| ~~130~~ | ~~-139.67~~ | ~~-232.78~~ | ~~-279.34~~ | ~~-325.90~~ | ~~-372.45~~ | ~~-419.01~~ | ~~-465.57~~ |
| ~~140~~ | ~~-167.74~~ | ~~-279.56~~ | ~~-335.47~~ | ~~-391.38~~ | ~~-447.29~~ | ~~-503.21~~ | ~~-559.12~~ |
| ~~150~~ | ~~-197.88~~ | ~~-329.80~~ | ~~-395.76~~ | ~~-461.72~~ | ~~-527.68~~ | ~~-593.64~~ | ~~-659.60~~ |
| ~~170~~ | ~~-264.41~~ | ~~-440.68~~ | ~~-528.81~~ | ~~-616.95~~ | ~~-705.08~~ | ~~-793.22~~ | ~~-881.35~~ |

 For SI: 1 foot=304.8mm; 1 pound per linear foot=1.488 kg/m; 1 mile per hour=0.305 m/s

1. The uplift loads are pounds per lineal foot of building length. For roof uplift connections multiply by 1.33 for framing spaced 16 inches on center and multiply by 2 for framing spaced 24 inches on center.
2. The uplift loads do not account for the effects of overhangs. The magnitude of the above loads shall be increased by adding the overhang loads found in the table. The overhang loads are also based on framing spaced 12 inches on center. The overhang loads given shall be multiplied by the overhang projection and added to the roof uplift value in the table.
3. For Ultimate design wind speeds, Vult, greater than 170 mph, wind uplift forces shall be determined in accordance with Section R802.3 or ASCE 7.
4. Ultimate Design Wind Speeds determined from Figure R301.2(4).

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**R907.8.1.1 Access for Retrofitting Roof to Wall Connections.** These provisions are not intended to limit the means for gaining access to the structural elements of the roof and wall for the purposes of retrofitting the connection. The retrofit of roof to wall connections can be made by access through the area under the eave, from above through the roof, or from the interior of the house. Methods for above access include removal of roof panels or sections thereof or removal of portions of roof paneling at selected locations large enough for access, viewing, and installing the retrofit connectors and fasteners.   
  
Where panels or sections are removed, the removed portions shall not be reused. New paneling shall be used and fastened as in new construction.   
  
Holes shall be deemed adequately repaired if a patch of paneling is installed with no gap greater than 1/2 inch (13 mm) between the patch and the existing sheathing and if the patch is supported using one of the following methods.

a) Solid 11/2 inch lumber shall fully support the patch and shall be secured to the existing sheathing with #8 by 11/4 inch screws spaced a minimum of 3 inches (76 mm) around the perimeter with screws a minimum of 3/4 inch from the near edge of the hole. The patch shall be secured to the lumber with #8 × 1-1/4 inch screws spaced on a grid no greater than 6 inches by 6 inches (152 mm × 152 mm) with no fewer than 2 screws.

b) Holes that extend horizontally from roof framing member to adjacent roofing framing member that are less than or equal to 7 inches (178 mm) wide along the slope of the roof shall be supported by minimum of 2 × 4 lumber whose face is attached to each roofing framing members using a minimum of 2 each 3-inch (76 mm) long fasteners (#8 screws or 10d common nails) connecting the two. The patch shall have attached to its bottom, running horizontally, a minimum 2 × 4 either flat wise or on edge secured with #8 × 11/4 inch screws a maximum of 4 inches (102 mm) on center and no more distant from the end of the added lumber than 3 inches (76 mm). The patch shall be secured with two #8 × 1-11/4 inch screws to each support member.

**R907.8.1.2 Partially inaccessible straps.** Where part of a strap is inaccessible, if the portion of the strap that is observed is fastened in compliance with these requirements, the inaccessible portion of the strap shall be presumed to comply with these requirements.

**R907.8.1.3 Prescriptive method for gable roofs on a wood frame wall.** The anchorage of each of the exposed rafters or truss within 6 feet (1829 mm) of the corner along the exterior wall on each side of each gable end shall be inspected. Wherever a strap is missing or an existing strap has fewer than four fasteners on each end, approved straps, ties or right angle brackets with a minimum uplift capacity of 500 lbs (740 kg) shall be installed that connect each rafter or truss to the top plate below. Adding fasteners to existing straps shall be allowed in lieu of adding a new strap provided the strap is manufactured to accommodate at least 4 fasteners at each end. Wherever access makes it possible (without damage of the wall or soffit finishes), both top plate members shall be connected to the stud below using a stud to plate connector with a minimum uplift capacity of 500 lbs (740 kg). Use of straps that connect directly from the rafter or truss to the wall stud below shall be allowed as an alternate provided the two members align with no more than 11/2 inches (38 mm) offset.

**R907.8.1.4 Prescriptive method for gable roofs on a masonry wall.** The anchorage of each of the exposed rafters or truss within 6 feet (1829 mm) of the corner along the exterior wall on each side of each gable end shall be inspected. Wherever a strap is missing or an existing strap has fewer than four fasteners on each end, approved straps, ties or right angle gusset brackets with a minimum uplift capacity of 500 lbs (740 kg) shall be installed that connect each rafter or truss to the top plate below or directly to the masonry wall using approved masonry screws of a length and diameter recommended by the manufacturer. In the absence of manufacturer’s recommendations, screws shall provide at least a 21/2 inch (64 mm) embedment into the concrete or masonry. When the straps or right angle gusset brackets are attached to a wood sill plate, the sill plate shall be anchored to the concrete masonry wall below. This anchorage shall be accomplished by installing 1/4-inch diameter masonry screws, each with supplementary 1/4-inch washer, having sufficient length to develop a 21/2 inch (64 mm) embedment into the concrete and masonry. These screws shall be installed within 4 inches (102 mm) of the truss or rafter on both sides of each interior rafter or truss and on the accessible wall side of the gable end truss or rafter.

**R907.8.1.5 Prescriptive method for hip roofs on a wood frame wall.** Unless it is possible to verify through non-destructive inspection or from plans prepared by a design professional that the roof structure is anchored at least as well as outlined below, access shall be provided at a minimum to the hip rafter (commonly known as a "king jack”), to the hip girder and at each corner of the hip roof. The hip rafter (commonly known as a "king jack”), the hip girder and the rafters/trusses adjacent to the hip girder that are not anchored with a strap having at least four fasteners on each end, shall be connected to the top plate below using a strap or a right angle gusset bracket having a minimum uplift capacity of 500 lbs (740 kg). Adding fasteners to existing straps shall be allowed in lieu of adding a new strap provided the strap is manufactured to accommodate at least 4 fasteners at each end. Wherever access makes it possible (without damage of the wall or soffit finishes), both top plate members shall be connected to the stud below using a stud to plate connector with a minimum uplift capacity of 500 lbs (740 kg). Use of straps that connect directly from the hip rafter, hip girder or adjacent rafters/trusses to the wall stud below shall be allowed as an alternate provided the two members align with no more than 11/2 inch (38 mm) offset.

**R907.8.1.6 Prescriptive method for hip roofs on a masonry wall.** Unless it is possible to verify through non-destructive inspection or from plans prepared by a design professional that the roof structure is anchored at least as well as outlined below, access shall be provided at a minimum to the hip rafter (commonly known as a "king jack”), to the hip girder and at each corner of the hip roof. The hip rafter (commonly known as a "king jack”), the hip girder and the rafters/trusses adjacent to the hip girder that are not anchored with a strap having at least four fasteners on each end, shall be connected to the concrete masonry wall below using approved straps or right angle gusset brackets with a minimum uplift capacity of 500 lbs (740 kg). Adding fasteners to existing straps shall be allowed in lieu of adding a new strap provided the strap is manufactured to accommodate at least 4 fasteners at each end. The straps or right angle gusset brackets shall be installed such that they connect each rafter or truss to the top plate below or directly to the masonry wall using approved masonry screws of a length and diameter recommended by the manufacturer. In the absence of manufacturer’s recommendations, screws shall provide at least 21/2 inches (64 mm) embedment into the concrete or masonry. When the straps or right angle gusset brackets are attached to a wood sill plate, the sill plate shall be anchored to the concrete masonry wall below. This anchorage shall be accomplished by installing 1/4-inch (6 mm) diameter masonry screws, each with supplementary 1/4-inch (6 mm) washer, with sufficient length to develop a 21/2 inch (64 mm) embedment into the concrete and masonry. These screws shall be installed within 4 inches (102 mm) of the truss or rafter on both sides of each interior rafter or truss and on the accessible wall side of the gable end truss or rafter.

**R907.8.1.7 Priorities for mandated roof-to-wall retrofit expenditures.** Priority shall be given to connecting the exterior corners of roofs to walls where the spans of the roofing members are greatest. For houses with both hip and gable roof ends, the priority shall be to retrofit the gable end roof-to-wall connections unless the width of the hip end is more than 1.5 times greater than the width of the gable end. When considering priorities for houses with both hip and gable roof ends, and the fifteen percent of the cost of roof replacement is sufficient to complete all of the prioritized elements pursuant to this section, but is not sufficient to complete all of the non-prioritized elements, then no portion of complete retrofit of the non-prioritized element is required.

**(R5554 AM R2)**

**CHAPTER 10. CHIMNEYS AND FIREPLACES.**

***Section R1001.12 Fireplace fireblocking. Change to read as shown:***

**R1001.12 Fireplace fireblocking.** Fireplace fireblocking shall comply with the provisions of Section R602.2.1~~R602.8~~.

**(S6171GlitchAS)**

**CHAPTER 11 ENERGY EFFICIENCY**

***Section N101 Energy efficiency. Revise to read as shown:***

**N1101 Energy efficiency.** The provisions of the *Florida Building Code, Energy Conservation,* shall govern the energy efficiency of residential construction.

**[The remaining text of this Chapter is deleted and reserved]**

**CHAPTER 12 MECHANICAL ADMINISTRATION. [No change]**

**CHAPTER 13 GENERAL MECHANICAL SYSTEM REQUIREMENTS**

***Section M1308.1 Drilling and notching. Change to read as follows:***

**M1308.1 Drilling and notching.** Wood-framed structural members shall be drilled, notched or altered in accordance with the provisions of Sections R502.1.11, R602.2.3, R602.2.3.1, and R802.1.8 ~~R502.8, R602.6, R602.6.1 and R802.7~~. Holes in load-bearing members of cold-formed steel light-frame construction shall be permitted only in accordance with AISI S230 ~~Sections R505.2.5, R603.2.5 and R804.2.5~~. In accordance with the provisions of AISI S230 ~~Sections R505.3.5, R603.3.4 and R804.3.4~~, cutting and notching of flanges and lips of load-bearing members of cold-formed steel light frame construction shall not be permitted. Structural insulated panels (SIPs) shall be drilled and notched or altered in accordance with the provisions of Section R613.7.

**(S6172GlitchAS)**

**CHAPTER 14, HEATING AND COOLING EQUIPMENT AND APPLIANCES**

***Section 1411.5 Insulation of refrigerant piping. Change to read as follows:***

**1411.5 Insulation of refrigerant piping.** Piping and fittings for refrigerant vapor (suction) lines shall be insulated with insulation having a thermal resistivity of at least R-3 ~~R-4~~ and having external surface permeance not exceeding 0.05 perm [2.87 ng/(s . m2 . Pa)] when tested in accordance with ASTM E 96.

**(M5756 AS)**

**CHAPTER 15, EXHAUST SYSTEMS. [No change]**

**CHAPTER 16, DUCT SYSTEMS**

***Section M1601.1.1 Above-ground duct systems. Change to read as shown:***

**M1601.1.1 Above-ground duct systems.** Above-ground duct systems shall conform to the following:

1. – 6. [No change]
2. Stud wall cavities and the spaces between solid floor joists to be used as air plenums shall comply with the following conditions:

7.1 – 7.3 [No change]

7.4 Stud wall cavities and joist-space plenums shall be isolated from adjacent concealed spaces by tight-fitting fireblocking in accordance with Section R602.2.1~~R602.8~~.

7.5 [No change]

**(S6174GlitchAS)**

***Section M1601.4.4 Fireblocking. Change to read as shown:***

**M1601.4.4 Fireblocking.** Duct installations shall be fireblocked in accordance with Section R602.2.1~~R602.8~~.

**(S6174GlitchAS)**

***Section M1602.4 Add a section to read as follows:***

**M1602.4 Balanced Return Air.** Restricted return air occurs in buildings when returns are located in central zones and closed interior doors impede air flow to the return grill or when ceiling spaces are used as return plenums and fire walls restrict air movement from one portion of the return plenum to another. Provisions shall be made in both residential and commercial buildings to avoid unbalanced air flows and pressure differentials caused by restricted return air. Pressure differentials across closed doors where returns are centrally located shall be limited to 0.01 inch WC (2.5 pascals) or less. Pressure differentials across fire walls in ceiling space plenums shall be limited to 0.01 inch WC (2.5 pascals) by providing air duct pathways or air transfer pathways from the high pressure zone to the low zone.

Exceptions:

1. Transfer ducts may achieve this by increasing the return transfer 11/2 times the cross sectional area (square inches) of the supply duct entering the room or space it is serving and the door having at least an unrestricted 1 inch undercut to achieve proper return air balance.

2. Transfer grilles shall use 50 square inches (of grille area) to 100 cfm (of supply air) for sizing through-the-wall transfer grilles and using an unrestricted 1 inch undercutting of doors to achieve proper return air balance.

3. Habitable rooms only shall be required to meet these requirements for proper balanced return air excluding bathrooms, closets, storage rooms and laundry rooms, except that all supply air into the master suite shall be included.

**(M5618 AS) (M5574 AM)**

**CHAPTER 17, COMBUSTION AIR [No change]**

**CHAPTER 18, CHIMNEYS AND VENTS**

***Section M1801.9 Fireblocking. Change to read as shown:***

**M1801.9 Fireblocking.** Vent and chimney installations shall be fireblocked in accordance with Section R602.2.1~~R602.8~~.

**(S6175GlitchAS)**

**CHAPTER 19, SPECIAL APPLIANCES, EQUIPMENT AND SYSTEMS [No change]**

**CHAPTE 20, BOILERS AND WATER HEATERS [No change]**

**CHAPTER 21, HYDRONIC PIPING**

***Section M2101.6 Drilling and notching. Change to read as shown:***

**M2101.6 Drilling and notching.** Wood-framed structural members shall be drilled, notched or altered in accordance with the provisions of Sections R502.1.11, R602.2.3, R602.2.3.1, and R802.1.8 ~~R502.8, R602.6, R602.6.1 and R802.7~~. Holes in load bearing members of cold-formed steel light-frame construction shall be permitted only in accordance with AISI S230 ~~Sections R505.2.5, R603.2.5 and R804.2.5~~.In accordance with the provisions of AISI S230 ~~Sections R505.3.5, R603.3.4 and R804.3.4~~, cutting and notching of flanges and lips of load-bearing members of cold-formed steel light frame construction shall not be permitted. Structural insulated panels (SIPs) shall be drilled and notched or altered in accordance with the provisions of Section R613.

**(S6176GlitchAS)**

**CHAPTE 22, SPECIAL PIPING AND STORAGE SYSTEMS [No change]**

**CHAPTER 23, SOLAR ENERGY SYSTEMS**

***Section M2301.2.2 Roof mounted collectors. Revise section to read as shown:***

**M2301.2.2 Roof-mounted collectors.** The roof shall be constructed to support the loads imposed by roof-mounted solar collectors. Roof-mounted solar collectors that serve as a roof covering shall conform to the requirements for roof coverings in Chapter 9 **(**the HVHZ shall comply with Chapter 44) of this code. Where mounted on or above the roof coverings, the collectors and supporting structure shall be constructed of noncombustible materials or fire-retardant-treated wood equivalent to that required for the roof construction.

**(R4980 AM R1)**

***Section M2301.2.7 Roof and wall penetrations. Revise section to read as shown:***

**M2301.2.7 Roof and wall penetrations.** Roof and wall penetrations shall be flashed and sealed in accordance with Chapter 9 (the HVHZ shall comply with Chapter 44) of this code to prevent entry of water, rodents and insects.

**[M4981 AS]**

***Section M2302.2.1 Roof-mounted panels and modules. Revise to read as shown:***

**M2302.2.1 Roof-mounted panels and modules.** Where photovoltaic panels and modules are installed on roofs, the roof shall be constructed to support the loads imposed by such modules. Roof-mounted photovoltaic panels and modules that serve as roof covering shall conform to the requirements for roof coverings in Chapter 9 **(**the HVHZ shall comply with Chapter 44). Where mounted on or above the roof coverings, the photovoltaic panels and modules and supporting structure shall be constructed of noncombustible materials or fire-retardant treated wood equivalent to that required for the roof construction.

**(R4982 AM R1)**

***Section M2302.2.2 Roof and wall penetrations. Revise to read as shown:***

**M2302.2.2 Roof and wall penetrations.** Roof and wall penetrations shall be flashed and sealed in accordance with Chapter 9 (the HVHZ shall comply with Chapter 44) to prevent entry of water, rodents, and insects.

**[M4983 AS]**

**CHAPTER 24, FUEL GAS**

***Section G2412.9 Identification. Revise to read as shown:***

**G2412.9 (401.9) Identification.** Each length of pipe and tubing and each pipe fitting, utilized in a fuel gas system, shall bear the identification of the manufacturer.

**Exception**: The manufacturer identification for fittings and pipe nipples shall be on each piece or shall be printed on the fitting or nipple packaging or provided documentation.

**[P5248 AM R1]**

***Section G2412.10 Third-party testing and certification. Revise to read as shown:***

**G2412.10· (401.10) Third-party testing and certification**. Reserved.  ~~All piping, tubing and fittings shall comply with the applicable referenced standards, specifications and performance criteria of this code and shall be identified in accordance with~~ [~~Section 401.9.~~](javascript:Next('./icod_ifgc_2012_4_sec001_par009.htm');) ~~Piping, tubing and fittings shall either be tested by an approved third-party testing agency or certified by an~~ approved*~~third-party certification agency.~~*

**[P5249 AM R1]**

**CHAPTER 25, PLUMBING ADMINISTRATION**

***Section P2503.8.2 Testing. Change to read as shown:***

**P2503.8.2 Testing.** Reduced pressure principle, double check, double check detector and pressure vacuum breaker backflow preventer assemblies shall be tested at the time of installation~~,~~ and immediately after repairs or relocation ~~and at least annually~~.

**(Post-Glitch Comment P1 AS)**

**CHAPTER 26, GENERAL PLUMBING REQUIREMENTS**

***Section P2603.2Drilling and notching. Change to read as shown:***

**P2603.2 Drilling and notching.** Wood-framed structural members shall not be drilled, notched or altered in any manner except as provided in Sections R502.1.11, R602.2.3, R602.2.3.1, and R802.1.8 ~~R502.8, R602.6, R802.7 and R802.7.1~~. Holes in load-bearing members of cold-formed steel light-frame construction shall be permitted only in accordance with AISI S230 ~~Sections R505.2.5, R603.2.5 and R804.2.5~~.In accordance with the provisions in AISI S230 ~~Sections R505.3.5, R603.3.4 and R804.3.4~~, cutting and notching of flanges and lips of load-bearing members of cold-formed steel light frame construction shall not be permitted. Structural insulated panels (SIPs) shall be drilled and notched or altered in accordance with the provisions of Section R613.7.

**(P6177GlitchAM)**

***Section P2603.3 Breakage and corrosion. Revise to read as shown:***

**P2603.3 Breakage and corrosion.** Pipes passing through concrete or cinder walls and floors, cold-formed steel framing or other corrosive material shall be protected against external corrosion by a protective sheathing or wrapping or other means that will withstand any reaction from lime and acid of concrete, cinder or other corrosive material. Sheathing or wrapping shall allow for movement including expansion and contraction of piping. Minimum wall thickness of material shall be 0.010 inch (0.254 mm).

**Exception:** Sleeving is not required for installation of CPVC into concrete or similar material

***P2603.3.1 Penetration. Add text to read as shown:***

**P2603.3.1 Penetration.** Protective sleeves around piping penetrating concrete slab-on-grade floors shall not be of cellulose-containing materials. If soil treatment is used for subterranean termite protection, the sleeve shall have a maximum wall thickness of 0.010 inch, and be sealed within the slab using a non-corrosive clamping device to eliminate the annular space between the pipe and the sleeve. No termiticides shall be applied inside the sleeve.

**CHAPTER 27, PLUMBING FIXTURES. [No change]**

**CHAPTER 28, WATER HEATERS [No change]**

**CHAPTER 29, WATER SUPPLY AND DISTRIBUTION**

**Section P2905.2.1 Lead content of drinking water pipe and fittings. Add to read as shown:**

**P2905.2.1 Lead content of drinking water pipe and fittings.** Pipe, pipe fittings, joints, valves, faucets, and fixture fittings utilized to supply water for drinking or cooking purposes shall comply with NSF 372 and shall have a weighted average lead content of 0.25 percent lead or less.

**(P6059Glitch AM)**

**CHAPTER 30, SANITARY DRAINAGE**

***Section P3003.19 Joints between drainage piping and water closets. Change to read as shown:***

**P3003.19 Joints between drainage piping and water closets.** Joints between drainage piping and water closets or similar fixtures shall be made by means of a closet flange or a waste connector and sealing gasket compatible with the drainage system material, securely fastened to a structurally firm base. ~~The inside diameter of the drainage pipe shall not be used as a socket fitting for a 4-inch by 3-inch (102 mm by 76 mm) closet flange.~~ The joint shall be bolted, with an approved gasket flange to fixture connection complying with ASME A112.4.3 or setting compound between the fixture and the closet flange or waste connector and sealing gasket. The waste connector and sealing gasket joint shall comply with the joint-tightness test of ASME A112.4.3 and shall be installed in accordance with the manufacturer’s installation instructions.

**(P6056GlitchAS)**

***Section P3009.1 Scope. Revise to read as shown:***

**P3009.1 Scope.** The provisions of shall govern the materials, design, construction and installation of gray water systems for flushing of water closets and urinals ~~and for subsurface landscape irrigation~~. See Figures P3009.1(1) ~~and P3009.1(2)~~.

|  |
| --- |
|  |
|  |

**FIGURE P3009.1(1)**

**GRAY WATER RECYCLING SYSTEM FOR FLUSHING WATER CLOSETS AND URINALS**

[No change to figure]

***Figure P3009.1(2). Delete table and reserve as shown:***

**FIGURE P3009.1(2)**

**GRAY WATER RECYCLING SYSTEM FOR SUBSURFACE LANDSCAPE IRRIGATION.**

Reserved.

***Section P3009.2 Installation. Revise to read as shown:***

**P3009.2 Installation.** In addition to the provisions of [Section P3009](javascript:Next('./icod_irc_2012_30_sec009.htm');), systems for flushing of water closets and urinals shall comply with [Section P3009.13](javascript:Next('./icod_irc_2012_30_sec009_par013.htm');) ~~and systems for subsurface landscape irrigation shall comply with~~ [~~Section P3009.14.~~](javascript:Next('./icod_irc_2012_30_sec009_par020.htm');) Except as provided for in [Section P3009](javascript:Next('./icod_irc_2012_30_sec009.htm');), all systems shall comply with the provisions of the other sections of this code.

**P3009.3-P3009.13** [Text is unchanged]

***Section P3009.14 Landscape irrigation systems. Revise to read as shown. Delete the remainder of Section P3009.14 and show as "Reserved."***

**P3009.14 Landscape irrigation systems.**Reserved. ~~Subsurface landscape irrigation systems shall comply with~~ [~~Sections P3009.14.1~~](javascript:Next('./icod_irc_2012_30_sec009_par021.htm');) ~~through~~ [~~P3009.14.11~~](javascript:Next('./icod_irc_2012_30_sec009_par043.htm');)

**P3009.14.1 Collection reservoir.**Reserved. ~~Reservoirs shall be sized to limit the retention time of gray water to a maximum of 24 hours.~~

**P3009.14.1.1 Identification.** Reserved.  ~~The reservoir shall be identified as containing nonpotable water.~~

**P3009.14.2 Valves required.**Reserved. ~~A check valve and a full-open valve located on the discharge side of the check valve shall be installed on the effluent pipe of the collection reservoir.~~

**P3009.14.3 Makeup water.**Reserved. ~~Makeup water shall not be required for subsurface landscape irrigation systems. Where makeup water is provided, the installation shall be in accordance with Section 3009.13.3.~~

**P3009.14.4 Disinfection.**Reserved. ~~Disinfection shall not be required for gray water used or subsurface landscape irrigation systems.~~

**P3009.14.5 Coloring.**Reserved. ~~Gray water used for subsurface landscape irrigation systems shall not be required to be dyed.~~

**P3009.14.6 Estimating gray water discharge.**Reserved. ~~The system shall be sized in accordance with the gallons-per-day-per-occupant number based on the type of fixtures connected to the gray water system. The discharge shall be calculated by the following equation:~~

|  |  |
| --- | --- |
| *~~C~~* ~~=~~ *~~A~~* ~~×~~ *~~B~~* | **~~(Equation 30-1)~~** |

~~where:~~

|  |  |  |  |  |  |  |  |
| --- | --- | --- | --- | --- | --- | --- | --- |
| *~~A~~* | | | ~~=~~ | ~~Number of occupants:~~ | | | |
| ~~Number of occupants shall be determined by the actual number of occupants, but not less than two occupants for one bedroom and one occupant for each additional bedroom.~~ | | | | | | | |
| ~~B~~ | | ~~=~~ | ~~Estimated flow demands for each occupant:~~ | |
| ~~Residential—25 gallons per day (94.6 lpd) per occupant for showers, bathtubs and lavatories and 15 gallons per day (56.7 lpd) per occupant for clothes washers or laundry trays.~~ | | | | | |
| *~~C~~* | ~~=~~ | ~~Estimated gray water discharge based on the total number of occupants.~~ | | | | |
|  |  |  | | | | |

**P3009.14.7 Percolation tests.**Reserved. ~~The permeability of the soil in the proposed absorption system shall be determined by percolation tests or permeability evaluation.~~

**P3009.14.7.1 Percolation tests and procedures.** Reserved. ~~At least three percolation tests in each system area shall be conducted. The holes shall be spaced uniformly in relation to the bottom depth of the proposed absorption system. More percolation tests shall be made where necessary, depending on system design.~~

**P3009.14.7.1.1 Percolation test hole.** Reserved. ~~The test hole shall be dug or bored. The test hole shall have vertical sides and a horizontal dimension of 4 inches to 8 inches (102 mm to 203 mm). The bottom and sides of the hole shall be scratched with a sharp-pointed instrument to expose the natural soil. All loose material shall be removed from the hole and the bottom shall be covered with 2 inches (51 mm) of gravel or coarse sand.~~

**P3009.14.7.1.2 Test procedure, sandy soils.** Reserved. ~~The hole shall be filled with clear water to a minimum of 12 inches (305 mm) above the bottom of the hole for tests in sandy soils. The time for this amount of water to seep away shall be determined, and this procedure shall be repeated if the water from the second filling of the hole seeps away in 10 minutes or less. The test shall proceed as follows: Water shall be added to a point not more than 6 inches (152 mm) above the gravel or coarse sand. Thereupon, from a fixed reference point, water levels shall be measured at 10-minute intervals for a period of 1 hour. Where 6 inches (152 mm) of water seeps away in less than 10 minutes, a shorter interval between measurements shall be used, but in no case shall the water depth exceed 6 inches (152 mm). Where 6 inches (152 mm) of water seeps away in less than 2 minutes, the test shall be stopped and a rate of less than 3 minutes per inch (7.2 s/mm) shall be reported. The final water level drop shall be used to calculate the percolation rate. Soils not meeting the above requirements shall be tested in accordance with Section 3009.14.7.1.3.~~

**P3009.14.7.1.3 Test procedure, other soils.**Reserved. ~~The hole shall be filled with clear water, and a minimum water depth of 12 inches (305 mm) shall be maintained above the bottom of the hole for a 4-hour period by refilling whenever necessary or by use of an automatic siphon. Water remaining in the hole after 4 hours shall not be removed. Thereafter, the soil shall be allowed to swell not less than 16 hours or more than 30 hours. Immediately after the soil swelling period, the measurements for determining the percolation rate shall be made as follows: Any soil sloughed into the hole shall be removed and the water level shall be adjusted to 6 inches (152 mm) above the gravel or coarse sand. Thereupon, from a fixed reference point, the water level shall be measured at 30-minute intervals for a period of 4 hours, unless two successive water level drops do not vary by more than~~ ~~1~~~~/~~~~16~~ ~~inch (1.59 mm). At least three water level drops shall be observed and recorded. The hole shall be filled with clear water to a point not more than 6 inches (152 mm) above the gravel or coarse sand whenever it becomes nearly empty. Adjustments of the water level shall not be made during the three measurement periods except to the limits of the last measured water level drop. When the first 6 inches (152 mm) of water seeps away in less than 30 minutes, the time interval between measurements shall be 10 minutes and the test run for 1 hour. The water depth shall not exceed 5 inches (127 mm) at any time during the measurement period. The drop that occurs during the final measurement period shall be used in calculating the percolation rate.~~

**P3009.14.7.1.4 Mechanical test equipment.**Reserved. ~~Mechanical percolation test equipment shall be of an approved type.~~

**P3009.14.7.2 Permeability evaluation.**Reserved. ~~Soil shall be evaluated for estimated percolation based on structure and texture in accordance with accepted soil evaluation practices. Borings shall be made in accordance with~~ [~~Section P3009.14.7.1~~](javascript:Next('./icod_irc_2012_30_sec009_par029.htm');) ~~for evaluating the soil.~~

**P3009.14.8 Subsurface landscape irrigation site location.** Reserved. ~~The surface grade of all soil absorption systems shall be located at a point lower than the surface grade of any water well or reservoir on the same or adjoining lot. Where this is not possible, the site shall be located so that surface water drainage from the site is not directed toward a well or reservoir. The soil absorption system shall be located with a minimum horizontal distance between various elements as indicated in Table P3009.14.8. Private sewage disposal systems in compacted areas, such as parking lots and driveways, are prohibited. Surface water shall be diverted away from any soil absorption site on the same or neighboring lots.~~

**TABLE P3009.14.8**

**LOCATION OF GRAY WATER SYSTEM**

Reserved.

|  |  |  |
| --- | --- | --- |
| **~~ELEMENT~~** | **~~MINIMUM HORIZONTAL DISTANCE~~** | |
| **~~HOLDING TANK  (feet)~~** | **~~IRRIGATION  DISPOSAL FIELD  (feet)~~** |
| ~~Buildings~~ | ~~5~~ | ~~2~~ |
| ~~Property line adjoining private property~~ | ~~5~~ | ~~5~~ |
| ~~Public water main~~ | ~~10~~ | ~~10~~ |
| ~~Seepage pits~~ | ~~5~~ | ~~5~~ |
| ~~Septic tanks~~ | ~~0~~ | ~~5~~ |
| ~~Streams and lakes~~ | ~~50~~ | ~~50~~ |
| ~~Water service~~ | ~~5~~ | ~~5~~ |
| ~~Water wells~~ | ~~50~~ | ~~100~~ |
| ~~For SI: 1 foot = 304.8 mm.~~ |

**P3009.14.9 Installation.**Reserved. ~~Absorption systems shall be installed in accordance with~~ [~~Sections P3009.14.9.1~~](javascript:Next('./icod_irc_2012_30_sec009_par037.htm');) ~~through~~ [~~P3009.14.9.5~~](javascript:Next('./icod_irc_2012_30_sec009_par041.htm');) ~~to provide landscape irrigation without surfacing of gray water.~~

**P3009.14.9.1 Absorption area.**Reserved. ~~The total absorption area required shall be computed from the estimated daily gray water discharge and the design-loading rate based on the percolation rate for the site. The required absorption area equals the estimated gray water discharge divided by the design-loading rate from Table P3009.14.9.1.~~

***Table P3009.14.9.1 Design Loading Rate. Delete and reserve as shown:***

**TABLE P3009.14.9.1**

**DESIGN LOADING RATE**

**Reserved**

|  |  |
| --- | --- |
| **~~PERCOLATION RATE  (minutes per inch)~~** | **~~DESIGN LOADING FACTOR  (gallons per square foot per day)~~** |
| ~~0 to less than 10~~ | ~~1.2~~ |
| ~~10 to less than 30~~ | ~~0.8~~ |
| ~~30 to less than 45~~ | ~~0.72~~ |
| ~~45 to 60~~ | ~~0.4~~ |
| ~~For SI: 1 minute per inch = min/25.4 mm,~~ |
| ~~1 gallon per square foot = 40.7 L/m~~~~2~~~~.~~ |

**P3009.14.9.2 Seepage trench excavations.** Reserved. ~~Seepage trench excavations shall be a minimum of 1 foot (304 mm) to a maximum of 5 feet (1524 mm) wide. Trench excavations shall be spaced a minimum of 2 feet (610 mm) apart. The soil absorption area of a seepage trench shall be computed by using the bottom of the trench area (width) multiplied by the length of pipe. Individual seepage trenches shall be a maximum of 100 feet (30 480 mm) in developed length.~~

**P3009.14.9.3 Seepage bed excavations.**Reserved. ~~Seepage bed excavations shall be a minimum of 5 feet (1524 mm) wide and have more than one distribution pipe. The absorption area of a seepage bed shall be computed by using the bottom of the trench area. Distribution piping in a seepage bed shall be uniformly spaced a maximum of 5 feet (1524 mm) and a minimum of 3 feet (914 mm) apart, and a maximum of 3 feet (914 mm) and a minimum of 1 foot (305 mm) from the sidewall or headwall.~~

**P3009.14.9.4 Excavation and construction.**Reserved. ~~The bottom of a trench or bed excavation shall be level. Seepage trenches or beds shall not be excavated where the soil is so wet that such material rolled between the hands forms a soil wire. All smeared or compacted soil surfaces in the sidewalls or bottom of seepage trench or bed excavations shall be scarified to the depth of smearing or compaction and the loose material removed. Where rain falls on an open excavation, the soil shall be left until sufficiently dry so a soil wire will not form when soil from the excavation bottom is rolled between the hands. The bottom area shall then be scarified and loose material removed.~~

**P3009.14.9.5 Aggregate and backfill.**Reserved. ~~A minimum of 6 inches (152 mm) of aggregate ranging in size from~~ ~~1~~~~/~~~~2~~ ~~inch to 2~~~~1~~~~/~~~~2~~ ~~inches (12.7 mm to 64 mm) shall be laid into the trench below the distribution piping elevation. The aggregate shall be evenly distributed a minimum of 2 inches (51 mm) over the top of the distribution pipe. The aggregate shall be covered with approved synthetic materials or 9 inches (229 mm) of uncompacted marsh hay or straw. Building paper shall not be used to cover the aggregate. A minimum of 9 inches (229 mm) of soil backfill shall be provided above the covering.~~

**P3009.14.10 Distribution piping.**Reserved. ~~Distribution piping shall be not less than 3 inches (76 mm) in diameter. Materials shall comply with Table P3009.14.10. The top of the distribution pipe shall be not less than 8 inches (203 mm) below the original surface. The slope of the distribution pipes shall be a minimum of 2 inches (51 mm) and a maximum of 4 inches (102 mm) per 100 feet (30 480 mm).~~

**TABLE P3009.14.10**

**DISTRIBUTION PIPE**

Reserved.

|  |  |
| --- | --- |
| **~~MATERIAL~~** | **~~STANDARD~~** |
| ~~Polyethylene (PE) plastic pipe~~ | ~~ASTM F 405~~ |
| ~~Polyvinyl chloride (PVC) plastic pipe~~ | ~~ASTM D 2729~~ |
| ~~Polyvinyl chloride (PVC) plastic pipe with a 3.5-inch O.D. and solid cellular core or composite wall~~ | ~~ASTM F 1488~~ |

**P3009.14.11 Joints.**Reserved. ~~Joints in distribution pipe shall be made in accordance with~~ [~~Section P3003~~](javascript:Next('./icod_irc_2012_30_sec003.htm');)~~.~~

**[SP5964 AS]**

**CHAPTER 31, VENTS [No change]**

**CHAPTER 32, TRAPS [No change]**

**CHAPTER 33, STORAGE DRAINAGE [No change]**

**CHAPTER 34, GENERAL REQUIREMENTS [No change]**

**CHAPTER 35, ELECTRICAL DEFINITIONS [No change]**

**CHAPTER 36, SERVICES [No change]**

**CHAPTER 37, BRANCH CIRCUIT AND FEEDER REQUIREMENTS [No change]**

**CHAPTER 38, WIRING METHODS [No change]**

**CHAPTER 39, POWER AND LIGHTING DISTRIBUTION [No change]**

**CHAPTER 40, DEVICES AND LUMINAIRES [No change]**

**CHAPTER 41, APPLIANCE INSTALLATION [No change]**

***Change the title of Chapter 42 to read as shown:***

**CHAPTER 42, SWIMMING POOLS, ELECTRICAL**

**CHAPTER 43 CLASS 2 REMOTE-CONTROL, SIGNALING AND POWER-LIMITED CIRCUITS [No change]**

**CHAPTER 44 HIGH-VELOCITY HURRICANE ZONES ~~REFERENCED STANDARDS~~**

***Section R4401. Change to read as follows:***

**SECTION R4401**

**HIGH-VELOCITY HURRICANE ZONES —**

**EXTERIOR WALL COVERING**

**R4401.1** Refer to Chapter 14 of the *Florida Building Code, Building.*

**[Mod R5008AS]**

***Section R4402. Change to read as follows:***

**SECTION R4402**

**HIGH-VELOCITY HURRICANE ZONES —**

**ROOF ASSEMBLIES AND ROOFTOP STRUCTURES**

**R4402.1.** Refer to Chapter 15 of the *Florida Building Code, Building*.

|  |  |  |
| --- | --- | --- |
|  | **~~[U~~** |  |

**[Mod 5009 AS]**

***Section R4403. Change to read as follows:***

**SECTION R4403**

**HIGH-VELOCITY HURRICANE ZONES—**

**GENERAL**

**R4403.1** Refer to Chapter 16 of the *Florida Building Code, Building.*

**[5850 AM R1]**

**(SP5290 AS)**

**[S5010-R1 AM]**

|  |
| --- |
|  |

***Section R4404. Change to read as follows:***

**SECTION R4404**

**HIGH-VELOCITY HURRICANE ZONES —**

**FOUNDATIONS AND RETAINING WALLS**

**R4404.1** Refer to Chapter 18 of the *Florida Building Code, Building*.

**[S5011 AS]**

***Section R4405. Change to read as follows:***

**SECTION R4405**

**HIGH-VELOCITY HURRICANE ZONES —**

**CONCRETE**

**R4405.1** Refer to Chapter 19 of the *Florida Building Code, Building***.**

**[Mod S5012 AS]**

***Section R4406. Change to read as follows:***

|  |
| --- |
| **SECTION 4406**  **HIGH-VELOCITY HURRICANE ZONES —**  **LIGHT METAL ALLOYS**    **R4406.1** Aluminum. Refer to Chapter 20 of the *Florida Building Code, Building*. |
|  |
| **[S5013 AS]**   |  | | --- | | ***Section R4407. Change to read as follows:***  **SECTION 4407**  **HIGH-VELOCITY HURRICANE ZONES — MASONRY**  **R4407.1** Refer to Chapter 21 of the *Florida Building Code, Building*.  **(S5014AS)** |      |  | | --- | | ***Section R4408. Change to read as follows:***  **SECTION 4408**  **HIGH-VELOCITY HURRICANE ZONES — STEEL**  **R4408.1** Refer to Chapter 22 of the *Florida Building Code, Building*.  **(S5015AS)** |  |  | | --- | | ***Section R4409. Change to read as follows:***  **SECTION 4409**  **HIGH-VELOCITY HURRICANE ZONES — WOOD**  **R4409.1** Refer to Chapter 23 of the *Florida Building Code, Building*.  **(S5016AS)** |   ***Section R4410. Change to read as follows:***   |  | | --- | | **SECTION R4410**  **HIGH-VELOCITY HURRICANE ZONES —**  **GLASS AND GLAZING**    **R4410.1** Refer to Chapter 24 of the *Florida Building Code, Building*.    **[S5017 AS]** | | ***Section R4411. Change to read as follows:***  **SECTION R4411**  **HIGH-VELOCITY HURRICANE ZONES —**  **GYPSUM BOARD AND PLASTER**  **R4411.1** Refer to Chapter 25 of the *Florida Building Code, Building.*  **[S5018 AS]** |      |  | | --- | | ***Section R4412. Change to read as follows:***  **SECTION R4412**  **HIGH-VELOCITY HURRICANE ZONES —**  **PLASTICS**    **R4412.1** Refer to Chapter 26 of the *Florida Building Code, Building*.    **[F5019 AS]** |   ***Add Chapter 45 to read as follows:***    **CHAPTER 45 PRIVATE SWIMMING POOLS**  **R4501 Private Swimming Pools**  **R4501.1 Definitions, general.**    **R4501.1.1 Tense, gender and number.** For the purpose of this code, certain abbreviations, terms, phrases, words, and their derivatives shall be construed as set forth in this section. Words used in the present tense include the future. Words in the masculine gender include the feminine and neuter. Words in the feminine and neuter gender include the masculine. The singular number includes the plural and the plural number includes the singular.    **R4501.1.2 Words not defined.** Words not defined herein shall have the meanings stated in the *Florida Building Code, Building; Florida Building Code, Mechanical; Florida Building Code, Plumbing; Florida Building Code, Fuel Gas*; or *Florida Fire Prevention Code*. Words not defined in the *Florida Building Code* shall have the meanings stated in the W*ebster's Third New International Dictionary of the English Language Unabridged*.    **R4501.2 Definitions.**    **ABOVEGROUND/ONGROUND POOL.** See "Swimming Pool."    **ADMINISTRATIVE AUTHORITY.** The individual official, board, department or agency established and authorized by a state, county, city or other political subdivision created by law to administer and enforce the provisions of the swimming pool code as adopted or amended.    **APPROVED.** Accepted or acceptable under an applicable specification stated or cited in this code, or accepted as suitable for the proposed use under procedures and power of the administrative authority.    **APPROVED SAFETY COVER.** A manually or power-applied safety pool cover that meets all of the performance standards of the ASTM International in compliance with ASTM F 1346.    **APPROVED TESTING AGENCY.** An organization primarily established for the purpose of testing to approved standards and approved by the administrative authority.    **BACKWASH PIPING.** See "Filter waste discharge piping."    **BARRIER.** A fence, dwelling wall or nondwelling wall or any combination thereof which completely surrounds the swimming pool and obstructs access to the swimming pool, especially access from the residence or from the yard outside the barrier.    **BODY FEED.** Filter aid fed into a diatomite-type filter throughout the filtering cycle.    **CARTRIDGE FILTER.** A filter using cartridge type filter elements.    **CHEMICAL PIPING.** Piping which conveys concentrated chemical solutions from a feeding apparatus to the circulation piping.    **CIRCULATION PIPING SYSTEM.** Piping between the pool structure and the mechanical equipment. Usually includes suction piping, face piping and return piping.    **COMBINATION VALVE.** A multipart valve intended to perform more than one function.    **DESIGN HEAD.** Total head requirement of the circulation system at the design rate of flow.    **DIATOMITE (DIATOMACEOUS EARTH).** A type of filter aid.    **DIATOMITE TYPE FILTER.** A filter designed to be used with filter aid.    **DIRECT ACCESS FROM THE HOME.** Means any opening which discharges into the "perimeter" of the pool or any opening in an exterior dwelling wall, or interior wall (for indoor pools) which faces the pool.    **EXIT ALARM.** A device that makes audible, continuous alarm sounds when any door or window which permits access from the residence to any pool that is without an intervening enclosure is opened or left ajar.    **FACE PIPING.** Piping, with all valves and fittings, which is used to connect the filter system together as a unit.    **FILTER.** Any apparatus by which water is clarified.    **FILTER AID.** A nonpermanent type of filter medium or aid such as diatomite, alum, etc.    **FILTER CARTRIDGE.** A disposable or renewable filter element which generally employs no filter aid.    **FILTER ELEMENT.** That part of a filter which retains the filter medium.    **FILTER MEDIUM.** Fine material which entraps the suspended particles and removes them from the water.    **FILTER RATE.** Average rate of flow per square foot of filter area.    **FILTER ROCK.** Specially graded rock and gravel used to support filter sand.    **FILTER SAND.** A specially graded type of permanent filter medium.    **FILTER SEPTUM.** That part of the filter element in a diatomite type filter upon which a cake of diatomite or other nonpermanent filter aid may be deposited.    **FILTER WASTE DISCHARGE PIPING.** Piping that conducts waste water from a filter to a drainage system. Connection to drainage system is made through an air gap or other approved methods.    **FRESH WATER.** Those waters having a specific conductivity less than a solution containing 6,000 ppm of sodium chloride.    **HIGH RATE SAND FILTER.** A sand filter designed for flows in excess of 5 gpm per square feet.    **HOT TUB.** See "Swimming pool."    **INGROUND POOL.** See "Swimming pool."    **INLET FITTING.** Fitting or fixture through which circulated water enters the pool.    **MAIN SUCTION OUTLET.** Outlet at the deep portion of the pool through which the main flow of water leaves the pool when being drained or circulated.    **MEDICALLY FRAIL ELDERLY PERSON.** Means any person who is at least 65 years of age and has a medical problem that affects balance, vision, or judgment, including but not limited to a heart condition, diabetes, or Alzheimer's disease or any related disorder.    **MESH SAFETY BARRIER.** A combination of materials, including fabric, posts, and other hardware to form a barrier around a swimming pool.    **POOL.** See "Swimming pool."    **POOL DEPTHS.** The distance between the floor of pool and the maximum operating water level.    **POOL PERIMETER.** A pool perimeter is defined by the limits of the pool deck, its surrounding area including yard area on same property, and any dwelling or nondwelling wall or any combination thereof which completely surrounds the pool.    **POOL PLUMBING.** All chemical, circulation, filter waste discharge piping, deck drainage and water filling system.    **PORTABLE POOL.** A prefabricated pool which may be erected at the point of intended use and which may be subsequently disassembled and reerected at a new location. Generally installed on the surface of the ground and without excavation.    **PRECOAT.** In a diatomite-type filter, the initial coating or filter aid placed on the filter septum at the start of the filter cycle.    **RAPID SAND FILTER.** A filter designed to be used with sand as the filter medium and for flows not to exceed 5 gpm per square feet.    **RECEPTOR.** An approved plumbing fixture or device of such material, shape and capacity as to adequately receive the discharge from indirect waste piping, so constructed and located as to be readily cleaned.    **RESIDENTIAL.** Means situated on the premises of a detached one-family or two-family dwelling or a one-family townhouse not more than three stories high.    **RETURN PIPING.** That portion of the circulation piping which extends from the outlet side of the filters to the pool.    **SALINE WATER.** Those waters having a specific conductivity in excess of a solution containing 6,000 ppm of sodium chloride.    **SEPARATION TANK.** A device used to clarify filter rinse or waste water. Sometimes called a reclamation tank.    **SKIM FILTER.** A surface skimmer combined with a vacuum diatomite filter.    **SPA, NONPORTABLE.** See "Swimming pool."    **SPA, PORTABLE.** Nonpermanent structure intended for recreational bathing, in which all controls and water heating and water circulating equipment are an integral part of the product and which is cord-connected and not permanently electrically wired.    **SUCTION PIPING.** That portion of the circulation piping located between the pool structure and the inlet side of the pump and usually includes main outlet piping, skimmer piping, vacuum piping and surge tank piping.    **SURFACE SKIMMER.** A device generally located in the pool wall which skims the pool surface by drawing pool water over a self adjusting weir.    **SWIMMING POOL, PRIVATE.** Any structure, located in a residential area, that is intended for swimming or recreational bathing and contains water over 24 inches (610 mm) deep including but not limited to inground, aboveground, and onground swimming pools, hot tubs, and nonportable spas.    **SWIMMING POOL, INDOOR.** A swimming pool which is totally contained within a structure and surrounded on all four sides by walls of said structure.    **SWIMMING POOL, OUTDOOR.** Any swimming pool which is not an indoor pool.    **SWIMMING POOL, PUBLIC.** A water-tight structure of concrete, masonry, fiberglass, stainless steel or plastic which is located either indoors or outdoors, used for bathing or swimming by humans, and filled with a filtered and disinfected water supply, together with buildings, appurtenances and equipment used in connection therewith. A public swimming pool or public pool shall mean a conventional pool, spa-type pool, wading pool, special purpose pool or water recreation attraction, to which admission may be gained with or without payment of a fee and includes, pools operated by or serving camps, churches, cities, counties, day care centers, group home facilities for eight or more clients, health spas, institutions, parks, state agencies, schools, subdivisions; or the cooperative living-type projects of five or more living units, such as apartments, boarding houses, hotels, mobile home parks, motels, recreational vehicle parks and townhouses.    **SWIMMING POOL, RESIDENTIAL.** See "Swimming pool, private."    **TURNOVER TIME.** The time in hours required for the circulation system to filter and recirculate a volume of water equal to the pool volume.    **VACUUM FITTING.** A fitting in the pool which is used as a convenient outlet for connecting the underwater suction cleaning equipment.    **VACUUM PIPING.** The piping from the suction side of a pump connected to a vacuum fitting located at the pool and below the water level.    **WASTE PIPING.** See "Filter waste discharge piping."    **WIDTH AND/OR LENGTH.** Actual water dimension taken from wall to wall at the maximum operating water level.    **YOUNG CHILD.** Means any person under the age of six years.    **R4501.3 Mechanical requirements.** Unless otherwise specified in this code, all piping, equipment and materials used in the process piping system of swimming pools that are built in place shall conform to the *Florida Building Code, Plumbing*.    **R4501.4 Approvals.**    **R4501.4.1 Compliance.** All materials, piping, valves, equipment or appliances entering into the construction of swimming pools or portions thereof shall be of a type complying with this code or of a type recommended and approved by a nationally recognized testing agency or conforming to other recognized standards acceptable to the administrative authority.    **R4501.4.2 Items not covered.** For any items not specifically covered in these requirements, the administrative authority is hereby authorized to require that all equipment, materials, methods of construction and design features shall be proven to function adequately, effectively and without excessive maintenance and operational difficulties.  **R4501.4.2.1. Flood hazard areas.**  Pools installed in flood hazard areas established in Section R322 shall comply with Section R322.2.4 (A Zones) or R322.3.3.1 in coastal high-hazard areas (V Zones).  **[SP5294 AS]**  **R4501.4.3 Applicant responsibility.** It shall be the responsibility of the applicant to provide such data, tests or other adequate proof that the device, material or product will satisfactorily perform the function for which it is intended, before such item shall be approved or accepted for tests.    **R4501.5 Alternate materials and methods of construction.**    **R4501.5.1 Approval and authorization.** The provisions of this code are not intended to prevent the use of any alternate material, method of construction, appliance or equipment, provided any such alternate has been first approved and its use authorized by the administrative authority.    **R4501.5.2 Required tests.** When there is insufficient evidence to substantiate claims for alternates, the administrative authority may require tests, as proof of compliance, to be made by an approved agency at the expense of the applicant.    **R4501.6 Engineering design.**    ***Section R4501.6.1 Conformance standard. Change to read as shown:***  **R4501.6.1 Conformance standard.** Design, construction and workmanship shall be in conformity with the requirements of ANSI/NSPI 3; ANSI/APSP/ICC~~NSPI~~ 4; ANSI/ APSP/ICC~~NSPA~~ 5; ANSI/ APSP/ICC~~NSPI~~ 6; and ANSI/APSP 7.  **(SW6267GlitchAS)**  **R4501.6.2 Required equipment.** Every swimming pool shall be equipped complete with approved mechanical equipment consisting of filter, pump, piping valves and component parts.    **Exception:** Pools with a supply of fresh water equivalent to the volume of the pool in the specified turnover time will be allowed.    **R4501.6.3 Water velocity.** Pool piping shall be designed so the water velocity will not exceed 10 feet per second (3048 mm/s) for pressure piping and 8 feet per second (2438 mm/s) for suction piping, except that the water velocity shall not exceed 8 feet per second (2438 mm/s) in copper tubing. Main suction outlet velocity must comply with ANSI/APSP 7.    **Exception:** Jet inlet fittings shall not be deemed subject to this requirement.    **R4501.6.4 Piping to heater.** Water flow through the heater, any bypass plumbing installed, any back-siphoning protection, and the use of heat sinks shall be done in accordance with the manufacturer's recommendations.    **R4501.6.5 Piping installation**. All piping materials shall be installed in strict accordance with the manufacturer's installation standards.    **Exception:** Primer and glue on exposed aboveground piping not required to be colored.    **R4501.6.6 Entrapment protection.** Entrapment protection for suction outlets shall be installed in accordance with requirements of ANSI/APSP 7.    **R4501.7 Pumps.**    **R4501.7.1 Strainer.** Pool circulating pumps shall be equipped on the inlet side with an approved type hair and lint strainer when used with a pressure filter.    **R4501.7.2 Installation.** Pumps shall be installed in accordance with manufacturer recommendations.    **R4501.7.3 Capacity.** Pumps shall have design capacity at the following heads:  1. Pressure Diatomaceous Earth-At least 60 feet (18 288 mm).  2. Vacuum Diatomaceous Earth-20-inch (508 mm) vacuum on the suction side and 40-foot (12 192 mm) total head.  3. Rapid Sand-At least 45 feet (13 716 mm).  4. High Rate Sand-At least 60-feet (18 288 mm).  **R4501.7.4 Materials.** Pump impellers, shafts, wear rings and other working parts shall be of corrosion-resistant materials.    **R4501.8 Valves.**    **R4501.8.1 General.** Valves shall be made of materials that are approved in the *Florida Building Code, Plumbing*. Valves located under concrete slabs shall be set in a pit having a least dimension of five pipe diameters with a minimum of at least 10 inches (254 mm) and fitted with a suitable cover. All valves shall be located where they will be readily accessible for maintenance and removal.    **R4501.8.2 Full-way (gate) valves.** Full-way valves shall be installed to insure proper functioning of the filtration and piping system. When the pump is located below the overflow rim of the pool, a valve shall be installed on the discharge outlet and the suction line.    **R4501.8.3 Check valves.** Where check valves are installed they shall be of the swing, spring or vertical check patterns.    **R4501.8.4 Combination valves.** Combination valves shall be installed per manufacturer's installation instructions.    **R4501.9 Water supply.** Unless an approved type of filling system is installed, any water supply which in the judgment of the administrative authority may be used to fill the pool, shall be equipped with backflow protection. No over the rim fill spout shall be accepted unless located under a diving board, or properly guarded.    **R4501.10 Waste water disposal.**    **R4501.10.1 Connection limitations.** Direct or indirect connections shall not be made between any storm drain, sewer, drainage system, seepage pit underground leaching pit, or subsoil drainage line, and any line connected to a swimming pool unless approved by the administrative authority.    **R4501.10.2 Disposal through public sewer.** When the waste water from a swimming pool is to be disposed of through a public sewer, a 3-inch (76 mm) P-trap shall be installed on the lower terminus of the building drain and the tall piece from the trap shall extend a minimum of 3-inches (76 mm) above finished grade and below finished floor grade. This trap need not be vented. The connection between the filter waste discharge piping and the P-trap shall be made by means of an indirect connection.    **R4501.10.3 Deviations.** Plans and specifications for any deviation from the above manner of installation shall first be approved by the administrative authority before any portion of any such system is installed. When waste water disposal is to seepage pit installation, it shall be installed in accordance with the approval granted by the administrative authority.    **R4501.11 Separation tank.** A separation tank of an approved type may be used in lieu of the aforementioned means of waste water disposal when connected as a reclamation system.    **R4501.12 Tests.**    **R4501.12.1 Pressure test.** All pool piping shall be tested and proved tight to the satisfaction of the administrative authority, under a static water or air pressure test of not less than 35 pounds per square inch (psi) (241 kPa) for 15 minutes.  **Exception:** Circulating pumps need not be tested as required in this section.    **R4501.12.2 Drain and waste piping.** All drain and waste piping shall be tested by filling with water to the point of overflow and all joints shall be tight.    **R4501.13 Drain piping.**    **R4501.13.1 Slope to discharge.** Drain piping serving gravity overflow gutter drains and deck drains shall be installed to provide continuous grade to point of discharge.    **R4501.13.2 Joints and connections.** Joints and connections shall be made as required by the *Florida Building Code, Plumbing*.    **R4501.14 Water heating equipment.**    **R4501.14.1 Labels.** Swimming pool water heating equipment shall conform to the design, construction and installation requirements in accordance with accepted engineering practices and shall bear the label of a recognized testing agency, and shall include a consideration of combustion air, venting and gas supply requirements for water heaters.    **R4501.14.2 Water retention.** If a heater is not equipped or designed for an approved permanent bypass or antisiphon device, an approved permanent bypass or antisiphon device shall be installed to provide a positive means of retaining water in the heater when the pump is not in operation.    **R4501.14.3 Pit drainage.** When the heater is installed in a pit, the pit shall be provided with approved drainage facilities.    **R4501.14.4 Connections.** All water heating equipment shall be installed with flanges or union connection adjacent to the heater.    **R4501.14.5 Relief valve.** When water heating equipment which is installed in a closed system has a valve between the appliance and the pool, a pressure relief valve shall be installed on the discharge side of the water heating equipment. For units up to and including 200,000 Btu/hour input, the relief valve shall be rated by the American Gas Association.    **R4501.15 Gas piping.** Gas piping shall comply with the *Florida Building Code, Fuel Gas*.    **R4501.16 Electrical.** Electrical wiring and equipment shall comply with ~~Chapter 27 of~~ the *Florida Building Code*.    **R4501.17 Residential swimming barrier requirement.** Residential swimming pools shall comply with Sections R4501.17.1 through R4501.17.3.    **Exception:** A swimming pool with an approved safety pool cover complying with ASTM F 1346.    **R4502.17.1 Outdoor swimming pools.** Outdoor swimming pools shall be provided with a barrier complying with R4501.17.1.1 through R4501.17.1.14.    **R4501.17.1.1** The top of the barrier shall be at least 48 inches (1219 mm) above grade measured on the side of the barrier which faces away from the swimming pool. The maximum vertical clearance between grade and the bottom of the barrier shall be 2 inches (51 mm) measured on the side of the barrier which faces away from the swimming pool. Where the top of the pool structure is above grade the barrier may be at ground level or mounted on top of the pool structure. Where the barrier is mounted on top of the pool structure, the maximum vertical clearance between the top of the pool structure and the bottom of the barrier shall be 4 inches (102 mm).    **R4501.17.1.2** The barrier may not have any gaps, openings, indentations, protrusions, or structural components that could allow a young child to crawl under, squeeze through, or climb over the barrier as herein described below. One end of a removable child barrier shall not be removable without the aid of tools. Openings in any barrier shall not allow passage of a 4-inch-diameter (102 mm) sphere.    **R4501.17.1.3** Solid barriers which do not have openings shall not contain indentations or protrusions except for normal construction tolerances and tooled masonry joints.    **R4501.17.1.4** Where the barrier is composed of horizontal and vertical members and the distance between the tops of the horizontal members is less than 45 inches (1143 mm), the horizontal members shall be located on the swimming pool side of the fence. Spacing between vertical members shall not exceed 1 ¾ inches (44 mm) in width. Where there are decorative cutouts within vertical members, spacing within the cutouts shall not exceed 1 ¾ inches (44 mm) in width.    **R4501.17.1.5** Where the barrier is composed of horizontal and vertical members and the distance between the tops of the horizontal members is 45 inches (1143 mm) or more, spacing between vertical members shall not exceed 4 inches (102 mm). Where there are decorative cutouts within vertical members, spacing within the cutouts shall not exceed 1¾ inches (44 mm) in width.    **R4501.17.1.6** Maximum mesh size for chain link fences shall be a 2 ¼ inch square (57 mm) unless the fence is provided with slats fastened at the top or bottom which reduce the openings to no more than 1¾inches (44 mm).    **R4501.17.1.7** Where the barrier is composed of diagonal members, the maximum opening formed by the diagonal members shall be no more than 1¾inches (44 mm).    **R4501.17.1.8** Access gates, when provided, shall be self-closing and shall comply with the requirements of Sections R4501.17.1.1 through R4501.17.1.7 and shall be equipped with a self-latching locking device located on the pool side of the gate. Where the device release is located no less than 54 inches (1372 mm) from the bottom of the gate, the device release mechanism may be located on either side of the gate and so placed that it cannot be reached by a young child over the top or through any opening or gap from the outside. Gates that provide access to the swimming pool must open outward away from the pool. The gates and barrier shall have no opening greater than ½ inch (12.7 mm) within 18 inches (457 mm) of the release mechanism.    **R4501.17.1.9** Where a wall of a dwelling serves as part of the barrier, one of the following shall apply:    1. All doors and windows providing direct access from the home to the pool shall be equipped with an exit alarm complying with UL 2017 that has a minimum sound pressure rating of 85 dB A at 10 feet (3048 mm). Any deactivation switch shall be located at least 54 inches (1372 mm) above the threshold of the access. Separate alarms are not required for each door or window if sensors wired to a central alarm sound when contact is broken at any opening**.**    **Exceptions:**    a. Screened or protected windows having a bottom sill height of 48 inches (1219 mm) or more measured from the interior finished floor at the pool access level.    b. Windows facing the pool on floor above the first story.    c. Screened or protected pass-through kitchen windows 42 inches (1067 mm) or higher with a counter beneath.    2. All doors providing direct access from the home to the pool must be equipped with a self-closing, self-latching device with positive mechanical latching/locking installed a minimum of 54 inches (1372 mm) above the threshold, which is approved by the authority having jurisdiction.  **R4501.17.1.10** Where an aboveground pool structure is used as a barrier or where the barrier is mounted on top of the pool structure, and the means of access is a ladder or steps, the ladder or steps either shall be capable of being secured, locked or removed to prevent access, or the ladder or steps shall be surrounded by a barrier which meets the requirements of Sections R4501.17.1.1 through R4501.17.1.9 and Sections R4501.17.1.12 through R4501.17.1.14. When the ladder or steps are secured, locked or removed, any opening created shall not allow the passage of a 4-inch-diameter (102 mm) sphere.    **R4501.17.1.11** Standard screen enclosures which meet the requirements of Section R4501.17 may be utilized as part of or all of the "barrier" and shall be considered a "nondwelling" wall. Removable child barriers shall have one end of the barrier nonremovable without the aid of tools.    **R4501.17.1.12** The barrier must be placed around the perimeter of the pool and must be separate from any fence, wall, or other enclosure surrounding the yard unless the fence, wall, or other enclosure or portion thereof is situated on the perimeter of the pool, is being used as part of the barrier, and meets the barrier requirements of this section.    **R4501.17.1.13** Removable child barriers must be placed sufficiently away from the water's edge to prevent a young child or medically frail elderly person who may manage to penetrate the barrier from immediately falling into the water. Sufficiently away from the water's edge shall mean no less than 20 inches (508 mm) from the barrier to the water's edge. Dwelling or nondwelling walls including screen enclosures, when used as part or all of the "barrier" and meeting the other barrier requirements, may be as close to the water's edge as permitted by this code.    **R4501.17.1.14** A wall of a dwelling may serve as part of the barrier if it does not contain any door or window that opens to provide direct access from the home to the swimming pool.    **R4501.17.1.14.1 Adjacent waterways.** Permanent natural or permanent man-made features such as bulkheads, canals, lakes, navigable waterways, etc., adjacent to a public or private swimming pool or spa may be permitted as a barrier when approved by the authority having jurisdiction. When evaluating such barrier features, the authority may perform on-site inspections and review evidence such as surveys, aerial photographs, water management agency standards and specifications, and any other similar documentation to verify, at a minimum, the following:    1. The barrier feature is not subject to natural changes, deviations, or alterations and is capable of providing an equivalent level of protection as that provided by the code.    2. The barrier feature clearly impedes, prohibits or restricts access to the swimming pool or spa.    **R4501.17.1.15** A mesh safety barrier meeting the requirements of Section R4501.17 and the following minimum requirements shall be considered a barrier as defined in this section:    1. Individual component vertical support posts shall be capable of resisting a minimum of 52 pounds (229 N) of horizontal force prior to breakage when measured at a 36-inch (914 mm) height above grade. Vertical posts of the child mesh safety barrier shall extend a minimum of 3 inches (76 mm) below deck level and shall be spaced no greater than 36 inches (914 mm) apart.    2. The mesh utilized in the barrier shall have a minimum tensile strength according to ASTM D 5034 of 100 lbf., and a minimum ball burst strength according to ASTM D 3787 of 150 lbf. The mesh shall not be capable of deformation such that a ¼ -inch (6.4 mm) round object could pass through the mesh.    The mesh shall receive a descriptive performance rating of no less than "trace discoloration" or "slight discoloration" when tested according to ASTM G 53 (Weatherability, 1,200 hours).    3. When using a molding strip to attach the mesh to the vertical posts, this strip shall contain, at a minimum, #8 by ½-inch (12.7 mm) screws with a minimum of two screws at the top and two at the bottom with the remaining screws spaced a maximum of 6 inches (152 mm) apart on center.    4. Patio deck sleeves (vertical post receptacles) placed inside the patio surface shall be of a nonconductive material.    5. A latching device shall attach each barrier section at a height no lower than 45 inches (11 613 mm) above grade. Common latching devices that include, but are not limited to, devices that provide the security equal to or greater than that of a hook and eye type latch incorporating a spring actuated retaining lever (commonly referred to as a safety gate hook).    6. The bottom of the child mesh safety barrier shall not be more than 1 inch (25 mm) above the deck or installed surface (grade).    **R4501.17.2 Indoor swimming pools.** All walls surrounding indoor swimming pools shall comply with Section R4501.17.1.9.    **R4501.17.3 Prohibited locations.** A barrier may not be located in a way that allows any permanent structure, equipment, or window that opens to provide access from the home to the swimming pool.    **R4501.18 Ladders and steps.** All pools whether public or private shall be provided with a ladder or steps in the shallow end where water depth exceeds 24 inches (610 mm). In private pools where water depth exceeds 5 feet (1524 mm), there shall be ladders, stairs or underwater benches/swimouts in the deep end. Where manufactured diving equipment is to be used, benches or swimouts shall be recessed or located in a corner.    **Exception:** In private pools having more than one shallow end, only one set of steps are required. A bench, swim-out or ladder may be used at all additional shallow ends in lieu of an additional set of steps.    **R4501.19 Final inspection.** Final electrical, and barrier code, inspection shall be completed prior to filling the pool with water.    **Exception:** Vinyl liner and fiberglass pools are required to be filled with water upon installation.    **R4501.20 Filters.** Components shall have sufficient capacity to provide a complete turnover of pool water in 12 hours or less.    **R4501.20.1 Sand filters.**    **R4501.20.1.1 Approved types.** Rapid sand filters (flow up to 5 gpm per square foot) shall be constructed in accordance with approved standards. Where high rate sand filters (flow in excess of 5 gpm per square foot) are used, they shall be of an approved type. The circulation system and backwash piping shall be adequate for proper backwashing of said filter and shall provide backwash flow rates of at least 12 gpm per square foot or rapid sand filters or 15 gpm per square foot or high rate sand filters.    **R4501.20.1.2 Instructions.** Every filter system shall be provided with written operating instructions.    **R4501.20.1.3 Filter system equipment.** On pressure type filters, a means shall be provided to permit the release of internal pressure. A filter incorporating an automatic internal air release as its principal means of air release shall have lids which provide a slow and safe release of pressure as part of its design. A separation tank used in conjunction with a filter tank shall have as part of its design a manual means of air release or a lid which provides a slow and safe release of pressure as it is opened.      **R4501.20.2 Diatomite type filters.**    **R4501.20.2.1 Design.** Diatomite-type filters shall be designed for operation under either pressure or vacuum. The design capacity for both pressure and vacuum filters shall not exceed 2 gpm per square foot of effective filter area.    **R4501.20.2.2 Filter aid.** Provision shall be made to introduce filter aid into the filter in such a way as to evenly precoat the filter septum.    **R4501.21 Pool fittings.**    **R4501.21.1 Approved type.** Pool fittings shall be of an approved type and design as to be appropriate for the specific application.    **R4501.21.2 Skimmers.** Approved surface skimmers are required and shall be installed in strict accordance with the manufacturer's installation instructions. Skimmers shall be installed on the basis of one per 800 square feet (74 m2) of surface area or fraction thereof, and shall be designed for a flow rate of at least 25 gallons per minute (gpm) (1.6 L/s) per skimmer.    **R4501.21.3 Main outlet.** An approved main outlet, when provided, shall be located on a wall or floor at or near the deepest point in the pool for emptying or circulation, or both, of the water in the pool.    **R4501.21.4 Hydrostatic relief device.** In areas of anticipated water table an approved hydrostatic relief device shall be installed.    **Exception:** Plastic liner pools (where there is no structural bottom to the pool).    **R4501.21.5 Inlet fittings.** Approved manufactured inlet fittings for the return of recirculated pool water shall be provided on the basis of at least one per 300 square feet (28 m2) of surface area. Such inlet fittings shall be designed and constructed to insure an adequate seal to the pool structure and shall incorporate a convenient means of sealing for pressure testing of the pool circulation piping. Where more than one inlet is required, the shortest distance between any two required inlets shall be at least 10 feet (3048 mm).    **R4501.22 Equipment foundations and enclosures.** All pool motors and equipment shall be installed in compliance with the manufacturer’s recommendations. All heating and electrical equipment, unless approved for outdoor installation, shall be adequately protected against the weather or installed within a building.    **R4501.23 Accessibility and clearances.** Equipment shall be so installed as to provide ready accessibility for cleaning, operating, maintenance and servicing.  **(SW5443 AS)**    ***CHAPTER 46. Change Chapter 44, Referenced Standards, to Chapter 46 as follows:***      **CHAPTER 46 ~~44~~, REFERENCED STANDARDS** |
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**[CA5460 AS]**

***Add or update the following standards:***

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| --- | --- | --- |
|  |  | **AAF Aluminum Association of Florida, Inc.**  3165 McCrory Place, Suite 185  Orlando, FL 32803  Standard Referenced in code  reference number Title section number  AAF-10 Guide to Aluminum Construction in High Wind Areas 2010 R301.2.1.1.1  **[S5840 AS]** |
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**AAMA American Architectural Manufacturers Association**

1827 Walden Office Square, Suite 550

Schaumburg, IL 60173-4268

Standard Referenced in code

reference number Title section number

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| --- | --- | --- |
| 101/I.S.2-97  101/I.S.2/NAFS-02  AAMA/WDMA/CSA  101/I.S.2/A440—05 or 08 or 11 | Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood  Windows and Glass Doors  Voluntary Performance Specifications for Windows, Skylights  And Glass Doors  North American Fenestration Standards/Specifications for Windows,  Doors and Skylights | R308.6.9, R612.3  R308.6.9, R612.3  R308.6.9, R612.3, ~~N1102.4.3~~ |
|  |  |  |
| 506—06 or 08 or 11  **(S5664 AM R1)**  **(S6086 AM)** | Voluntary Specifications for ~~Hurricane~~ Impact and Cycle Testing of Fenestration Products | R301.2.1.2  R612.6.1 |
| AAMA/NPEA/NSA2100-11  **[S5895 AS](S5659 AM)** | Voluntary Specifications for Sunrooms | R202, R301.2.1.1.2.1, R301.2.1.1.2.1 |
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FMA/AAMA 100-12 Standard Practice for the Installation of Windows with Flanges or Mounting R703.8

FMA/AAMA 200-12~~09~~ Standard Practice for the Installation of Windows with Frontal Flanges

for Surface Barrier Masonry R703.8

FMA/WDMA 250-10 Standard Practice for the Installation of Non-Frontal Flange Windows with R703.8

Mounting Flanges for Surface Barrier Masonry for Extreme Wind/Water Conditions

FMA/AAMA/WDMA300-12 Standard Practice for the Installation of Exterior Doors in Wood Frame

Construction for Extreme Wind/Water Exposure R703.8

**[S5891 AM R1)(S5686 AM)**

AAMA 800-05 Voluntary Specifications and Test Methods for Sealants R703.8

AAMA 812-04 Voluntary Practice for Assessment of Single Component Aerosol Expanding

Polyurethane Foams for Sealing Rough Openings of Fenestration Installations R703.8

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**ACI**

530-11 Building Code Requirements for Masonry Structures R301.2.1.1, R318.4, R606.2.4

**AFPA American Forest and Paper Association (American Wood Council Division)**

111 19th Street, NW, #800

Washington, DC 20036

Standard Referenced in code

reference number Title section number

NDS—12 National Design Specification (NDS) for Wood Construction with R404.2.2, R502.2, Table R503.1,

2005 Supplement R602.2, ~~Table R602.3.1,~~ R611.9.2, R611.9.3, R802.2

AFPA—2012 Span Tables for Joists and Rafters Reserved ~~R502.2.2, R802.2.2, R802.2.3~~

WFCM—2012 Wood Frame Construction Manual for One- and Two-family Dwellings R301.2.1.1,

R614.2.5, R614.3.1, ~~R802.2~~

**APSP Association of Pool and Spa Professionals**

2111 Eisenhower Avenue

Alexandria, VA 22314

Standard                                                                                                               Referenced in code

reference number    Title                                                                                           section number

ANSI/NSPI 3—99               American National Standard for Permanently Installed

Residential  Spas………………………………………..…..~~AG104~~  R4501.6.1

ANSI/APSP/ICC~~NSPI~~ 4—12~~99~~     American National Standard for Aboveground**/**On ground                       ~~AG103.2~~

                                                        Residential Swimming Pools. . . . ............................................................R4501.6.1

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ANSI/APSP/ICC ~~NSPI~~ 5-~~03~~11        American National Standard for Residential In ground Swimming        ~~AG103.1~~

Pools………………………………………………………………….  R4501.6.1

ANSI/APSP/ICC~~NSPI~~ 6—13~~99~~       American National Standard for Portable Spas. . . . . . .. . . . ~~AG104.2,~~ R4501.6.1

ANSI/APSP 7—06     American National Standard for Suction Entrapment Avoidance

                                        In Swimming Pools, Wading Pools, Spas, Hot Tubs, and Catch              ~~AG106.1~~

                                         Basins…………….…………………………  R4501.6.1, R4501.6.3, R4501.6.6

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| **(SW6264GlitchAS)** |  |

**ASCE American Society of Civil Engineers**

1801 Alexander Bell Drive

Reston, VA 20191-4400

Standard Referenced in code

reference number Title section number

7— 10 Minimum Design Loads for Buildings and Other Structures R301.2.1.1, R301.2.1.2.1.1, R301.2.1.6,

Table R611.3(1), Table R611.7.4, R612.2,

R612.4, R615.1, Table R907.8.1

24—05 Flood-resistant Design and Construction R301.2.4, R301.2.4.1, R322.1, R322.1.1,

R322.1.6, R322.1.9, R322.2.2,

R322**.**3.3.1, ~~AG103.3~~

**ASTM ASTM International**

100 Barr Harbor Drive

West Conshohocken, PA 19428

Standard Referenced in code

reference number Title section number

A 653/A 653M-08 Specification for Steel Sheet, Zinc Coated (Galvanized) or Zinc Iron

Alloy Coated (Galvanized) by the Hot Dip Process R317.3.1, R505.2.1, R505.2.3, ~~R603.2.1,~~

~~R603.2.3~~, Table R606.15.1, R611.5.2.3, ~~R804.2.1~~, ~~R804.2.3~~,

Table R905.4.4, ~~Table R905.10.3(1),~~ ~~Table R905.10.3(2),~~ M1601.1.1

B 370—09 Specification for Copper Sheet and Strip for Building Construction ~~Table R905.2.8.2~~, Table R905.4.4,

~~Table R905.10.3(1),~~ Table P2701.1

C 90—11b~~08~~ Specification for Load Bearing Concrete Masonry Units Table R301.2(1)

**[S6019AS]**

C 920 - 08 Specification for Elastomeric Joint Sealants ………………….. R406.4.1, R703.8

C 926-11a~~06~~ Specification for Application of Portland Cement Based Plaster [No change] **[S6019AS]**

C 1063-12a~~08~~ Specification for Installation of Lathing and Furring to Receive Interior and

Exterior Portland Cement-based Plaster [No change]

**[S6019AS]**

C 1281- 03 Standard Specification for Preformed Tape Sealants for Glazing Applications .… R703.8

D 226-06 Specification for Asphalt saturated (Organic Felt) Used in Roofing and Waterproofing

R703.2, R905.2.3, R905.2.7, R905.3.3, R905.4.3, ~~R905.4.3.2,~~ R905.5.3,

~~R905.5.3.2,~~ R905.5.3.3, R905.6.3, ~~R905.6.3.2,~~ R905.6.3.3, R905.7.3,

Table R905.7.5, R905.8.3, ~~R905.8.3.2,~~ R905.8.3.3, R905.8.3.4, ~~R905.8.4,~~

R905.8.10.1, Table R905.9.2, R905.10.5, ~~R905.10.5.1~~ R905.10.5.2, R907.7.2

D 5034—95 Test Method for Breaking Strength and Elongation of Textile Fabrics (Grab Test) Specifications

for Adhesives for Field Gluing Plywood to Lumber Framing for Floor Systems R4501.17.1.15

D 6509-00 Specification for Atactic Polypropylene (APP) Modified Bituminous

Base Sheet Materials Using Glass Fiber Reinforcements ………… ……….Table R905.11.2

D6757- 07 Standard Specification for Underlayment Felt Containing Inorganic Fibers

Used in Steep-Slope Roofing R905.2.3, R905.4.3, R905.4.3.3, R905.5.3,

R905.5.3.3, R905.6.3, R905.6.3.3, R905.10.5, R905.10.5.2

D7158- 07 Standard Test Method for Wind Resistance of Sealed Asphalt Shingles

(Uplift Force/Uplift Resistance Method) ~~R905.2.4.1, Table R905.2.4.1(1),~~

R905.2.6.1, Table R905.2.6.1

E 1300—04e01, Practice for Determining Load Resistance of Glass in Buildings R612.3

07e01 or 09a~~98~~ **[S5664 AM]**

E 1886-02 or 05 Test Method for Performance of Exterior Windows, Curtain Walls, Doors and R301.2.1.2, R301.2.1.2.1.1

Storm Shutters Impacted by Missiles and Exposed to Cyclic Pressure Differentials R612.3.1, R612.6.1

**[S5664 AM]**

E1996-02, 05 Standard Specification for Performance of Exterior Windows, Curtain Walls, Doors

06, 09, or 12 and Impact Protective Systems Impacted by Windborne Debris in Hurricanes R301.2.1.2, R612.6.1

**[S5664 AM]**

F 1346-91(2003) Performance Specification for Safety Covers and Labeling Requirements for ~~AG105.2, AG105.5,~~

All Covers for Swimming Pools , Spas and Hot Tubs R4501.2, R4501.17

**Florida Codes Florida Building Commission**

Building Codes and Standards

Department of Business and Professional Regulation

1940 N. Monroe Street, Suite 90A

Tallahassee, Fl. 32399-0772

Standard Referenced in code

reference number Title section number

FBC-A-14 Florida Building Code, Accessibility R320.1

FBC-B-14 Florida Building Code, Building R101.2.1, R301.2.1.1, R301.2.1.1.3, R322.1.11,

R403.1.2, R404.5.1, R606.13.8, R615.1, R802.1.3.4,

R905.3.1, R905.10.3, Table R907.7.1.2, R907.7.2,

R907.8, M1904.1, R4201.1.2, R4401.1, R4402.1,

R4403.1, R4404.1, R4405.1, R4406.1, R4407.1,

R4408.1, R4409.1, R4410.1, R4411.1, R4412.1

FBC-EC-14 Florida Building Code, Energy Conservation R302.13, R408.3, R806.5, N1101~~.2~~,

FBC-EB-14 Florida Building Code, Existing Building R101.2, R907.1, M1202.1, P2502, Table E3403

FFPC-14 Florida Fire Prevention Code M1904.1, M2201.7, G2412.2

dFBC-FG-14 Florida Building Code, Fuel Gas M1301.1, M1904.1, G2401.1, G2423.1,

Table E3403, R4201.1.2, R4201.15

FBC-M-14 Florida Building Code, Mechanical M1301.1, M1304.7.2, G2402.3, Table E3403,

R4201.1.2, R4201.15

FBC-P-14 Florida Building Code, Plumbing Table R301.2(1), R903.4.1,G2402.3, P2601.1, P2902.5.5,

Table E3403, R4201.1.2, R4201.3, R4201.8.1,

64E Rule 64E, Florida Administrative Code (Sewage Disposal) R322.1.7

FS Florida Statutes R202, R318.1.7, R502.1.10.1, R802.1.7.2

FBC-TPHVHZ-14 Florida Building Code, Test Protocols for High Velocity Hurricane Zones

RAS 118 R905.3, R905.3.2, R905.3.3, R905.3.3.1, R905.3.6, R905.3.8

RAS 119 R905.3, R905.3.2, R905.3.3, R905.3.3.1, R905.3.6, R905.3.8

RAS120 R905.3, R905.3.2, R905.3.3, R905.3.3.1, R905.3.6, R905.3.8

TAS 107 R905.2.6.1, R905.16.3

TAS 114 R904.4.2, R904.4.3

TAS 201 R301.2.1.2, R612.3.1

TAS 202 R301.2.1.2, R612.3, R612.4

TAS 203 R301.2.1.2, R612.3.1

**FRSA Florida Roofing, Sheet Metal and**

Air Conditioning Contractors Association

4111 Metric Drive

Winter Park, Florida 32792

 Standard Referenced in code

reference number Title section number

FRSA/TRI Florida High Wind Concrete and Clay Roof Tile

Installation Manual, Fifth Edition, April 2012 R905.3, R905.3.2, R905.3.3, R905.3.3.1,

R905.3.6, R905.3.7, R905.3.7.1, R905.3.8

**[S5604 AS](S6065GlitchAS)**

**ICC International Code Council, Inc.**

500 New Jersey Avenue, NW, 6th Floor

Washington, DC 20001

**[Replace all references to IBC, IRC, IFGC, IMC, IPC, etc. with references to Florida Codes]**

 Standard Referenced in code

reference number Title section number

IBHS-2005 Guidelines for Hurricane Resistant Residential Construction

(with Errata) Figure R614.3(1), Figure R614.3(2)

ICC 400-12 Standard on the Design and Construction of Log Structures R301.1.1

ICC 600-08 Standard for Residential Construction in High -wind Regions R301.2.1.1

SSTD 12-99 Standard for Determining the Wind Resistance from Wind-Bourne Debris R301.2.1.2

**MAF Masonry Association of Florida**

398 Camino Gardens Blvd, Suite 108

Boca Raton, FL 33432

Standard Referenced in code

reference number Title section number

MAF-97 Guide to Concrete Masonry Residential Construction in High Wind Areas R301.2.1.1

**[S6018 AS]**

**NSF**

Standard Referenced in code

reference number Title section number

372-2010 Drinking Water System Com~~n~~ponents - Lead Content P2905.2.1

**(P6059GlitchAM)**

**UL**

2034-08 Standard for Single- and Multiple-station Carbon Monoxide Alarms R202, RR315.4

2075-04 Standard for Gas and Vapor Detectors and Sensors—with revisions

Through September 28, 2007 R202, R315.2

***Appendix E: Manufactured housing used as dwellings. Change to read as follows:***

**Appendix E: Chapter 9B-53, Standard for Mitigation of Radon in Existing Buildings.**

**See Appendix E of the 2010 FBCR**

***Appendix F: Radon control methods. Change to read as follows:***

**Appendix F: Chapter 9B-52, Florida Standard for Passive Radon-Resistant New Residential Building Construction.**

**See Appendix F of the 2010 FBCR.**

***Appendix G Swimming Pools, Spas and Hot Tubs. Change to read as shown.***

***APPENDIX G***

***SWIMMING POOLS, SPAS AND HOT TUBS***

***RESERVED.***